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Greenland Hydropower Project Site 6g

Prefeasibility report

05-18015

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1 Background and purpose

1.1 Project Overview

1.1.1 Background

The Greenland Power Project (Greenland Project) consists of a greenfield aluminum reduction plant to be developed at a site in or near the town of Maniitsoq in western Greenland and a number of hydro developments to provide electricity for the smelter. The project will involve the development of sufficient hydro power resources among two identified hydro areas in Western Greenland to serve the requirements of the aluminum plant and the development of power transmission facilities between the hydro resources and the aluminum plant. Alcoa will develop the aluminum plant facilities on the plant site, as well as a harbor facility near the plant. The hydro sites will be developed and operated to meet the needs of the aluminum plant. During construction, other infrastructures between population centers, the harbor, the hydro sites and the smelter site will also be developed.

In 2007, during Phase I of the Memorandum of Understanding (MOU), PB Power (PB), with the assistance of outside consultants, completed various field studies and technical analyses as part of an initial feasibility study including: Phase I of the geotechnical and hydrologic investigations; field measurements of flow and sediment; transmission line conceptual design and cost study; office studies of scope and cost of three potential hydro sites (7e, 7d and 6g); and aerial survey and topographic mapping. Based on the results of these field and office studies, conceptual engineering for the project such as dams, tunnels, canals, roads and transmission lines were completed. The power availability at each of the three sites was determined based on the results of a hydrologic investigation as well as the field studies. A conceptual hydro project schedule and cost estimate was also prepared.

Originally, there were five hydropower sites considered as sources of energy for the Greenland Project. Prior to Phase I of the MOU, the three most favorable sites were chosen for further study to provide approximately 600 to 750 MW of power to the aluminum smelter. In MOU Phase I, Site 7d was excluded since the available power at the site could be obtained with an increase water storage capacity at site 7e.

1.1.2 Project status

MOU Phase I concluded on April 20, 2008. Additional investigations for the remainder of Phase I, completed in 2008 by Alcoa, included: refinement of the conceptual quantities; further review of future hydrology; further review of project works required for future hydrology; further review/resolution of transmission line technical/cost data and additional evaluation of mechanical/electrical equipment. These studies further updated the cost/schedule estimate for the project and provided input to the preliminary design and 2007 field study program to be coordinated by PB Power.

During the spring of 2008, Alcoa and the Greenland Homerule Government concluded that it was desirable to continue further evaluation and development of the Project and begin the second phase of investigative activities, MOU Phase II. Phase II continued until Fall 2009.

If, following the conclusion of Phase II activities, Alcoa and the Greenland Home rule Government conclude that it is desirable to pursue further evaluation and development of the Project, they will begin the third phase of activities. The third phase is currently expected to start in 2010 with construction beginning in 2011.

1.1.3 Scope of Work for Phase II Studies

The scope of work is for engineering and construction related services during MOU Phase II to provide preliminary design, cost estimates and planning for the development of the hydro site components to meet the needs of the aluminum plant (650 MW), including the development of preliminary design drawings, construction related services, electrical and mechanical works services. Specifically, this includes the following:

- general layouts;
- design criteria and preliminary design for every major project component;
- firm power capacity of the project (650 MW needed at the smelter);
- preliminary construction cost estimate (+25%, -15%) for each site;
- preliminary construction schedule;
- risk assessment.

In addition to that main scope, a single site scheme has been developed to use the full potential of site 7e. This scheme includes higher dams. The firm power availability doesn't meet the full smelter requirement (533 MW with maximum operating level of 726 m) but reduces the capital cost significantly.

1.2 Hydro sites

1.2.1 Site 7e

1.2.1.1 Description

The 7e hydropower project is located at the western end of Lake Tasersiaq, approximately 100 km south of the town of Kangerlussuaq. This high head scheme's main components and key figures are presented in the project metrics table and include:

- A reservoir which will be created by raising Lake Tasersiaq's present water level (690 m) by 24 m. It's normal operating level will be between 680 m and 714 m, with 4 085 hm³ of live storage.
- The headworks, including:
 - Two asphalt core rockfill dams, with the larger one being approximately 55 m high;
 - A spillway including a 200 m long ungated concrete weir discharging into a side channel spillway, followed by a rock chute;
 - A temporary diversion tunnel and cofferdams;
 - A headrace channel and intake;
 - A 42 km access road between the harbor and the headworks.
- The conveyance structures and power system, including:
 - A 26.6 km headrace tunnel. At this stage of the study, it is planned to excavate the power tunnel using two TBMs.
 - An air cushion surge chamber;
 - An underground powerhouse equipped with 5 Pelton turbines, a transformer cavern, access and cable galleries;
 - A tailrace tunnel whose outlet discharges at Evighedsfjord;
 - A service building and harbor facility.

Table 1.1 7e project metrics

<i>Design Flood</i>	Permanent civil works - 1:10 000 years flood	2 710 m ³ /s
	Temporary civil works - 1:20 years flood	1 280 m ³ /s
<i>Water Levels</i>	<i>Reservoir</i>	
	Maximum operating level	714 m
	Minimum operating level	680 m
	Water level with the 1:10 000 years flood	717.3 m
	<i>Downstream - Fjord</i>	
	Maximum tide level	2.6 m
	Minimum tide level	-2.3 m
<i>Production Devices</i>	Number of turbines	5
	Types of turbines	Pelton
	Net head (at max level)	697 m
	Unit discharge	17.4 m ³ /s
	Maximum unit capacity (Generator output)	126.4 MW
	Voltage Output	13.8 kV
	Maximum generator output - total	595 MW
	Firm power	500 MW
<i>Headrace Canal</i>	Length	2 100 m
	Water velocity	0.65 m/s
<i>Intake</i>	Type of intake	Surface - horizontal
	Number of gates	1
<i>Headrace Tunnel</i>	Length	26.6 km
	Diameter	8 m
	Cross-section shape	Circular (TBM)
	Cross-sectional area	50.3 m ²
<i>Powerhouse</i>	Width (upstream - downstream)	16.5 m
	Length	86 m
	Height	28 m
<i>Tailrace Tunnel</i>	Length	3.67 km
	Cross-section shape	Reverse-D
	Cross-sectional area	83.2 m ²
<i>Dams</i>	Type	Asphalt core rockfill
	Crest width	6 m
	<i>Dam 1 (Alternate axis)</i>	
	Crest elevation	719 m
	Length	330 m
	Maximum height	55 m
	<i>Dam 2</i>	
	Crest elevation	719.5 m
	Length	995 m
	Maximum height	27 m

<i>Spillway (overflow weir)</i>	Crest length	200 m
	Discharge capacity	2 440 m ³ /s
<i>Catchment Area</i>	Total area	6 789 km ²
<i>Access Road</i>	Total length	41.75 km
<i>Transmission Line</i>	Total length	125 km

1.2.1.2 Hydrology

The total area of the drainage basin at Site 7e is 6 789 km², of which 78% are glacier covered. Most of the inflow comes from glacier melting and occurs between June and October.

Daily flow series have been generated for a 50 year period. Three cases have been considered, producing the following set of data:

- historical series, using past climate data from September 1, 1958 to August 31, 2008;
- projected series 2020, using a climate warming scenario (from DMI) to produce an inflow projection at the year 2020;
- projected series with an horizon set at 2040 using a similar methodology to the 2020 run.

The historical synthetic series comes from an energy balance model and was calibrated on observed data.

Table 1.2 Yearly average flow at site 7e

Case	Yearly average (m ³ /s)
Historical	83.4
2020	96.1
2040	104.0

1.2.1.3 Power production

The net head is approximately 697 m (~2,287 ft) at full pool.

Based on the 2020 projection (or on the last 20 years), the predicted firm power capacity of Site 7e is approximately 500 MW, providing more than 70% of the planned smelter's total power requirement. The expected firm power for 2040 horizon would reach 536 MW. The powerhouse is equipped with 5 Pelton turbines which can produce a maximum output of 595 MW. The units were selected to provide a minimum of 505 MW over the complete reservoir fluctuation range with one unit shut down for maintenance either at Site 6g or 7e; this was done in order to guarantee the firm power availability.

1.2.1.4 Arctic conditions and permafrost

Since the field investigation results showed the presence of deep permafrost in the vicinity of the intake, a number of measures were taken to prevent freezing problems.

- The headrace canal approach velocity was set in order to ensure that a stable ice cover will rapidly form.
- The intake design includes a 200 m high drop shaft to bring the water below the 0 degree isotherm in the shortest possible distance; this is to avoid ice buildup in the tunnel in case of powerhouse shutdown.

- In order to avoid ice formation in the intake gate shaft, electric heating elements are inserted in tubes embedded in the wall of the gate over the full height of the gate. A 10.6 kV line along the access road is planned.
- It is planned to unfreeze and grout the dam foundations at the most critical section.
- The tailrace tunnel outlet was relocated downstream of a glacial tongue in the fjord to avoid ice obstruction problems.

1.2.1.5 Construction

The construction schedule spans five years. The critical activities are related to the access road and headworks construction. It takes a little less than 2 years to complete the access road to the dam area and a year is required to impound the reservoir, leaving only 2 years for headworks construction.

Two construction camps are required. The main one (Camp a) will be located near the powerhouse access tunnel entrance. The second one (Camp b) will be located in the dam and intake area. It is expected that construction of Site 7e will require 4.2 million man-hours to complete.

1.2.2 Site 6g

1.2.2.1 Description

The 6g hydropower project (Imarsuup Isua) is located approximately midway between the towns of Nuuk and Maniitsoq in the north-south direction, and approximately 120 km east of them. This high head scheme's main components and key figures are presented in the Site 6g project metrics table and include:

- The main reservoir, which will be created by raising Lake Imarsuaq's (Big lake) present water level (675 m) by 7 m. It's normal operating level will be between 669 m and 682 m with 945.8 hm³ of live storage;
- The lower reservoir, which will be created by raising Lake Tussapp Tasis' (Lower lake) present water level (653 m) by 14 m. It will be operated at a constant level (667 m);
- The headworks, with the dams, regulating structure and intake include:
 - Four asphalt core rockfill dams, with the largest one being approximately 32 m high;
 - Two concrete spillways, one for each reservoir;
 - Two temporary diversion tunnels and cofferdams;
 - A regulation tunnel connecting the two reservoirs;
 - 2 diversion canals to increase inflow to the Big Lake;
 - 2 canals to avoid ice build-up problems in shallow areas of the Lower lake
 - 46.7 km of access roads between the harbor and the various headworks components;
- The conveyance structure and power system
 - A 10 km headrace tunnel. At this stage of the study, the power tunnel is planned to be excavated by one TBM;
 - An underground powerhouse equipped with 2 Pelton turbines, a transformer cavern, access and cable galleries;
 - A tailrace tunnel whose outlet discharges at Godthabsfjord;
 - A service building and harbor facilities.

Table 1.3 6g project metrics

<i>Design Flood</i>	Permanent civil works BIG LAKE- 1:10 000 years flood	400 m ³ /s
	Temporary civil works BIG LAKE- 1:20 years flood	245 m ³ /s
	Permanent civil works LOWER LAKE- 1:10 000 years flood	160 m ³ /s
	Temporary civil works LOWER LAKE - 1:20 years flood	40 m ³ /s
<i>Water Levels</i>	<i>Reservoir-Big Lake</i>	
	Maximum operating level	682 m
	Minimum operating level	669 m
	Water level with the 1:10 000 years flood	683.8 m
	<i>Reservoir-Lower Lake</i>	
	Operating level (constant)	667 m
	Water level with the 1:10 000 years flood	668.3 m
	<i>Downstream – Fjord</i>	
	Maximum tide level	2.1 m
Minimum tide level	-3.5 m	
<i>Production Devices</i>	Number of turbines	2
	Type of turbines	Pelton
	Net head (at max level)	655.7 m
	Unit discharge	16.6 m ³ /s
	Maximum unit capacity	97 MW
	Voltage Output	13.8 kV
	Maximum generator output - total	194 MW
	Firm power	185 MW
<i>Headrace Canal</i>	Length	65 m
	Water velocity	0.65 m/s
<i>Intake</i>	Type of intake	Surface - horizontal
	Number of gates	1
<i>Headrace Tunnel</i>	Length	9.99 km
	Diameter	5.1 m
	Cross-section shape	Circular (TBM)
	Cross-sectional area	20.4 m ²
<i>Powerhouse</i>	Width (upstream - downstream)	15.3 m
	Height	28 m
	Length	29.5 m
<i>Tailrace Tunnel</i>	Length	1.1 km
	Cross-section shape	Reverse-D
	Cross-sectional area	39.4 m ²

<i>Tunnel 1</i>	<i>Connecting the two reservoirs - regulating structure</i>	
	Upstream and downstream canal length	170 m
	Tunnel length	1 690 m
	Cross-section shape	Reverse-D (Drill and Blast)
	Cross-sectional area	29.4 m ²
	Design discharge	40 m ³ /s
<i>Dams</i>	Type	Asphalt core rockfill
	Crest width	6 m
	<i>Dam 1</i>	
	Crest elevation	671.5 m
	Length	310 m
	Maximum height	31 m
	<i>Dam 2</i>	
	Crest elevation	671.5 m
	Length	290 m
	Maximum height	18 m
	<i>Dam 3 with geomembrane</i>	
	Crest elevation	685.5 m
	Length	560 m
	Maximum height	13 m
	<i>Dam 4</i>	
	Crest elevation	685.5 m
	Length	175 m
	Maximum height	21 m
	<i>Dam 5</i>	
	Crest elevation	685.5 m
Length	310 m	
Maximum height	32 m	
<i>Canals</i>	<i>Canal 1</i>	
	Length	190 m
	Bottom width	3 m
	Design discharge	40 m ³ /s
	<i>Canal 2</i>	
	Length	180 m
	Bottom width	3 m
	Design discharge	40 m ³ /s
	<i>Canal 3</i>	
	Length	675 m
	Bottom width	5 m
	Design discharge	28 m ³ /s
	<i>Canal 4</i>	
	Length	28 m
	Bottom width	10 m
	Design discharge	28 m ³ /s

<i>Spillways (overflow weir)</i>	<i>Spillway 1</i>	
	Crest length	50 m
	Discharge capacity	105 m ³ /s
	<i>Spillway 2</i>	
	Crest length	72 m
	Discharge capacity	305 m ³ /s
<i>Catchment Area</i>	Total area	1 375 km ²
<i>Access Roads</i>	Total length	46.6 km
<i>Transmission Line</i>	Total length	169 km

1.2.2.2 Hydrology

The total area of the drainage basin at Site 6g is 1 375 km², of which 58% are glacier covered. A large part of the inflow comes from glacier melting and occurs between June and October.

Daily flow series have been generated for a 50 year period. Three cases have been considered, producing the following set of data:

- historical series, using past climate data from September 1, 1958 to August 31, 2008;
- projected series 2020, using a climate warming scenario (from DMI) to produce an inflow projection at the year 2020;
- projected series with an horizon set at 2040 using a similar methodology to the 2020 run.

The historical synthetic series comes from an energy balance model and was calibrated on observed data.

Table 1.4 Yearly average flow at site 6g

Case	Yearly average (m ³ /s)
Historical	34.0
2020	37.4
2040	39.5

1.2.2.3 Power production

The net head is approximately 655 m (~2,148 ft) at full pool. The powerhouse is equipped with 2 Pelton turbines and the predicted firm power capacity of Site 6g is approximately 185 MW based on the 2020 projection.

1.2.2.4 Arctic conditions and permafrost

The field investigation results did not confirm the presence of permafrost at Site 6g. However, a number of measures were taken to prevent freezing problems.

- The canal's design water velocities were set in order to ensure that a stable ice cover will rapidly form;
- In order to avoid ice formation in the intake gate shaft, electric heating elements are inserted in tubes embedded in the wall of the gain of the gate over the full height of the gain. A 10.6 kV line along the access road is planned;
- It is planned to unfreeze and grout the dams foundation at the most critical section.

1.2.3 Construction

The construction schedule spans five years. The critical activities are related to the headworks construction. It takes 2 years to construct an access to the Northern dam area and a year is required to impound the reservoir, leaving less than 2 years (only one summer season) for the headworks construction.

Since the headworks of Site 6g are spread over a large territory, four construction camps are required:

- the main one (Camp 1) will be located near the Godthabsfjord and powerhouse access tunnel entrance;
- camp 2 will be located in the Lower lake dam and intake area;
- camp 3 will be located in the Southern end of Big lake near the tunnel connecting the two reservoirs;
- camp 4 will be located in the Northern end of Big Lake, in the vicinity of its present outlet.

It is expected that construction of Site 6g will require 2.3 million man-hours.

1.3 Project Enhancements

During the FEL 2 studies a number of project enhancements were achieved:

Hydrology

- Redefined catchment after inclusion of sub-ice contour and high resolution topographical data;
- Improved modeling approach after calibration of a new temperature model with meteorological measurement taken from stations operating on the Tasersiaq basin glacier margin;
- Flow series projection to consider climate change effect;
- Refined model and projection approach were reviewed and approved by glaciology experts.

Power generation

- Power production evaluation evolved from a 20 years series to a 50 years projected series taking into account all droughts and the natural variability;
- The firm power generator output was increased from 650 MW to 683 MW in order to take into account transmission losses and station service power to provide 650 MW of firm Power at the Smelter itself;
- Added overload capacity to the turbine-generator units to allow for one unit maintenance over the full range of reservoir level, without power reduction at the smelter;
- While the reservoir elevations were maintained to the FEL 1 levels, the potential to increase storage and power production at Site 7e exists and could be developed economically, although it could require extending the construction period to 6 years in order to fill the reservoir;
- Elimination of two diversion tunnels at Site 6g in the base scheme leaves potential for added power at 6g. This added power has a higher marginal cost than Site 7e's potential added power.

Arctic conditions, permafrost, site remoteness

- The design has been adjusted so that the arctic conditions and permafrost do not represent a fatal flaw;
- The Intake concept was adapted to the deep permafrost at Site 7e;
- Electrical lines and heating are planned at every gated intake and tunnel to ensure year-round gate operation;
- Gate redundancy at tunnel 1 (Site 6g) to guarantee continuous operation and room for maintenance of regulating equipment;
- Construction camp were adapted to the sites' remoteness and access conditions;
- Construction schedules now account for seasonal work and daylight periods.

Miscellaneous

- The Site 7e access road alignment in the power tunnel valley was further studied to give access up to the dam area and eliminate a 65 km portion between the second harbor in Sondre Stromfjord and the dam area that was planned in FEL 1.
- The cost of the redundant transmission line was reduced.
- The surge chamber/shaft was eliminated at Site 6g.

1.4 Project risks

The five main risks that were identified for the hydro are:

- Greater than anticipated infrastructure and logistics difficulties could increase costs and delays project start up ;
- Civil works construction difficulties could increase costs and delays (access road, tunneling, dam construction);
- Unfavorable weather conditions (change in duration of either winters or summers - movement of materials is easier during winter conditions -Fjord ice, fog, movement over snow or ice whereas construction is easier during summer conditions.);
- Difficulties could be encountered along the 300 km transmission lines to be constructed in rough terrain, with long fjord and glacier crossings. Some of them are state-of-the-art
- Environmental issues increase project cost, potentially impact start and completion dates/schedules and reduce available power output (NGO delays, Water releases downstream of dams, Ecosystems or archeological features in flooded areas or T-line corridor, project footprint).

1.5 Potential cost savings

During the FEL 2 studies and design review process, a number of potential alternatives or improvement to the design have been evaluated to reduce the overall project cost but were not actually incorporated in the drawings and base cost estimate. The potential savings (or cost increase) regarding these design modifications are included within the cost summary table for the project and are explained below for both Sites 6g and 7e.

1.5.1 Design modifications and purchasing costs

Headrace canal and intake design (7e): A new location for the intake allowed a reduction in the surface excavation volume by 60%, although slightly extending the headrace tunnel.

Headrace tunnel diameter (6g and 7e): Potential savings included in the cost summary reflect the reduction in headrace tunnel diameter that will likely be adopted after a further optimization study in this area.

Powerhouse location (6g and 7e): Following the results of 2009 field tests, the powerhouse location has to be moved for both sites, to avoid a sub vertical dolerite dyke at Site 7e and an area of low minimum stress level at Site 6g. These changes in the powerhouse location increase the project cost due to slightly longer access tunnels.

Penstocks and manifold optimization (6g and 7e): Potential savings included in the cost summary reflect the reduction in penstock diameter and length that will likely be adopted.

Dam axis (7e): The axis of dam 1 at Site 7e may be moved 200 m downstream to reduce its overall length and volume. At this site, the topography is favorable for the implementation of an ogee weir, unlined chute spillway.

Dam cross-section (6g and 7e): A number of adjustments to the dam cross-sections can be adopted to reduce their construction cost and ease the schedule.

Rock support (6g and 7e): Following the 2009 site investigations, the rock support criteria were reduced compared to those used in the base cost estimate.

Road construction (6g and 7e): The road construction methodology will likely be modified to increase the progression rate, improve the schedule and reduce the number of airlifts. A mobile camp is suggested to follow the road construction during the initial effort, Along with access at both ends of the steep sections, airlifts were proposed. A tunnel could also be excavated during the winter season in one of the section at Site 7e.

Diversion tunnel (6g and 7e): Potential savings included in the cost summary reflect the expected changes in diversion tunnel and cofferdam size after further optimization.

Concrete plugs (6g and 7e): The length and rebar quantities in the concrete plugs can be reduced slightly versus what was included in the base cost estimate.

Cable tunnel (6g and 7e): A new ventilation concept allows elimination of the concrete blocks in the middle of the tunnels, thus reducing the overall cross-section.

Equipment cost: A number of budget prices were received by a Danish supplier for the base cost estimate. Lower prices are expected from a North-American supplier even with addition of freight.

Construction camps: Salvage cost were applied to the construction camps permanent materials and infrastructure as well as temporary construction facilities, which wasn't done for the base cost estimate.

Fuel cost: was set by Alcoa at 0.66\$ USD per liter, which is lower than the 0.72\$ USD that was used in the base cost estimate.

All of the above potential savings and modifications were applied to the initial cost estimate to determine the overall saving that could be applied to the project. They're estimated at 75 M\$ for Site 7e and 53 M\$ for Site 6g

1.5.2 Working conditions and project contingencies

Additional savings are possible for the project, depending on the working conditions that are assumed, and the contingencies that are applied to the project. Alcoa suggested various criteria to consider in the cost estimate that are different from the parameters used in the base cost estimate, which roughly represent the actual practices in Canada. It is possible that the working conditions could be below the western countries standards if workers from other countries are employed for the project.

The criteria considered in the base cost estimate concerning the workers conditions compared with the new criteria proposed by Alcoa are the followings:

Table 1.5 Working conditions

	Initial cost estimate criteria	Revised criteria proposed by Alcoa
Hourly rate	24\$/hr	10\$/hr
Workers shift	40 days of work	120 days
Staff shift	40 days of work	60 days of work

Applying the new hourly rate to the cost estimate yields important cost savings on all project items. As for the longer work shifts, it reduced the cost of man power transportation to and from Greenland, as well as the number of overall trips.

The potential savings that can be obtained from the above considerations are:

New hourly rate of 10\$/hr: Alcoa suggested the use of a 10\$/h rate for Chinese labor. This change represents approximately 75 M\$ total for both sites, considering a productivity reduction of 25%

Reduced man-power transportation due to longer working shifts: approximately 40 M\$ total for both sites

Finally, Alcoa suggested to apply an overall contingency of 10% to the project total cost instead of the average contingency of 13% which was applied in the base cost estimate (contingencies varied between 10 and 25% depending on the item).

1.6 Opportunities

In addition to the potential savings outlined above, a number of potential opportunities could be further analysed and developed during 2010. They include:

- Construction of tunnel 2 and 3 at Site 6g which are not included in the base scheme and could increase the power output by 8 MW;
- 7F adjacent catchment could be diverted into 7e reservoir to increase the output of a single 7e scheme;
- Use of 2040 hydrology to plan future expansion or increased capacity;
- Staggered development of 7e and 6g to reduce 6g development cost.
- Other potential savings from the base cost estimate could arise with lower unit cost for the main component of the project, which include the equipments costs, the fuel cost, the labor rate and the man-power transportation.

The possibility to develop only Site 7e is one option that was studied in more depth. It is proposed that the maximum operating water level could be raised to 726 m to increase the firm power at the smelter to approximately 530 MW. The cost increase of implementing such a maximum water level would be of approximately 60 M\$ at Site 7e. This estimation considers the increase in the overall cost of camp operations, as it would likely require an extra working year relative to the proposed base schedule.

1.7 Project capital cost

The hydro project capital cost is presented in table 1.6. The difference with the cost presented in section 8 comes from the different contingency level.

Table 1.6 Project Costs by Site

Pos.	Item	Site 7e	Site 6g	Cost (M\$ USD)	Potential savings Site 7e	Potential Savings Site 6g	Cost with potential savings (M\$ USD)
1. CIVIL WORKS							
1.1	DAMS	37.8	29.0	66.9	-6.3	-3.2	57.3
1.2	TUNNELS						
1.2.1	Headrace Tunnel	137.5	43.8	181.3	-11.4	-6.8	163.1
1.2.2	Tailrace Tunnel	16.0	7.4	23.4	-1.6	-0.7	21.0
1.2.3	All other tunnels	4.4	19.0	23.4	-3.7	-3.8	15.9
1.3	CANALS		2.5	2.5		-0.1	2.5
1.4	INTAKE STRUCTURE	59.0	9.2	68.2	-25.1	-0.2	42.9
1.5	UNDERGROUND POWER STATION	34.5	32.4	67.0	-1.4	7.1	72.7
2. MECHANICAL AND ELECTRICAL EQUIPMENT							
2.1	GENERATING EQUIPMENT	121.5	49.9	171.4	-2.2	-1.0	168.2
2.2	AUXILIARY MECHANICAL EQUIPMENTS	11.1	13.5	24.6	-0.2	-0.3	24.2
2.3	ELECTRICAL EQUIPMENT	37.6	30.5	68.1	-0.7	-0.6	66.8
3. INFRASTRUCTURE							
3.1	HARBORS	5.7	8.8	14.5	-0.1	-0.2	14.3
3.2	ROADS	58.0	50.2	108.2	-3.6	-3.5	101.1
3.3	CONSTRUCTION CAMPS						
3.4.1	Construction	56.3	117.4	173.7	-15.7	-37.2	120.8
3.4.2	Operation and maintenance	50.9	35.1	86.0	-0.9	-0.7	84.3
3.5	Construction material transportation	25.1	16.6	41.7	-0.5	-0.3	40.9
DIRECT COSTS TOTAL		656	465	1 121	-74	-51	996
4. INDIRECT COSTS							
4.1	Construction services and temporary facilities	32.7	39.6	72.3	-0.6	-1.1	70.6
4.2	Travel cost	41.0	25.8	66.8	-26.8	-16.4	23.6
4.6	Insurance	25.9	24.9	50.8			50.8
4.8	EPCM (Home office)	12.3	5.8	18.2			18.2
4.9	EPCM (Field office)	54.2	45.8	100.0			100.0
INDIRECT COSTS TOTAL		166	142	308	-27.4	-17.5	263
5. TRANSMISSION LINE							
5.1	Transmission line	93.9	121.0	214.9	-5.0	-5.0	204.9
5.2	Substations	21.6	18.4	40.0			40.0
TRANSMISSION LINE TOTAL		115	139	255	-5.0	-5.0	245
SUB-TOTAL		937.0	746.6	1 684	-105.9	-73.9	1 504
TOTAL (with 10% contingency)		1 031	821	1 852			1 654
	Hydro Plant Output (MW)	500	185	685			685
	M\$/MW	2.06	4.44	2.70			2.41

N-1 TRANSMISSION LINE (ADDED COST)							
	Transmission Line	64.0	76.1	140.0			140.0
	Substations	2.7	3.3	6.0			6.0
	N-1 Transmission Line Total (added cost)	66.7	79.3	146.1			146.1
	TOTAL (with contingency)	1 104	909	2 013			1 815
	TOTAL (M\$/MW)	2.21	4.91	2.94			2.65

The single site 7e option capital cost is presented in Table 1.7.

Table 1.7 Single Site 7e Cost

Pos.	Item	Single Site 7e Cost (M\$ USD)
<i>1. Civil works</i>		
1.1	Dams	40.0
1.2	Tunnels	
1.2.1	Headrace Tunnel	137.5
1.2.2	Tailrace Tunnel	14.3
1.2.3	All other tunnels	0.7
1.3	Canals	
1.4	Intake structure	36.9
1.5	Underground power station	33.1
<i>2. Mechanical and electrical equipment</i>		
2.1	Generating equipment	132.5
2.2	Auxiliary mechanical equipments	10.9
2.3	Electrical equipment	36.9
<i>3. Infrastructure</i>		
3.1	Harbors	5.6
3.2	Roads	54.4
3.3	Construction camps	
3.4.1	Construction	40.6
3.4.2	Operation and maintenance	57.5
3.5	Construction material transportation	26.4
<i>Direct costs total</i>		<i>627</i>
<i>4. Indirect costs</i>		
4.1	Construction services and temporary facilities	32.1
4.2	Travel cost	15.7
4.6	Insurance	27.8
4.8	EPCM (Home office)	13.2
4.9	EPCM (Field office)	58.1
<i>Indirect costs total</i>		<i>147</i>
<i>5. transmission line</i>		
5.1	Transmission line	130.1
5.2	Substations	15.9
<i>Transmission line total</i>		<i>146</i>
<i>Sub-total</i>		<i>920</i>
<i>TOTAL (with 10% contingency)</i>		<i>1 012</i>
Hydro Plant Output (MW)		535
<i>M\$/MW</i>		<i>1.89</i>
<i>N-1 transmission line (added cost)</i>		
<i>Transmission Line</i>		<i>90.4</i>
<i>Substations</i>		<i>3.2</i>
<i>N-1 Transmission Line Total (added cost)</i>		<i>102.9</i>
Total		1 115
Total (M\$/MW)		2.08

Table 1.8 shows a summary of the project Capex under two working conditions assumptions.

Table 1.8 Summary of the expected hydro project Capex

Case #	Description	Power capacity ⁽¹⁾	Hydro Only Cost	Million's ⁽²⁾ With Base Case T-Line (N)	With Redundant T-Line (N-1)
1	7e & 6g Total – Base Case (w/Contingencies) (\$24/hr labor)	650	1 384	1 654	1 815
2	7e & 6g Total – Base Case (w/Contingencies) (\$10/hr labor)	650	1 304	1 574	1 735
3	Site 7e only – Single Hydro (w/Contingencies) (\$24/hr labor)	520	851	1 012	1 115
4	Site 7e only – Single Hydro (w/Contingencies) (\$10/hr labor)	520	797	958	1 061

Notes:

- 1) Firm power at the Smelter
- 2) All cases include 10% contingency
- 3) Labor based on a 4 month on/2 week off work schedule
- 4) Single hydro options are based on a 12m increase in dam height relative to the base case
- 5) Single hydro options extend construction schedule to 6 years due to increased fill time

2 Site description

2.1 Topography

Greenland's topography has a general bowl shape with peripheral mountainous areas surrounding a central basin that extends below the sea level. The Greenland Ice Sheet occupies the central bowl, covers much of the fringing mountains, and in places pushes to the coast where it calves into the sea. Ice-free regions at the fringes of the ice sheet are in most areas mountainous, cut by fjords and contain scattered thin deposits of till and local thick deposits of Quaternary nonglacial sediments of a variety of different ages. The surface elevations of the ice sheet are shown in Figure 2.1.

The general features are a southern and a northern dome with maximum elevations of 2 830 and 3 205 m respectively connected by a long almost horizontal saddle with elevations around 2 500 m.

The main drainage divide runs north-south near the eastern ice margin, living large parts of the Inland Ice to flow towards the west, while only smaller sectors drain eastwards. A major drainage outlet from the Inland Ice is located on the west coast in the Disko Bugt area.

The dominant landscape type in West Greenland is hilly up land composed of rounded knolls of crystalline bedrock at elevations between 300 and 1 500 meters.

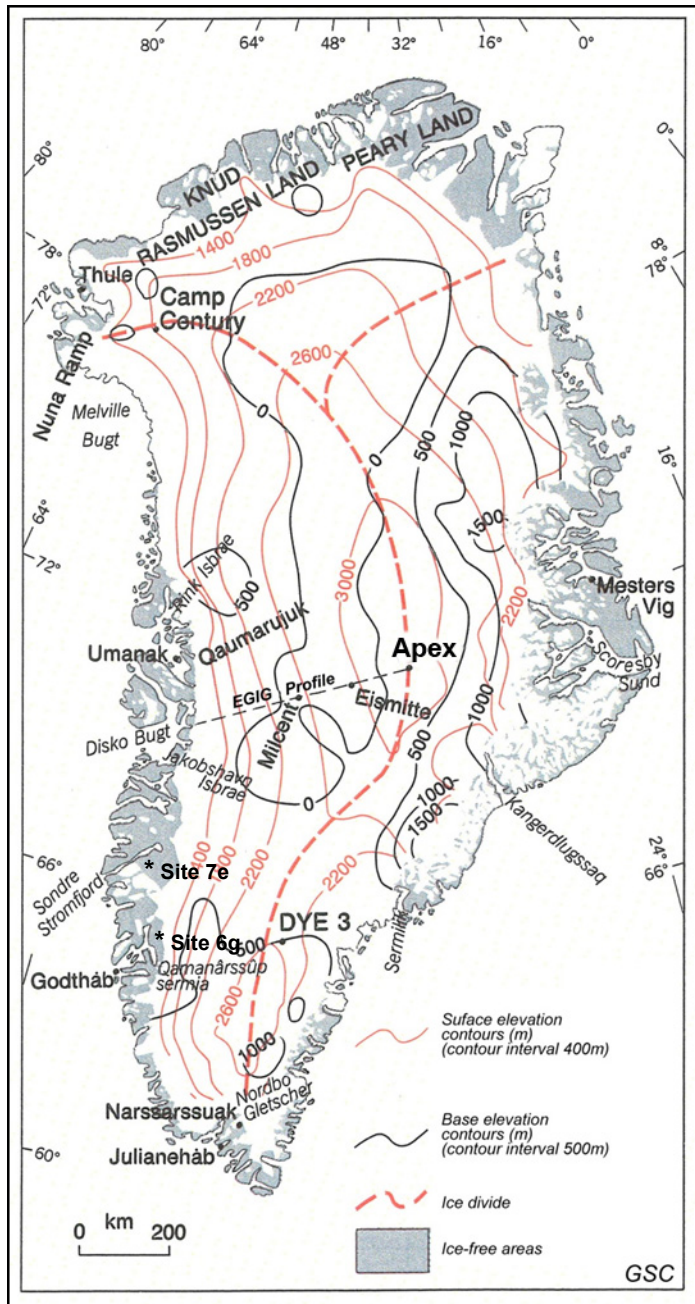
2.2 General geology and seismicity

The geological history of Greenland spans more than 3.8 billion years (Vilumsen et al., 2007). The basement consists largely of composite gneisses that were formed more than 1.6 billion years ago. About 60-55 million years ago, there was widespread volcanic activity in Greenland and extensive volcanic provinces developed. The last major event was the Ice Age over the last two million years, when most of Greenland was covered by ice. All of West Greenland – excluding only some high mountains near the coast – was covered by the Inland Ice during the Sisimiut glaciations of Late Wisconsinan age.

At site 6g, rock is predominantly amphibolites gneiss, although iron-rich rock, fine-grained schist and metasediments were also reported by PB Power (2009). Geologic mapping from 2007, 2008, and 2009 investigations generally agreed with the historic geologic maps, with the exception of some general foliation trends.

Recent glaciations are marked by erosion rather than sediment accumulation in most areas. Thick Quaternary deposits are of restricted occurrence and are generally confined to major valleys and lowlands along the coasts. The Quaternary geology of West Greenland is described in detail by Fulton (1989). The following is a brief summary.

Figure 2.1 Surface and base elevations (m) and main ice divides of the Greenland Ice Sheet



(Reet. N., 1989)

The most widespread glacial deposits are patches of loose gravelly and sandy diamicton and scattered erratic boulders considered to be melt-out till. These materials form a continuous cover in the interior near the Inland Ice margin. Thicker deposits of melt-out till occur in lateral and terminal moraines. Moraines dating from Holocene deglaciation stages occur in all parts of the area, and in their general distribution follow that outlined for till deposits. Moraines generally have developed only along active sectors of the ice margin – lobes and outlet glaciers – while the regionally more extensive passive sectors have created few moraines. The location of moraines along the fjords – at fjord junctions and bends, and at places where the sides change from steep to gentle slopes – suggests that the moraines commonly were formed as an interaction between the glacier and the topography of its bed, rather than in response to climatic change.

Glaciofluvial and fluvial sediments cover the floors of all major valleys, occurring as outwash plains and fluvial terraces deposited from braided rivers. The most extensive plains occur in the valleys between the Inland Ice margin and the heads of fjords. Glaciofluvial sand and gravel also occur as kame terraces along valley sides. Eskers forming from subglacial meltwater are not common.

Marine sediments ranging from coarse littoral gravel to massive or laminated silt are widespread in the coastal areas, occurring up to 140 m above present sea level – the maximum elevation of the Holocene marine limit.

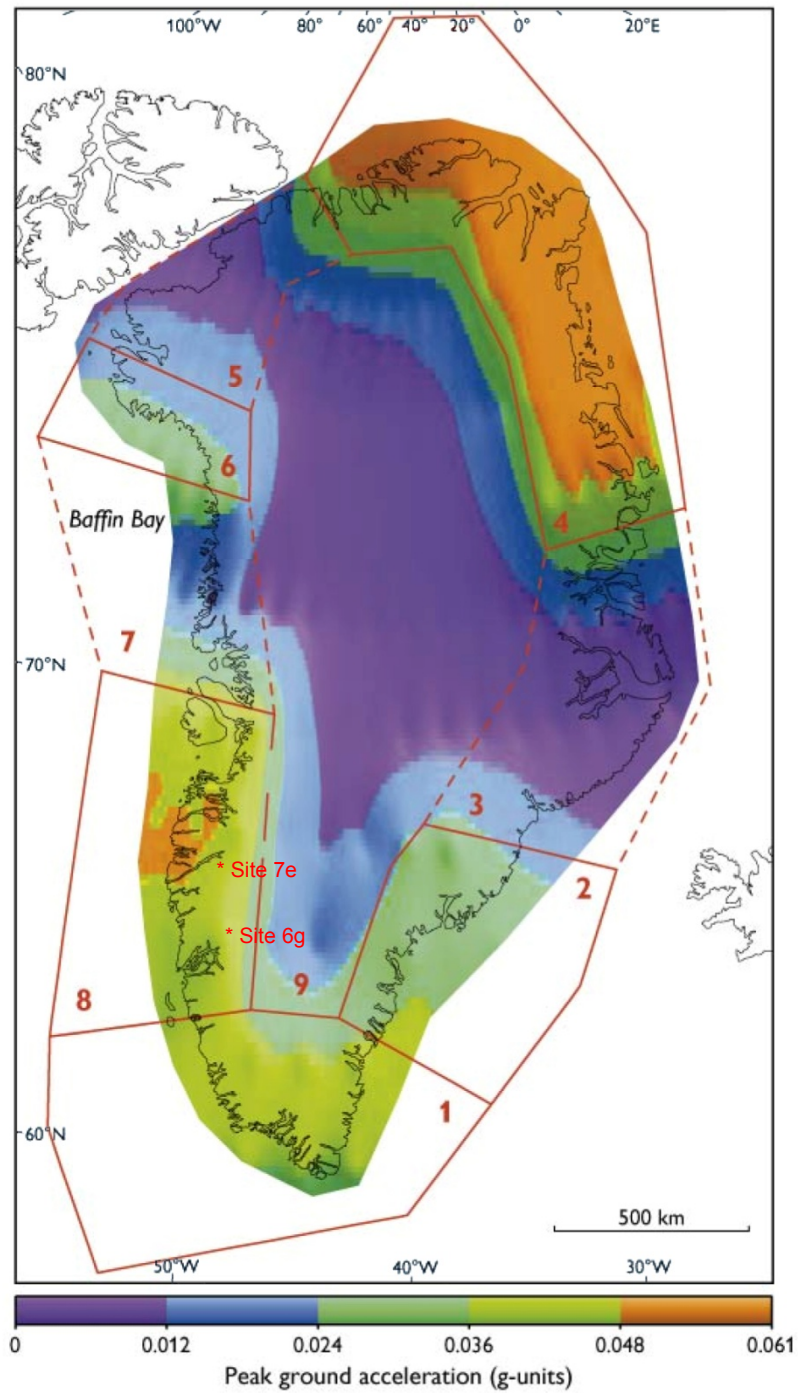
Earthquake activity in Greenland has been registered and mapped since 1907 and thus a long (albeit relatively sparse) record of seismic activity is available for evaluation of seismic hazard and risk.

The seismic hazard assessment of Greenland is computed for a return period of 475 years as shown in Figure 2.2. The maximum hazard is found in seismic source zone 4 at a value of 0.051 g (50.37 cm/s²). From this result, the general seismic hazard in Greenland is considered to be low, following the classification of Jiménez et al. (2003) in which low, moderate and high hazards correspond to peak ground accelerations of 0.0-0.08 g, 0.08-0.24 g and above 0.24 g, respectively, for a 475-year return period (GEUS, 2007 – Geological survey of Denmark and Greenland, bulletin 13, 57-60).

Seismic source zone 4 covering the northern and north-eastern parts of Greenland is the area with the highest seismic hazard; the seismic hazard is below 0.05 g in the other seismic source zones where the highest hazards are encountered in the Disko Bugt – Sisimiut area (seismic source zone 8) followed by southern Greenland (seismic source zone 1).

Site 6g is located in zone 8. Thus, the peak ground acceleration is 0.048 g for a 475-year return period.

Figure 2.2 Seismic hazard in Greenland for a 475-year return period



2.3 Site investigations and local geology

2.3.1 Previous investigations

In 2007, during phase I of the Memorandum of Understanding (MOU), PB Power completed various field studies and technical analyses as part of an initial feasibility study including Phase I of the geological and hydrological investigations. Specifically, the geotechnical investigations consisted of preliminary geologic mapping of proposed civil works alignments and preliminary borrow source assessments as well as aerial and topographic surveys.

Additional investigations were carried out in 2008 during MOU, Phase II. This investigation included geologic mapping in addition to borehole drilling. Two boreholes were drilled and thermistor strings were installed for permafrost characterization. Five samples of rock core were recovered and tested for a variety of laboratory tests.

Electromagnetic (EM) and seismic refraction surveys were carried out along proposed dam, canal and borrow areas. Only overland profiles were obtained; no profiles were obtained beneath major streams and rivers. The geophysical data were processed and interpreted with calibration to observed rock outcrops, but not against geotechnical boreholes.

A summary of the 2007/2008 geotechnical investigations and the main findings are presented on Table 2.1.

Table 2.1

Site 6g – Existing investigations – Main geological and geotechnical characteristics – Summary

Location	Existing investigations	Rock properties	Soil properties and remarks
Power Tunnel (2007/2008 axis)	<p>Aerial photography</p> <p>Topographic maps (2 m contours)</p> <p>Mapping</p> <p>Two boreholes : GPT-1 (Inlet) GPT-2 (outlet)</p> <p>Seismic survey: Inlet (PT-In) Outlet (PT-Out)</p>	<p>Inlet</p> <ul style="list-style-type: none"> Slightly to moderately weathered gneiss with few sets of discontinuities. Foliation striking NE/SW dipping SE. First recovery at borehole GPT-1 at 7.2 m due to casing and fractured rock. Rock covers more than 70 % of the surface RMR Classification: average value of 78 (Class II, Good) Geo mechanical properties (2 samples) <ul style="list-style-type: none"> Uniaxial compressive strength: 112 and 163 MPa Young's modulus: 33 and 60 GPa Poisson's ratio: 0.17 and 0.28 Indirect (Brazilian) tensile strength: 6.8 and 9.6 MPa Cerchar abrasivity index: 5.8 and 4.9 Slake durability: 99.6 to 99.4% Petrographic analysis: quartzo-feldspathic gneiss and biotite-amphibole gneiss <p>Along the length</p> <ul style="list-style-type: none"> Slightly weathered amphibolite gneiss, jointing and foliation changes very frequently RMR classification : average value of 81.8 (Class I, Very Good) <p>Outlet</p> <ul style="list-style-type: none"> Slightly weathered gneiss with few sets of discontinuities. Foliation striking NE/SW, dipping NW Rock covers more than 70% of the surface RMR classification: average value of 70.2 (Class II, Good) Geo mechanical properties (3 samples) <ul style="list-style-type: none"> Uniaxial compressive strength: 176 MPa (2 samples) and 243 MPa (1 sample) Young's modulus: 27 to 40 GPa Poisson's ratio: 0.04 to 0.16 Indirect (Brazilian) tensile strength: 5.2 to 10.1 MPa Cerchar abrasivity index: 5.6 to 6.3 Slake durability: 99.3 to 99.4% (2 samples) Petrographic analysis: quartzo-feldspathic gneiss 	<p>Glacial Till</p> <p>No overburden in the inlet area (borehole GPT-1)</p> <p>Thin soil covers valleys between the gneissic rock hills</p> <p>Glacial Till</p>
Tunnel I	<p>Aerial photography</p> <p>Topographic maps (2 m contours)</p> <p>Mapping</p> <p>Seismic survey: Inlet: T3-In Outlet: T3-Out</p>	<ul style="list-style-type: none"> Gneissic rocks. Foliation strikes east-west in the northern area, north-south in the south. Some dikes, possibly doleritic In the inlet area, rock covers more than 60% of the surface, in the outlet area, rock outcrops cover approximately 25% of the surface RMR classification: average values varie from 71.9 (inlet) to 79.8 (Class II, Good) 	<p>Glacial Till</p>
Spillway1	<p>Aerial photography</p> <p>Topographic maps (2 m contours)</p> <p>Mapping</p> <p>Seismic survey – 4 lines (G90, G90-A, G90-B, G-92)</p>	<ul style="list-style-type: none"> Gray gneiss with consistently spaced joints, foliation striking NW/SE, dipping steeply to the SW. RMR classification: average values of 62.2 (left abutment), 64.7 (middle) and 73.2 (right abutment) (Class II, Good). 	<p>Very little soil (glacial till) cover along the spillway</p> <p>Overburden less than 2.5 m</p>
Dam I	<p>Aerial photography</p> <p>Topographic maps (2 m contours)</p> <p>Mapping</p> <p>Seismic survey – 4 lines (G21 to G24)</p>	<ul style="list-style-type: none"> Distinctly banded granitic gneiss. Foliation strikes east-west, dips 45° south. Bedrock mostly exposed (over 80% of the entire dam surface), slightly weathered iron rich rock present at several locations on the left abutment. RMR classification (left and right abutments): average values of 75.2 and 70.4 (Class II, Good) 	<p>Glacial Till</p> <p>Overburden less than 3 m</p>

Table 2.1

Site 6g – Existing investigations – Main geological and geotechnical characteristics – Summary

Location	Existing investigations	Rock properties	Soil properties and remarks
Dam 2	Aerial photography Topographic maps (2 m contours) Mapping Seismic survey – 7 lines (G30, G32 to G35)	<ul style="list-style-type: none"> • Distinctly banded granitic gneisses. Foliation strikes east-west, dips 45° south. • Bedrock mostly exposed (over 85% of the surface) • RMR classification: <ul style="list-style-type: none"> - Left abutment in the granitic gneiss: 25.5 (Class IV, Poor) - Left abutment in the gneiss: average values of 77.3 (Class II, Good) - Right abutment: average value of 64.8 (Class II, Good) 	Glacial Till Overburden less than 6 m
Dam 3	Aerial photography Topographic maps (2 m contours) Mapping Seismic survey – 7 lines (G40, G41, G43, G44)	<ul style="list-style-type: none"> • Gneissic and granitic rock, inclusions of basic rock. • Sound rock covered by big boulders • At left abutment, presence of 2 m wide zone of dark gray to black banding amphibolite gneiss • RMR classification: <ul style="list-style-type: none"> - Left and right abutments: 71.4 and 71.8 (Class II, Good) - Middle: 67.8 (Class II, Good) 	Glacial Till Boulders Based on seismic survey, the depth of the overburden reaches locally more than 4 m
Dam 4 and Spillway 2	Aerial photography Topographic maps (2 m contours) Mapping Seismic survey – 9 lines (G11, G13 to G16)	<ul style="list-style-type: none"> • Mainly greenschist and metasedimentary (sandstone and siltstone) rock. Rock covers more than 2/3 of the surface along the entire dam. • RMR classification (left and right abutments): average values of 62.2 and 59 (respectively Class II, Good and Class III, Fair) 	Glacial Till Overburden less than 2.5 m
Dam 5	Aerial photography Topographic maps (2 m contours) Mapping Seismic survey – 5 lines (G1W, G1, G1E, G5, G6)	<ul style="list-style-type: none"> • Mainly greenschist and metasedimentary (sandstone and siltstone) rock with inferior quality compared to gneiss (left and right abutment). • 2/3 of the surface along the dam is covered by rock outcrops • RMR classification (left and right abutments): average values of 60.1 and 61.3 (Class II, Good). 	Glacial Till Overburden less than 2.5 m
Canal 1	Aerial photography Topographic maps (2 m contours) Mapping	<ul style="list-style-type: none"> • Slightly weathered gneiss • RMR classification: 80.8 (Class I, Very Good) 	Glacial till Boulders
Canal 2	Aerial photography Topographic maps (2 m contours) Mapping	<ul style="list-style-type: none"> • Slightly weathered gneiss • RMR classification: average value 76.6 (Class II, Good) 	Limited soil cover
Canal 3	Aerial photography Topographic maps (2 m contours) Mapping Seismic survey – 3 lines (North, Middle, South)	<ul style="list-style-type: none"> • West side of canal: dark brown to dark gray metasediment, bedding strikes parallel to canal with few sets of discontinuities • East side of canal: Iron-rich rock. Almost half of the surface is covered by rock outcrops • RMR classification : 80 (Class II, Good) 	Glacial Till Boulders Overburden less than 3 m
Canal 4	Aerial photography Topographic maps (2 m contours) Mapping	<ul style="list-style-type: none"> • Slightly to moderately weathered metasediments, foliation striking NE/SW, dipping steeply NW. Rock along both shores and at falls • RMR classification: average value 70.2 (Class II, Good) 	Glacial Till Boulders (Overburden less than 3 m)

2.3.2 2009 Field investigation¹

An additional geotechnical investigation program was conducted in 2009. The main objective was to gather further data required for the completion of the Phase II engineering study.

Table 2.2 presents the nature and the scope of the 2009 geotechnical investigation. It comprises one deep borehole in the vicinity of the powerhouse, and a total of 10 boreholes at the dam sites. Apart from gathering information of rock characteristics and conducting in situ testing of the rock, the deep borehole in the vicinity of the powerhouse allowed the installation of a thermistor string for the permafrost characterization.

The determination of the stress levels in rock formations will allow to validate the powerhouse location and orientation, rock reinforcement and also the length of the penstock steel liner upstream of the powerhouse.

Boreholes at dam areas will allow to determine the overburden thickness and the rock characteristics of the foundations. Thermistor strings were installed in one of the boreholes in Dam 5 and in the borehole at Tunnel 1. These data will allow to validate the decisions made with regard to foundation treatment and typical cross sections.

2.3.3 Local geology

A summary of the main geotechnical features for each of the civil works as to the layouts studied during MOU Phase I is presented in Table 2.1. Since the beginning of the MOU Phase II engineering study in January 2009, some changes in the previous layouts were made and in some cases the civil works were completely or partially relocated. Therefore, the aim of the 2009 geotechnical investigation was to obtain information at these new sites and to confirm or complete existing data.

2.3.3.1 Powerhouse and power tunnel

Compared to the former power tunnel axis, the inlet and outlet remain at the same locations, while the powerhouse complex was moved some 2 km downstream. Beside the two existing boreholes from the previous investigation (borehole GPT-1 in the inlet area and borehole GPT-2 in the outlet area), one deep borehole in the vicinity of the powerhouse, as part of the 2009 investigation program has been completed. Jacking tests were performed in this last borehole to determine stress levels in rock. Piezometer and thermistor strings were installed to determine permeability and permafrost conditions.

Following the analysis of the results obtained from the hydraulic jacking tests, it was concluded that the minimum stress levels in the rock formations are rather low compared to the Norwegian recommendations for the design of underground hydroelectric works. Therefore, in accordance with the topography, it was decided to move the powerhouse complex approximately 500 m away from the fjord. At this location the rock cover is 555 m. This new location has to be investigated.

Available data from the previous geological mapping and boreholes GPT-1 and GPT-2 show that rock formations consist mainly of slightly to moderately weathered gneisses

¹ Only partial and preliminary 2009 investigation results were available for the production of this report. Consequently, they are not fully integrated into the report. The main outcome of the 2009 investigations is the displacement of the powerhouse as described in section 12.

(quartzo-feldspathic and biotite-amphibolite gneisses). The foliation strikes NE/SW, dipping shallowly or moderately (less than 45 degrees) SE in the inlet and NW in the outlet areas. In both areas, the rock quality is Good (Class II, RMR classification). The average Mohs Hardness of the quartzo-feldspathic gneiss samples range from H=5.82 to 6.26, with lower values attributed to rocks with a higher content of biotite (H= 2.5-3).

Table 2.2 Site 6g – Greenland Hydropower – Early engineering, Phase II – 2009 Investigation Program

Location	Borehole		Testing			
	Number	Total length (m)	Permafrost	Piezometer	Hydrojacking test	Acoustic Survey
Powerhouse	1 ⁽¹⁾	452	X	X	X	X
Power tunnel intake	---	---	---	---	---	---
Tailrace tunnel	---	---	---	---	---	---
Spillway 1	1	9.6	---	---	---	---
Dam 1	2	27.5	---	---	---	---
Dam 2	2	21	---	---	---	---
Dam 3	1	15	---	---	---	---
Dam 4 and Spillway 2	1	15	---	---	---	---
Dam 5	2	30	X	---	---	---
Tunnel 1	1	51	X	---	---	---
<i>Total</i>	<i>11</i>	<i>621.1</i>	<i>3</i>	<i>1</i>	<i>1</i>	<i>1</i>

⁽¹⁾ Borehole at 30° angle from vertical

The geomechanical tests performed during Phase I on 2 rock cores from borehole GPT-1 (see Table 2.1) show that rock properties are typical of those of metamorphic (gneissic) rocks. The uniaxial compressive strength is 112 and 163 MPa qualifying the rock formations as Very Strong (Class R4). The rock has very high slake durability (>99%). In borehole GPT-2, 3 samples were analyzed for Uniaxial Compressive Strength and the values are rather high (175 MPa for 2 samples and 243 MPa for one sample).

2.3.3.2 Spillway 1

Bedrock consisting of gray gneiss of Good quality is mostly exposed on the surface (more than 85%). Most of the joints are aligned with the foliation, striking NW/SE, dipping steeply (more than 45°) SW. Overburden, when present, composed of glacial till, is less than 2,5 m deep.

2.3.3.3 Dams 1 and 2

In both site locations, most of the bedrock is also exposed on the surface. Overburden, when present, is less than 3 m deep. Most of the rock outcrops consist of distinctly banded granitic gneiss. In both sites, at least one of the joint sets is aligned with the

foliation, striking EW, dipping 45 degrees south. Iron rich rock is present at several locations on the left abutment of Dam 1. Although in most places, the quality of rock was classified as Good (Class II), poor quality rock with an average RMR value of 25.5 (Class IV) was identified at the left abutment of Dam 2.

2.3.3.4 Dam 3

Based on the seismic survey, the depth of the overburden (glacial till), reaches locally 4.5 meters.

Sound rock, consisting of gneiss and granite with inclusions of basic rock is at many places, covered by big boulders. A two meters wide zone of dark gray to black banding amphibolite gneiss was observed at the left abutment of the dam. Overall quality of rock is Good (Class II).

2.3.3.5 Dam 4 and Spillway 2

Rock outcrops consisting mainly of gneiss cover close to 2/3 of the surface along the dam. Greenschists of inferior quality compared o gneissic rock are present at both abutments. Rock was classified as Fair at right abutment, and as Good at left abutment. Overburden (glacial till) is less than 3 m deep.

2.3.3.6 Dam 5

Geological conditions at this site are very similar to those of Dam 4. Overburden is less than 2.5 m deep. Most of the surface is covered by rock outcrops consisting mainly of greenschist.

2.3.3.7 Canals

Geological conditions at all canals are relatively similar. There is limited soil cover and the overburden (glacial till) is less than 3 m deep. Boulders, some reaching 3 m in diameter, are visible randomly on the surface at Canals 1, 3 and 4, and are generally not nested. At Canals 1 and 2, the rock consist mainly of slightly weathered gneiss of Good and Very Good quality. At Canals 3 and 4, the rock is mainly metasedimentary (sandstone and siltstone). An iron rich rock formation is present at the west side of the Canal 3. At the west site, bedding strikes parallel to the canal.

2.3.3.8 Tunnel 1

In the inlet area, rock outcrops cover more than 60% of the surface; while in the outlet area rock outcrops cover approximately 25% of the surface. Rock is mainly gneissic and is of Good (Class II) quality. In the northern area of the tunnel, foliation strikes east-west, while in the south area, the foliation strikes north-south.

2.3.3.9 Construction materials

2.3.3.9.1 Rockfill

Rockfill is the main material needed for the construction of the dams and for the production of aggregates for concrete. Most of the rockfill will come from the planned excavations. Moreover, rock outcrops for quarries are present at all sites and in the vicinity of the dam sites. Preferably, quarries for the dams should be located immediately upstream of each dam, within the future reservoir areas for environmental reasons.

Representative rock samples should be collected and sent to the laboratory for standard testing as to ensure that the excavated rock meets the requirements for concrete production. Although concrete out of granitic and gneissic rock usually fulfills the requirements for a good aggregate, some essential tests such as alkali-aggregate reactivity and resistance of unconfined coarse-aggregate to freezing and thawing should be performed.

2.3.3.9.2 Till and other granular material

Potential borrow sources consisting of glacial soils in the vicinity of the dams and canal locations were identified during the 2007/2008 investigations. Although the thickness of the overburden seems rather limited (in most cases less than 3 m with exception of Dam 3 site), there are few areas covered with soil along the dam alignments with occasional rock outcrops protruding the surface, in Canal 1 area, and in the power tunnel outlet area. Glacial moraines (till) are essentially needed for the construction of the cofferdams. Based on 2007/2008 field observations, these moraines are generally a widely graded material consisting of silt, sand, gravel and stones (cobbles and boulders). The stones are hard and durable and usually sub-angular or sub-rounded. Boulders reach up to 3 meters in diameter and are spaced randomly on the surface and are generally not nested.

River terrace deposits consisting of granular material such as sand, gravel and cobbles were observed some 1.5 km east of the tailrace discharge to Godthalsfjord, and in the deposits upstream of the Spillway 1. These deposits being poorly graded could be suitable for the supply of fine aggregate for concrete. Areas of interest were identified and investigated by auger sounding during the 2009 investigations. As the information concerning the acceptability of these deposits for the production of fine concrete aggregate is not yet available, it is assumed at this stage that fine aggregates will be processed from blasted rock.

Table 2.3 shows the quantities of rockfill (random rockfill, crushed stone and riprap) and till required for the construction of the dams and cofferdams.

Table 2.3 Site 6g – Borrow material – Quantities required for dam construction

Site	Random rockfill ⁽¹⁾	Rockfill (m ³)		Till (m ³)	Total (m ³)
		Crushed stone ⁽²⁾ (all sizes)	Riprap		
Dam 1	60 900	40 000	7 500	3 000	<i>111 400</i>
Dam 2	31 200	37 200	10 000	---	<i>78 400</i>
Dam 3	77 000	33 500	9 500	---	<i>120 000</i>
Dam 4	35 600	20 650	1 400	2 000	<i>59 650</i>
Dam 5 ⁽³⁾	81 500	42 950	5 200	---	<i>129 650</i>
<i>Total</i>	<i>286 200</i>	<i>174 300</i>	<i>33 600</i>	<i>5 000</i>	<i>499 100</i>

(1) Including a small volume of 0-450 mm selected rockfill for Dams 1 and 2

(2) Including asphaltic concrete core aggregate

(3) Including the cofferdam at canal 4

2.4 Climate

Along West Greenland, a 3-4 m thick sea ice (called the west ice) covers most of Baffin Bay during the winter from the Polar Sea to approximately Sisimut. In summer the ice situation in the same waters is influenced by icebergs from the West Greenland Glaciers, mainly the Ilulissat Glacier. Varying quantities of west ice is brought along with the Labrador Sea Current down along the Canadian east coast; only a small proportion of the west ice remains during the summer. The climate in Greenland is an Arctic climate. The climate in Greenland varies considerably, even over short distances. The katabatic wind system of the Greenland Ice Cap results in wind moving from the central portion of the ice cap towards the coast. At the proposed project site, wind is predominantly from the east or southeast.

Along West Greenland, air temperatures vary between the coast and inland. Along the coast, drifting ice or cold water makes the air cold and humid. Further inland, the weather is often warmer and sunny. Differences of up to 5°C have been reported. Temperatures also vary according to altitude. While normally air temperatures decrease with altitude by 6.5°C per kilometer, in the Arctic, the change in temperature is smaller, owing to temperature inversions. One result of such inversions is that spring snow starts melting in the mountains rather than at sea level.

The amount of precipitation is generally higher along the coast than inland. Snow cover is also higher along the coast than inland. Many of the country's long-term meteorological stations operated by the Danish Meteorological Institute (DMI) are situated in Greenland's major population centres, which are mostly located on the coast. In the vicinity of the project area, meteorological stations have been established and maintained by ASIAQ for prior studies. Although the period of record for these stations is generally shorter and frequently with data gaps, these data are useful in better-understanding the climate of the project area. Figure 2.3 shows the locations of the DMI long-term meteorological stations, along with the locations of the nearby ASIAQ stations.

Figure 2.3 Locations of weather stations, West Greenland

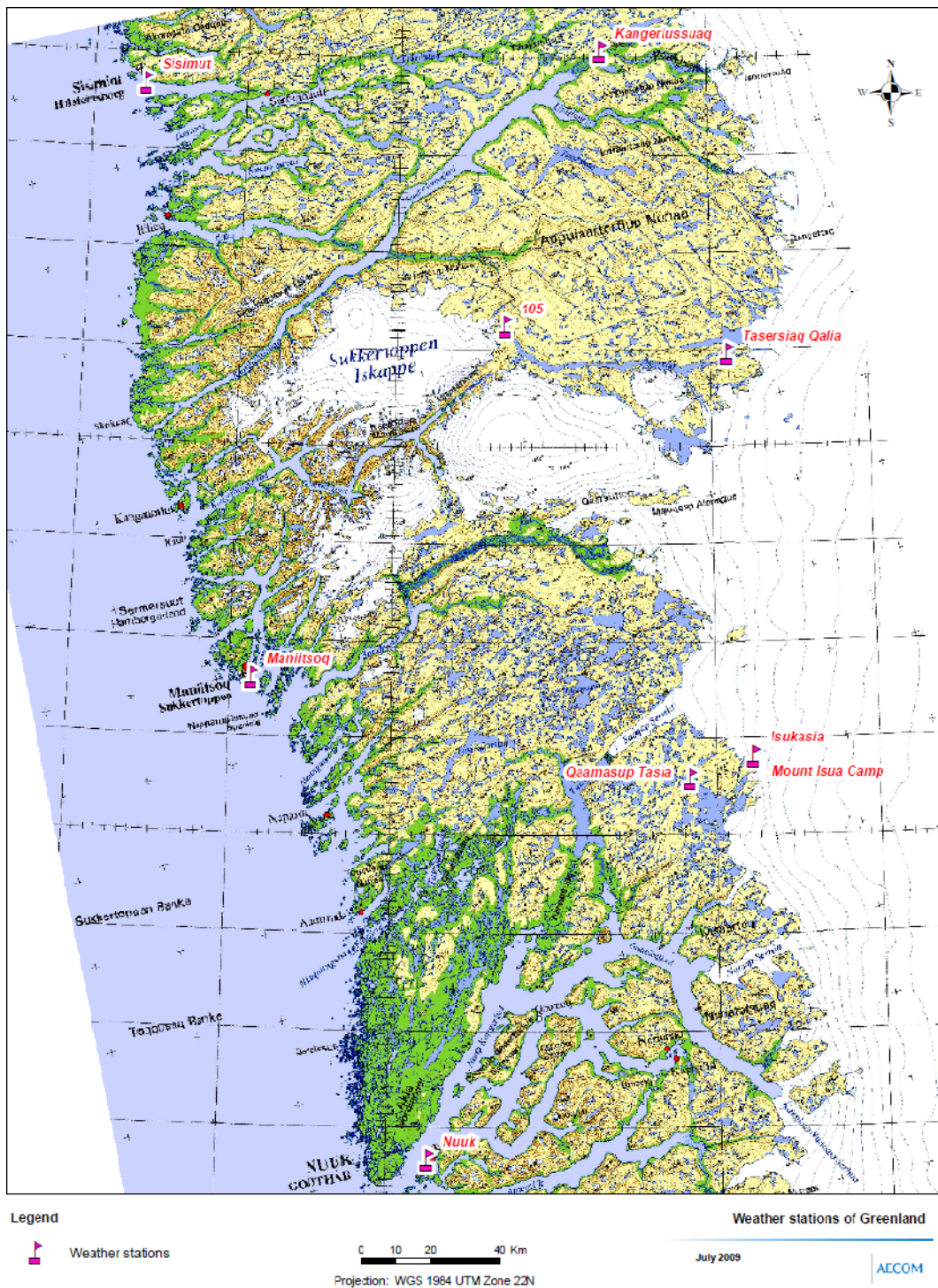


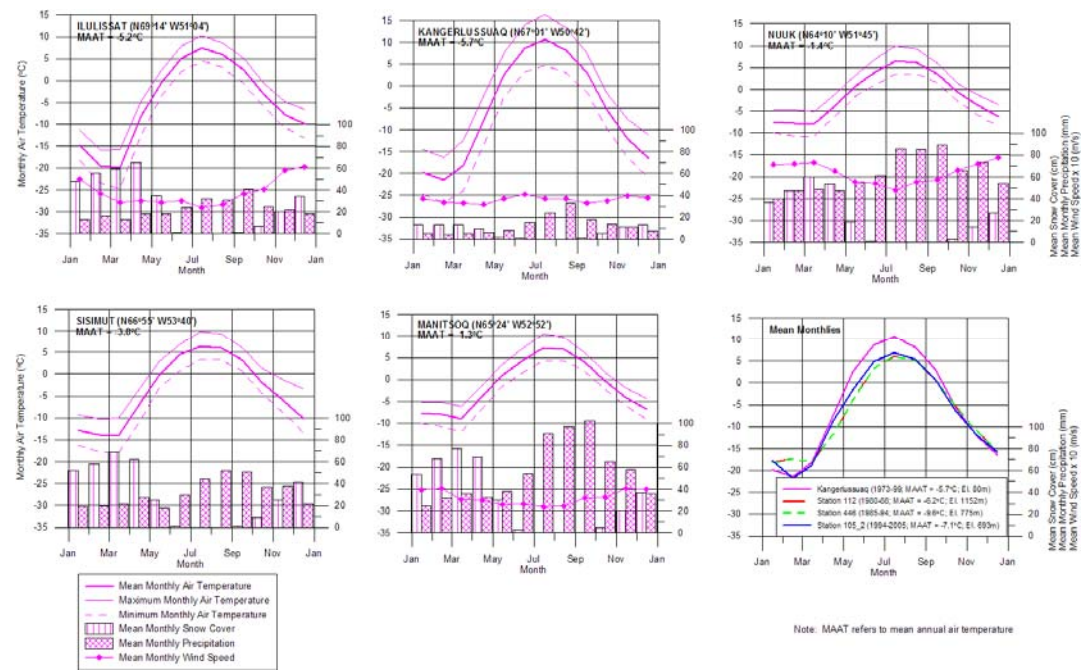
Table 2.4 summarizes the locations, elevations, and period of record for the various stations with air temperature records. Kangerlussuaq is the only DMI station that is located inland; the other DMI stations are located along the western coast.

Table 2.4 List of Meteorological Stations in West Greenland

Station	Name	Period of Record	Operated by	Latitude	Longitude	Elevation (m)
04221	Ilulissat	1961-90	DMI	N 69° 14'	W51° 04'	29
04230	Sisimut	1961-90	DMI	N 66° 55'	W 53° 40'	12
04231	Kangerlussuaq	1973-now	DMI	N67° 01'	W50° 42'	50
04240	Maniitsoq	1961-90	DMI	N65° 24 '	W52° 52'	25
04250	Nuuk	1961-90	DMI	N64° 10'	W 51° 45'	80
105	Tasersiaq	1994-04	ASIAQ			~ 750
112	Isukasia	1985-88 (86-87)	ASIAQ	N 65° 12'22"	W49° 45'54"	1152
	Mount Isua Camp	1971-78	ACG-VBB	N65° 12'13"	W49° 46'25"	
114	Tasersiap Qalia	1980-85	ASIAQ	N 66° 14'16"	W49° 53'42"	1000
446	Qaamasup Tasia	1985-94 (87-91)	ASIAQ	N65° 08'48"	W50° 09'47"	775

Figure 2.4 compares the climatic conditions of these stations.

Figure 2.4 Climatic Normals, West Greenland Meteorological Stations



For the DMI stations, monthly air temperatures (mean, maximum, and minimum), precipitation, snow cover and wind speed are presented for the climatic normal period of 1961-90 (Capellan et al., 2001). Figure 2.6 also presents the mean monthly air temperatures for the ASIAQ stations. Figure 2.6 shows that there is no strong relationship between latitude and air temperature. Summers are generally warmer and winters are colder inland (Kangerlussuaq) compared to the coastal stations. Kangerlussuaq also receives much less precipitation and thinner snow cover compared to the coastal stations. By inference, the climate of the project site is also expected to have warmer summers, cooler winters, and less precipitation and snow cover compared to the coastal stations.

The climate at Site 7e is influenced by the rugged topography, the Sukkertoppen ice cap, and the deep fjord. Climate may be variable even over short distances (e.g. from one end of the fjord to the other). The climate at Site 6g is expected to be warmer than at Site 7e because of its location further south, its distance from the ice sheet, and its more flatter and, less-rugged topography.

Table 2.5 lists the mean and extreme maximum/minimum daily air temperatures for the Tasersiaq station. Subfreezing temperatures can be anticipated at any time of year.

Table 2.5 Summary of Monthly Air Temperatures, Tasersiaq Station, 1994-2005

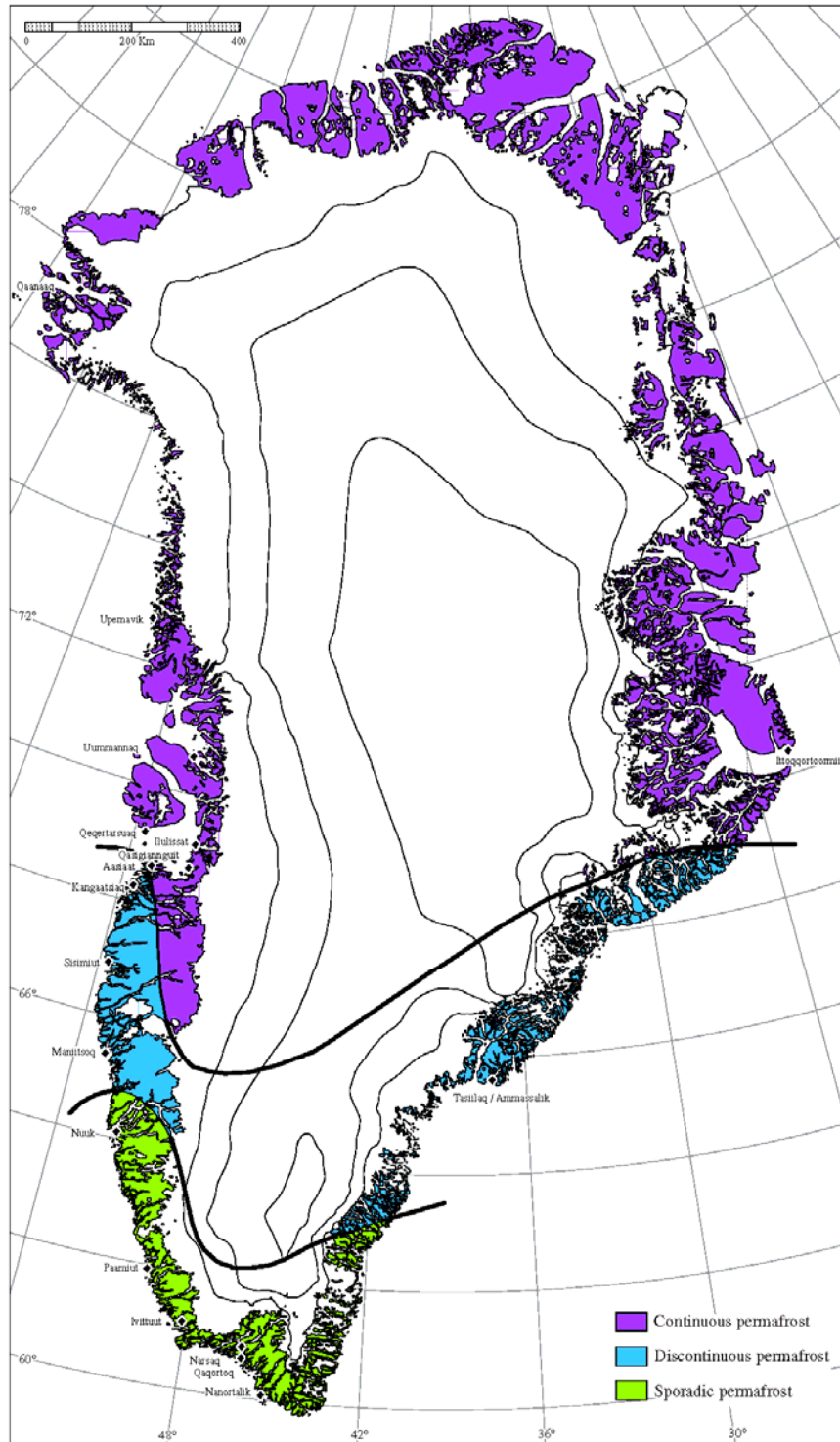
Month	Air Temperature (°C)		
	Mean Monthly	Extreme Minimum Recorded	Extreme Maximum Recorded
January	-17.6	-39.4	6.4
February	-21.8	-41.8	8.2
March	-18.9	-43.6	6.4
April	-8.8	-30.8	6.1
May	-1.5	-23.5	13.9
June	4.8	-5.7	17.9
July	6.9	-2.2	17.5
August	5.3	-3.4	15.2
September	0.7	-13.3	10.4
October	-6.4	-31.0	7.4
November	-11.8	-32.9	8.2
December	-15.6	-41	5.8

The mean annual air temperature at the Site 7e dam areas is expected to be approximately -7°C, based on the monitoring data from the Tasersiaq station. Considering the topographic variations across the site, the climate is expected to be cooler (mean annual air temperature of approximately -9°C) and windier at the top of the fjords near the edge of the ice cap, and warmer (mean annual air temperature approximately -4°C) at the head of Evighedsfjord. At Site 6g, the mean annual air temperature is estimated to range from -5°C to -4°C. Closer to the ice sheet to the east, temperatures are expected to be somewhat cooler.

2.5 Permafrost and ground temperatures

The presence of permafrost, or ground that is perennially frozen for at least two consecutive years, is controlled primarily by climate (air temperature, snow cover, solar radiation), but also by terrain factors such as subsurface conditions, surface cover characteristics, and proximity to water bodies. The project area is within the area of discontinuous permafrost, according to the Greenland permafrost distribution map by Weidick (1968) (see Figure 2.5). The southern limit for continuous permafrost follows approximately the mean annual temperature isotherm of -5°C (Weidick, 1975).

Figure 2.5 Permafrost Distribution Map for Greenland



Permafrost has been categorized into three different types: continuous permafrost, which has continuous regions of permafrost with dispersed frost-free “islands”, discontinuous permafrost, which has more and larger areas without permafrost, and sporadic

permafrost, where the permafrost is limited to small areas. The map shows the distribution of permafrost in regions at sea level. After Weidick (1968).

Ground temperature data for west Greenland area are available from the following sources:

- Ilulissat Airport (Ingeman-Nielsen, et al., 2008)
- Kangerlussuaq (van Tatenhove, 1994)
- Buksefjord Hydropower Project (N&R Consult A/S, 1994)
- Paakitsup Akuliarusersua Hydroelectric Power Station (Arctic Consultant Group and LIC Consult, 1985)
- Site 7e Power Tunnel Inlet (PB Power, pers. Communication)
- Sites 6g Power Tunnel Inlet and Outlet (PB Power, pers. Communication).

Figure 2.6 compares the measured ground temperature data. Table 2.6 summarizes the available ground temperature data. Permafrost thickness is not expected to exceed 300 m at the project site. Permafrost is expected to be more discontinuous (i.e., dispersed) at Site 6g than at Site 7e.

Figure 2.6 Ground Temperature Profiles, West Greenland

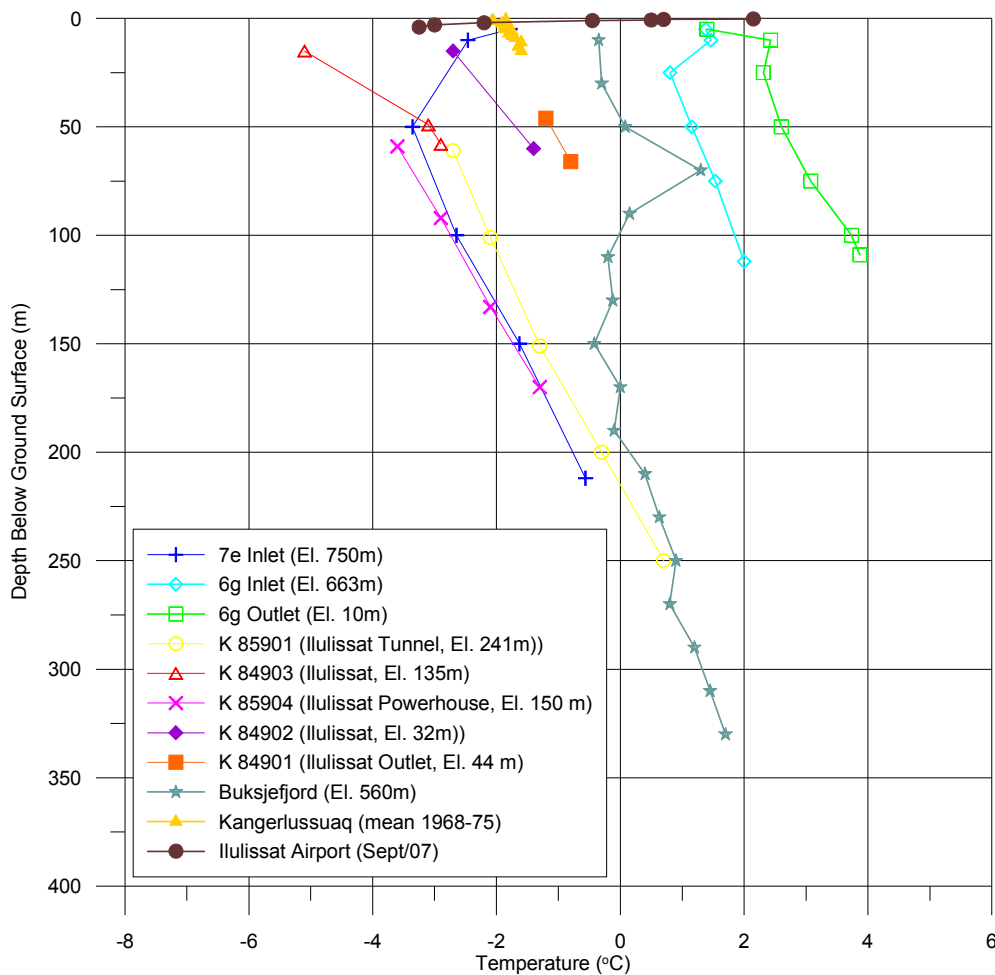


Table 2.6 Summary of Ground Temperature Data, West Greenland

Location	Ground Temperature	Permafrost Thickness	Comments
Ilulissat	-3.25°C at 4 m depth in early September 2007	Indeterminate	Measured ground temperatures up to 5 m depth
Kangerlussuaq	Mean ground surface - 1.6°C	Approximately 127 m	Measured ground temperatures up to 15 m depth
Buksefjord stage III	Approximately -0.4°C at ground surface	Up to 170 m depth	Warm (+1.3°C) temperatures at 70 m depth possibly attributed to flowing groundwater
Paakitsup Akuliarusersua	Approximately -5°C to -2°C at ground surface	Up to approximately 240 m; thinner near inlet/outlet of power tunnel because of thermal influence from lake/fjord	5 deep ground temperature profiles, additional shallow temperatures (within 3.5 m depth).
Site 7e	Approximately 4.4°C at ground surface	Approximately 240 m	Measured ground temperatures up to 218 m depth. Located approximately 200 m from lake, only minor thermal influence from lake
Site 6g	Approximately 0.4°C to +1.4°C at ground surface	No permafrost	Holes located within 20 m distance of lake/fjord; thermally influenced by water bodies, Temperature measurements up to 112 m depth

The published ground temperature data summarized in Table 2.6 suggest that ground surface temperatures are between approximately 3 to 4°C warmer than the average annual air temperature; this is consistent with observations from northern Canada.

Ice sheets and glaciers dominate much of the Greenland landscape and impact the project site. It has been postulated by many that the bottom temperature of a continental ice sheet is colder than 0°C (Brown and Pewe, 1973). However, because of the proximity of bottom temperatures to 0°C, permafrost beneath the ice cap or glacier may be thinner than in areas exposed to cold air temperatures.

3 Hydrology

3.1 Previous study

Hydrologic modeling of the study area was carried out by Vatnatskil in 2005, and updated in 2008 and 2009. The modeling work produced long term series (50 years) for the past climatic as well as projected series for 2020 and 2040 time horizons.

3.2 Available data

3.2.1 Hydrography and Drainage Basins

At Site 6g, the proposed project layout takes advantage of the presence of two main lakes: the Lake Tussaap Tasia (labeled Lower Lake) where the intake for the power tunnel will be constructed and to the north, Lake Imarsuaq (labelled Big Lake). The layout of these lakes is presented in Figure 3.1 hereafter.

With a surface area of 76.54 km² at the normal elevation of 675 m, the storage capacity of the Big Lake is much higher than for the Lower Lake whose surface area is 12.51 km² at the normal elevation of 654 m. Between these two lakes is a smaller one (labeled Little Lake) with a normal water elevation of 666 m.

The Big Lake will constitute the main storage for the installations and will be regulated, while the Lower Lake water level will be kept constant to maximize the head.

These two lakes are not connected in natural conditions: the Lower Lake outlet flows southward, while the Big Lake outlet is located in the north part of the lake and flows northward. It is planned to transfer water from the Big Lake to the Lower Lake with Tunnel 1: the water will flow to the Little Lake and then to the Lower Lake. The transfer of the flow from the Little Lake to the Lower Lake will be improved by excavating two channels: Canals 1 and 2.

At the west of the Big Lake and included in his catchment is another Lake (Lake 682) with an area of 15.1 km². It could be interesting to use the proximity of this lake to combine its storage capacity to the Big Lake.

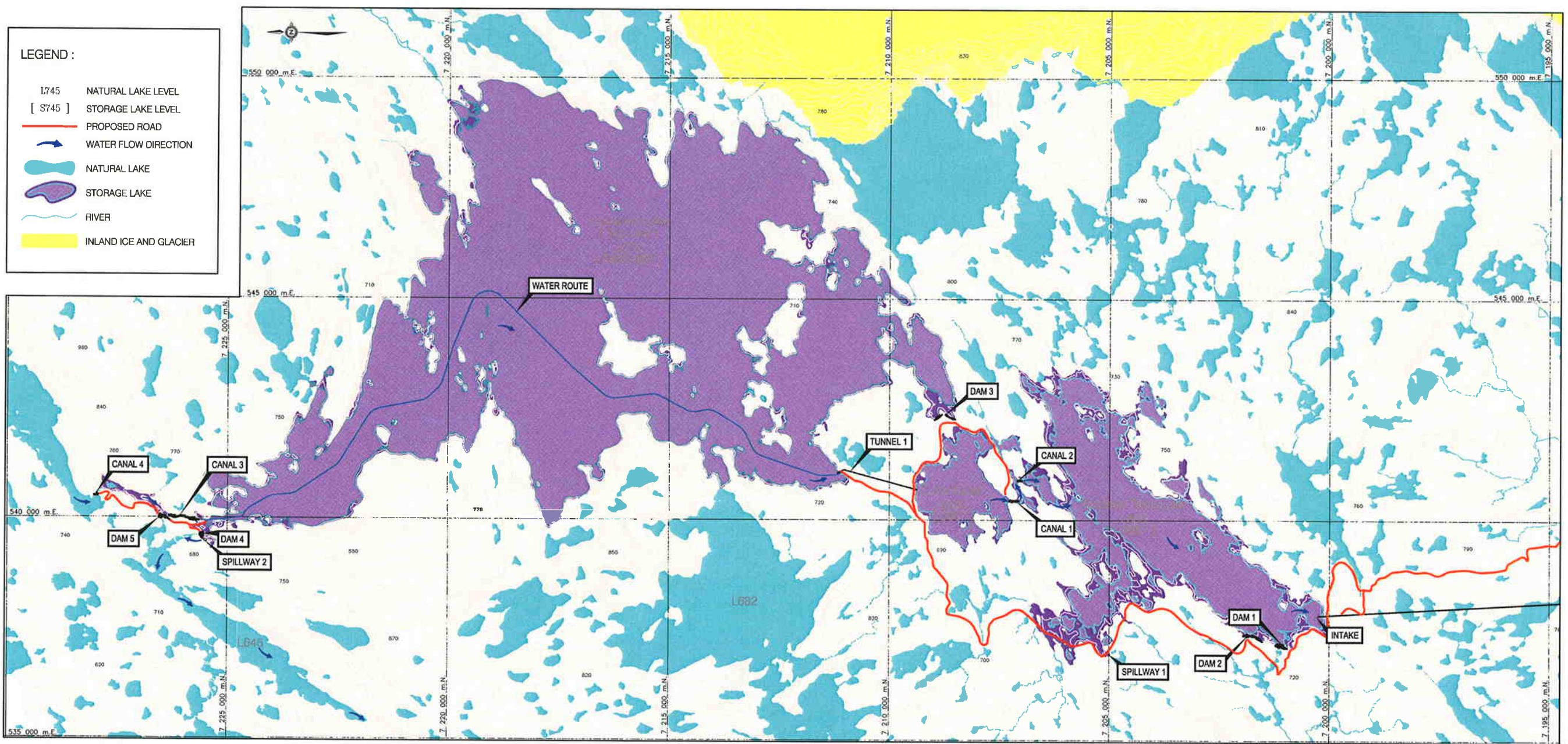
Two adjacent catchments, one northeast of the Big Lake and the other southeast of the Lower Lake, present configurations that would allow the diversion of their flows towards projected reservoirs.



Figure 3.1
Layout of the Reservoirs at 6g

Project : 0518015
Date : November 2009

AECOM



The drainage basins have been delineated in the hydrologic report prepared by Vatnatskil (2005 to be updated). The sub-catchments for site 6g are presented in Figure 3.2. The catchments contributing to the inflows present large parts of glacial areas as indicated in Table 3.1. At site 6g, the total area of the drainage basin is 1 548 km², from which 63% is glacier covered.

The catchment areas as well as the average inflows to site 6g are presented in Table 3.1 below:

Table 3.1 Average inflows at Site 6g

	Catchment area (km ²)			Module discharge* (m ³ /s)
	Non Glacial	Glacial	Total	
Big Lake	419	878	1298	28.1
Lower Lake	95	14	109	3.3
Adjacent Northeast catchment	9	19	28	1.7
Adjacent Southeast catchment	54	59	113	0.88
<i>Total</i>	<i>578</i>	<i>970</i>	<i>1548</i>	<i>34.0</i>

* Estimated from the generated series 1958-2008

From the layout presented in Figure 3.1, the required structures to control the inflows include:

- two dams (Dam 1, Dam 2) and a spillway (Spillway 1) to close the Lower Lake, where is located the intake;
- two canals (Canal 1, Canal 2) to ensure the flow transfer from Little Lake to Lower Lake;
- a regulated tunnel (Tunnel 1) to transfer water from Big Lake to Lower Lake;
- two dams (Dam 3, Dam 4) and a spillway (Spillway 2) to close the Big Lake.

Options including the connection of Big Lake with Lake 682 (labelled Middle Lake) to use the bathymetry of this lake, and also recuperating the flows from adjacent catchments may be considered. Additional structures involve:

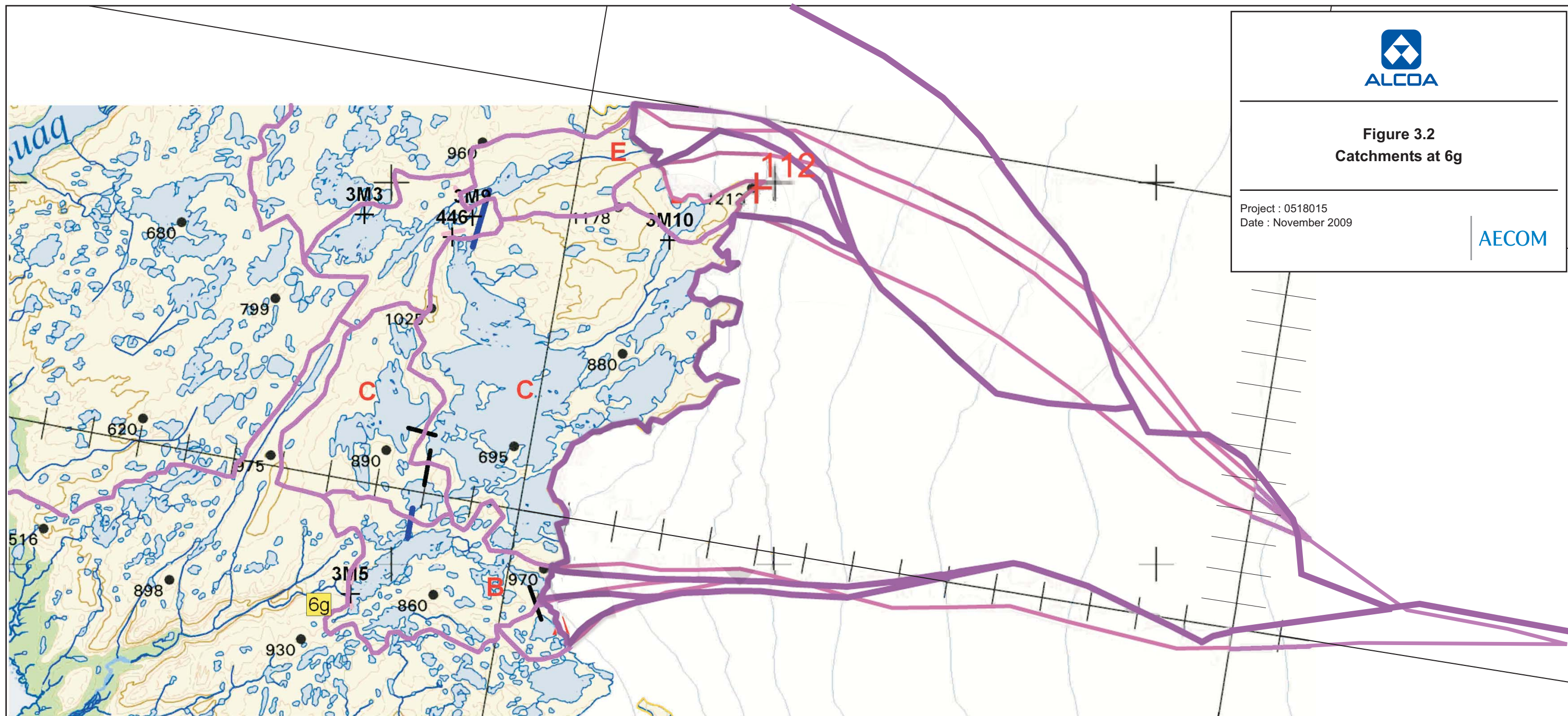
- a tunnel to connect Lake 682 with Big Lake;
- a tunnel to transfer the flow from the southeast catchment;
- two canals (Canal 3, Canal 4) and a dam (Dam 5) to ensure the transfer of the flow from the northeast catchment.



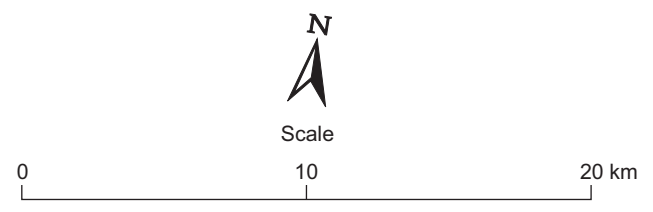
Figure 3.2
Catchments at 6g

Project : 0518015
Date : November 2009

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- Legend
- A** Catchment letter
 - Catchment limit
 - New watersheds
 - Tunnels
 - Canals



Source : VERKFRÆDISTOFAN VATNASKIL
SW-GREENLAND HYDROLOGIC MODELS
Overview of model 6g7d, Figure 8, page 289
Greenland Hydropower - Feasibility assessment
BP Power Boston, MA
January 2008

3.2.2 Water levels and tides

3.2.2.1 Water Level of Lake Imarsuaq Lake and Tussaap Tasia Lake

Water levels of Imasuaq Lake (Big Lake / L675) and Tussaap Tasia Lake (Lower Lake, L654) may be obtained respectively from hydrometric observations at the gauging station 446 operated between 1978 to 1990, and the gauging station M6 operated between 1974 and 1985.

Water levels at Site 6g were also measured during the bathymetric surveys. The relevant values are presented in Table 3.2.

Table 3.2 Measured water level at Site 6g from bathymetric survey

Location	UTM Coordinate		Water level (m)
	Northing	Easting	
Big Lake	7 215 790	544 780	674.77
Canal 3 Outlet	7 225 420	539 910	675.08
Canal 1 Inlet	7 207 370	540 430	665.82
Little Lake	7 208 350	540 530	665.87
Middle Lake	7 213 270	537 710	682.33
Power Tunnel Outlet	7 187 504	538 780	0.00
Tunnel 3 Inlet	7 202 200	549 680	761.46
Tunnel 3 Outlet	7 203 960	548 810	744.92
Tunnel 2 Inlet	7 213 310	540 060	682.26
Tunnel 2 Outlet	7 212 560	540 860	674.91
Tunnel 1 Inlet	7 211 130	540 980	674.89
Tunnel 1 Outlet	7 209 370	540 650	665.82

* New numbering are used for the structures.

3.2.2.2 Tides

The tidal water has been measured near the end of the fjord at Anavik for 8 weeks. A 3 year tidal record and a one year 10 minute tidal record for Anavik have been established, by correlating the measured data from Anavik with record tidal data from Nuuk.

Tidal extreme values have been calculated on the basis of the three year tidal record, while mean sea level is calculated on basis of the one year 10 minute tidal record. The values are presented in Table 3.3 hereafter. No estimation has been made on how the water level will be affected by extreme situations with low pressure systems, wind setup, surge flood, and the effects of the periodic tidal cycle of 18.6 years.

Table 3.3 Tides characteristics

	Height (m)
Highest Astronomical Tide	2.07
Mean High Water of Spring Tide	1.64
Mean Sea Level	-0.75
Mean Low Water of spring Tide	-2.96
Lowest Astronomical Tide	-3.49
Delay of the tidal wave, Nuuk, mean value	Approx. 10 minutes

3.2.3 Inflows

The runoff has been modeled by Vatnaskil (2007, 2008, and 2009) to produce synthetic discharge series based on the past climate (1958-2008) and projected series which takes into account climate warming.

The catchments contributing to the inflows present large parts of glacial areas as indicated in Table 3.4. At site 6g, the total area of the drainage basin is 148 km², from which 63% is glacier covered.

Table 3.4 Repartition of the catchments

	Catchment Area (km ²)	Contribution* to the module discharge (m ³ /s)
Non glacial area	578	8.8
Glacial area	970	25.2
Total catchment	1 548	34.0

* Based on historic synthetic series

3.2.3.1 Gauging stations

The gauging stations located in the drainage basin of site 6g cover different sub-catchments. These stations include:

- Station 446, which measures the inflows to the Big Lake (L676), with data from 1978 to 1990;
- Station 3M5, which measures the inflows to the Lower Lake (L654), with data from 1974 to 1985;
- Station 3M9, which measures the inflows to the lake (L701) northeast of the Big Lake, with data from 1977 to 1983;
- Station 3M10, which covers a sub catchment of the Big Lake.

3.2.4 Storage curves

The layout of the lakes at site 6g is presented in Figure 3.1. The storage curves data are based on 2 meters contour topography and 10 meters contour bathymetry. The surface area measured on the maps for various elevations are presented in Table 3.5 for each Lake.

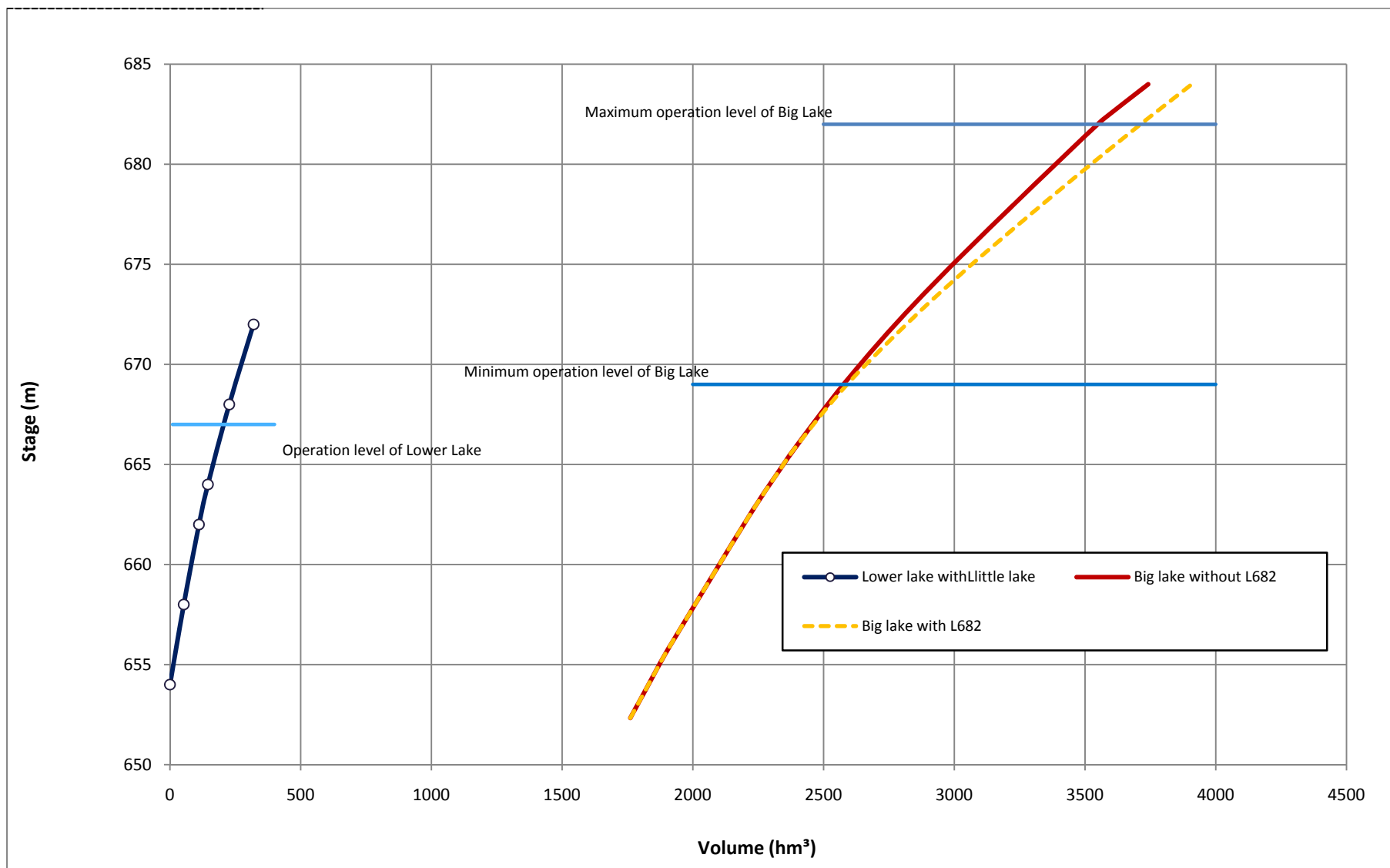
The storage curves used for site 6g are presented in Figure 3.3 hereafter, for the Lower Lake, the Big Lake alone and the Big Lake connected with Lake 682.

Due to the shoals at an elevation of about 669 m in the Big Lake, the minimum operating level of this lake may not be lower than this value. The same limitation would apply if it is planned to use Lake 682 bathymetry.

Table 3.5 Stage - Surface Areas of the lakes at site 6g

Lake 682		Big Lake		Little Lake		Lower Lake (including Little Lake over 664 m)	
Stage (m)	Surface area (km ²)	Stage (m)	Surface area (km ²)	Stage (m)	Surface area (km ²)	Stage (m)	Surface area (km ²)
562.33	0.009	444.77	0.002	625.87	0.002	654.00	12.511
572.33	0.040	454.77	0.043	635.87	0.164	658.00	13.860
582.33	0.588	464.77	0.137	645.87	0.491	662.00	15.315
592.33	0.761	474.77	0.259	655.87	1.237	664.00	18.815
602.33	1.605	484.77	0.419	665.87	3.064	668.00	22.000
612.33	2.326	494.77	0.642			672.00	24.537
622.33	2.958	504.77	0.898			676.00	26.571
632.33	3.740	514.77	1.214			682.00	31.410
642.33	5.011	524.77	1.598			684.00	32.940
652.33	6.379	534.77	2.076				
662.33	7.904	544.77	2.777				
672.33	9.723	554.77	3.918				
682.33	15.065	564.77	5.085				
		574.77	6.956				
		584.77	8.758				
		594.77	11.314				
		604.77	14.767				
		614.77	18.438				
		624.77	22.928				
		634.77	28.144				
		644.77	34.796				
		654.77	42.518				
		664.77	51.998				
		674.77	76.540				
		676.00	89.469				
		678.00	91.605				
		680.00	93.791				
		682.00	96.031				
		684.00	99.881				

Figure 3.3 Storage Curves of the lakes at Site 6g



3.3 Observed data and synthetic series

3.3.1 Observed data

The discharges were measured at various gauging stations. The average and maximum measured discharges are presented in Table 3.6, for the inflows to the Big Lake (Station 446), the Lower Lake (Station 3M5) and from the Northeast catchment (Station 3M9).

The available data indicate fairly stable inflows from year to year, with variation coefficients of 16%, 12% and 19% respectively.

Table 3.6 Measured discharges

Year	Station 446		Station 3M5		Station 3M9	
	Average Discharge (m ³ /s)	Max Discharge (m ³ /s)	Average Discharge (m ³ /s)	Max Discharge (m ³ /s)	Average Discharge (m ³ /s)	Max Discharge (m ³ /s)
1974			2.78	10.75		
1975			3.57	17.58		
1976			4.18	21.39		
1977			2.91	13.90	1.77	7.91
1978	25.59	157.42	4.40	24.69	1.47	20.30
1979	17.93	99.13	3.20	15.30	1.42	10.48
1980	25.27	130.91	2.71	10.47	1.60	16.94
1981	27.69	208.98	3.36	18.39	1.67	20.30
1982	19.50	138.88	2.37	12.58	1.34	8.60
1983	20.52	115.80	4.05	22.77	1.89	17.48
1984						
1985	27.29	133.95	3.38	23.24		
1986	25.95	133.80				
1987	31.23	200.77				
1988	27.41	161.31				
1989	26.48	163.81				
<i>Average</i>	<i>25.0</i>		<i>3.4</i>		<i>1.6</i>	

3.3.2 Synthetic series

3.3.2.1 Hydrological model

Daily flow series have been generated for the period September 1958 to August 2008 by Vatnaskil (2007², 2008, and 2009³). The discharge data considered in this evaluation are those from the 2009 Vatnaskil report.

The hydrological model uses the energy balance approach, which requires climate parameters including air temperature, precipitation, wind speed, air humidity, surface air pressure and incoming long-wave and short-wave radiation data.

² Vatnaskil (2007) Hydrologic modeling in Southwest Greenland, prepared for Alcoa

³ Vatnaskil (2009) Revised hydrological models in Southwest Greenland and future flows, prepared for Alcoa

The terrain data come from a high-resolution digital elevation model for the glacier surface and glacier base and a geographical map from GTK⁴. The catchments delineation is based on these terrain data. Where the DEM is available, the catchments were delineated based on the Shreve potential theory, which takes into account an ice load factor. (Vatnaskil, 2009).

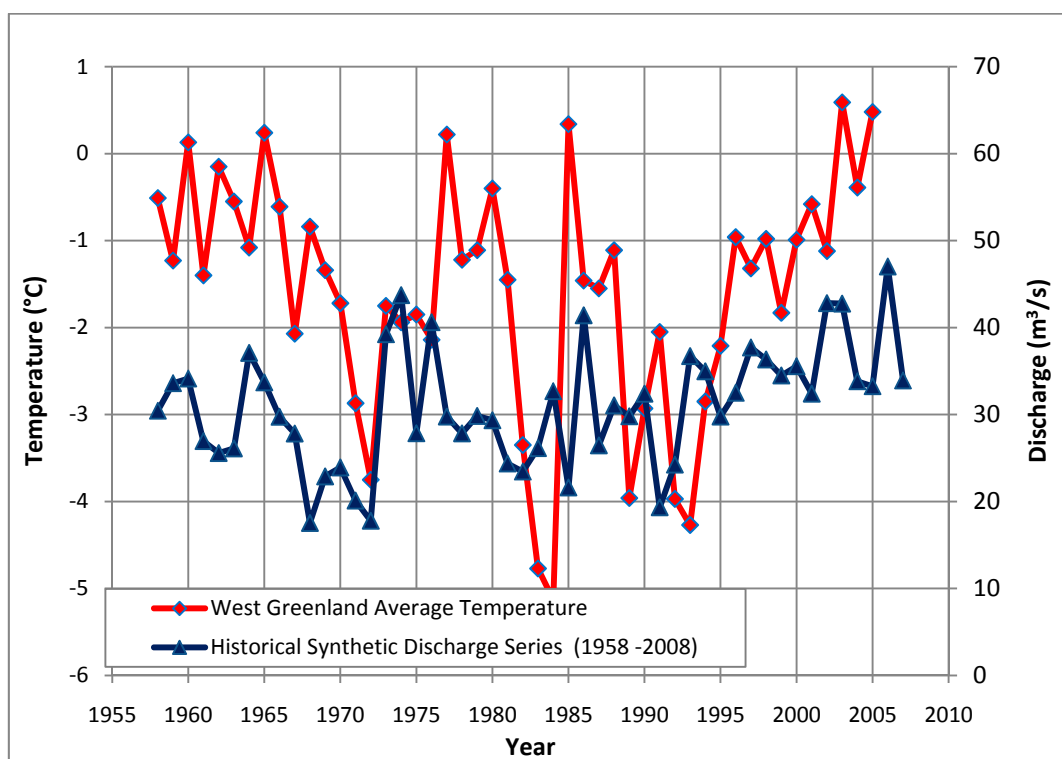
First, the model was calibrated to better fit total water balance, observed average discharge distribution within a year and the measured ablation. It then served to produce historical synthetic discharge series, using past climate data from September 1, 1958 to August 31, 2008.

As a high percentage of the discharges come from glacier melting, the main independent variable is temperature. Figure 3.4 shows a plot of the annual temperatures and discharges.

It was then assumed that future flows can be estimated on the basis of assumption of climate warming projected changes. The simulations have been done for three cases:

- daily historical series, using past climate data from September 1, 1958 to August 31, 2008;
- projected daily discharge series for 2020, using a scenario of climate warming to have a projection of the inflows within the horizon of 2020;
- projected discharge series for 2040, using similar methods to that of 2020.

Figure 3.4 Temperature and discharges



⁴ Grønlands Topografiske Kortværk (GTK)

3.3.2.2 Warming climate effect

The future warming rates have been assessed by several climate model studies, using different scenarios of greenhouse gas emissions. Future melt rates on Greenland ice sheet can be expected to increase on average in the coming decades, due to the anticipated global warming of the atmosphere.

The approach chosen by Vatnaskil was to use monthly average warming rates from a model study, to project historical temperature and longwave radiation fields to the future reference years (2020 and 2040), then running the already calibrated hydrological model using the projected meteorological fields as input. The resulting discharge series can be considered to determine the probability distribution of the discharge for that reference year.

The warming rates used for the projection were extracted from a downscaling of a global climate model run, for Greenland and the surrounding seas (Vatnaskil, 2009). The model used for the downscaling is the regional climate model HIRHAM4. The global climate model data were interpolated to the regional climate model grid (25x25 km) every six hours for the period 1950-2080. The lateral forcing data came from a simulation with the global coupled climate model ECHAM5/MPI-OM1.

The greenhouse gas forcing in the model is from observations up to the year 2000 and follows scenario A1B thereafter. The monthly average warming rates are given in Table 3.7.

Table 3.7 Monthly average warming rates (1960-2040)

Month	Warming rate (°C/Decade)
January	0.28
February	0.50
March	0.48
April	0.52
May	0.30
June	0.21
July	0.13
August	0.05
September	-0.07
October	0.06
November	0.04
December	0.017

The effects of warming of the atmosphere on the melt rates do not emerge only through higher sensible heat flux due to higher temperatures, but also through higher incident longwave radiation. Longwave radiation is actually the thermal radiation of the atmosphere, which basically depends of temperature increments.

The temperature projection for a given date is performed in the following steps:

- Calculating the number of years from the given date until the reference year;
- Calculating the temperature change as the product of the number of years and the temperature change per year for the specific month;
- Adding the temperature change to the historical temperature.

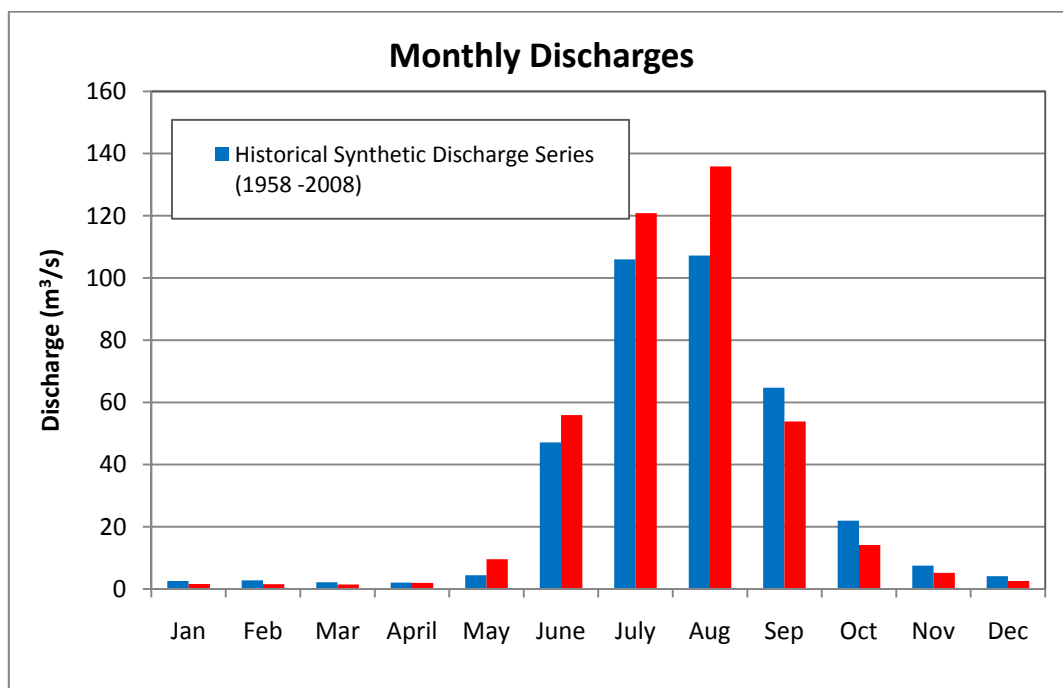
3.3.2.3 Discharge simulation results

For the synthetic historic series, the annual average discharge is 34 m³/s. The projected series for the 2020 horizon has an annual average discharge estimated to 37.4 m³/s, which shows an increase of 10% from the historic data. The Table 3.8 presents the mean and extreme annual average discharge for the three series that were computed, while Figure 3.5 shows the monthly distribution of discharges. The inflows from the northeast catchment (sub-basin E) and the southeast one (sub-basin A) are included in these module discharges.

Table 3.8 Module discharges of site 6g (all catchments)

	Historic synthetic discharge / 1958–2008 (m ³ /s)	Projected 2020 discharge (m ³ /s)	Projected 2040 discharge (m ³ /s)
Minimum	20.8	21.8	22.6
Average	34.0	37.4	39.5
Maximum	56.6	58.9	62.6
<i>Standard deviation</i>	<i>7.2</i>	<i>7.7</i>	<i>8.0</i>

Figure 3.5 Average Monthly Discharges



The inflows coming from the northeast catchment (basin E) were measured at gauging station 3M9, and have been modeled. The characteristics of the synthetic series are presented in the Table 3.9 for this subwatershed.

Table 3.9 Module discharge of the northeast subwatershed (basin E)

	Historic synthetic discharge / 1958–2008 (m ³ /s)	Projected 2020 discharge (m ³ /s)	Projected 2040 discharge (m ³ /s)
Minimum	1.2	1.3	1.3
Average	1.7	1.8	1.9
Maximum	2.4	2.4	2.5
<i>Standard deviation</i>	<i>0.3</i>	<i>0.3</i>	<i>0.3</i>

The southeast catchment (basin A) inflows were also estimated. No gauging station was installed to measure the inflows from the catchment but they could be determined by subtracting the inflows of the other catchments of site 6g from the computed synthetic series. The inflows for this subcatchment are presented in Table 3.10.

Table 3.10 Module discharge of the southeast subwatershed (basin A)

	Historic synthetic discharge / 1958–2008 (m ³ /s)	Projected 2020 discharge (m ³ /s)	Projected 2040 discharge (m ³ /s)
Minimum	0.57	0.60	0.62
Average	0.88	0.96	1.00
Maximum	1.23	1.25	1.30
<i>Standard deviation</i>	<i>0.14</i>	<i>0.14</i>	<i>0.14</i>

The discharged series considered for power availability estimation and design purposes are those estimated for the 2020 horizon.

3.4 Floods

3.4.1 Methodology

The floods estimates were based principally on the 50 years series generated by Vatnatskil (synthetic series). The methodology adopted to determine the flood characteristics considers the following points:

- the time span of observed data is not sufficient to conduct flood frequency analysis. At Site 6g, there is less than 10 complete years of data;
- the annual peak discharges are underestimated in the synthetic series, but the annual volumes are fairly represented;
- the annual flood hydrographs go from June to October, with a peak in July or August.

Two methods have been considered to estimate the floods. The first method is based on the frequency analysis of the annual synthetic maximum discharges and the second is based on the frequency analysis of the annual synthetic volume discharges.

The frequency analysis has been done with HYFRAN (Hydrological Frequency Analysis) a statistic tool developed by INRS-ETE (Institut National de la Recherche Scientifique - Eau Terre et Environnement) of Quebec. It gives a large panel of probability law, applicable to hydro-meteorological series. The retained laws are the Generalized Extreme Value (GEV) and Pearson type III.

The observed data used are the combined observations of stations 446, 3M5 and 3M9.

3.4.1.1 Peak discharge

For this method, the ratio between the observed peak discharges and the synthetic peak discharges are used to adjust 2020 projected synthetic discharges.

A constant estimated ratio of 1.2, representing the average ratio between observed peaks to synthetic peaks, is used to adjust the results of frequency analysis for the projected series.

3.4.1.2 Flood volumes

This method is based on the annual volume inflows. It appears that the volume inflows are independent and can be fitted to probability distributions. The results from frequency analysis of these data are used to determine the flood peaks from calculated ratios evaluated between peak discharges and volume inflows of the flood hydrographs.

The average of the ratio of peak discharge to average discharge of the hydrographs is about 2.4. The relevant characteristic of the hydrographs used is the ratio between peak discharges and the average discharges of the combined observations of stations 446, 3M5 and 3M9.

At site 6g, the base time of the hydrographs is about five months, from June to October. The characteristics of the hydrographs for each year show a ratio of peak discharge to average discharge varying from 2.04 to 2.99. The ratio used is the average coefficient which is estimated to a value of 2.4. The equivalent ratio of peak discharge to hydrograph volume is 0.2 and is easier to use.

3.4.1.3 Inflow hydrograph

The synthetic input hydrographs are based on aggregated observed discharges of 1981, 1982 and 1983 from stations 446, 3M5 and 3M6, and are deducted by proportional transformation.

3.4.1.4 Results of flood frequency analysis

The flood frequencies for Site 6g are presented in Table 3.11. The hydrograph used for the outlets of Big Lake and Lower Lake are presented in Figures 3.6 and 3.7. The 20 years flood will be used as design discharge during construction: for the Big Lake, it doesn't take into account the inflow coming from the northeast catchment.

Table 3.11 Flood peaks at site 6g

Return Period (years)	Flood discharges (m ³ /s)						
	Total inflows (Projected 2020)	Total inflows (Projected 2040)	Northeast Catchment (Canals 3 and 4)	Big Lake (Dams 3 and 4, & Spillway 2)	Lower Lake	Lower Lake with Tunnel 1 (Dams 1 and 2, and spillway 1)	Southeast Catchment (Tunnel 3)
10 000	419	425	28	399	62	160	6
2 000	387	394	27	368	56	155	6
1 000	372	380	26	354	54	153	6
200	337	346	24	319	49	148	6
100	321	330	23	303	46	145	6
50	304	313	22	286	44	143	5
20	280	290	21	263	40	140	5
10	260	269	20	243	37	137	5
5	237	247	18	221	34	134	5
3	217	227	17	202	31	132	5
2	198	207	15	183	29	129	4

The design criteria for the permanent structures use the flood discharges for the projected 2020 series. At big lake, corresponding values for the projected 2040 series are about 3% higher and have a little incidence.

Figure 3.6 Synthetic hydrographs used for Big Lake

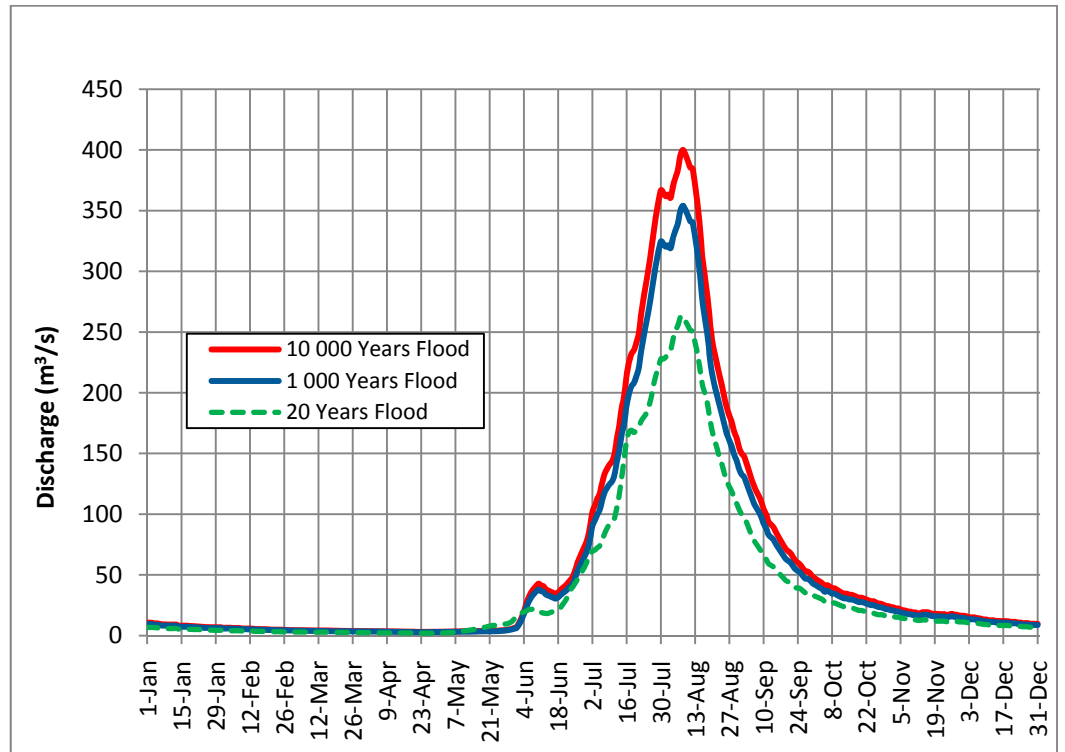
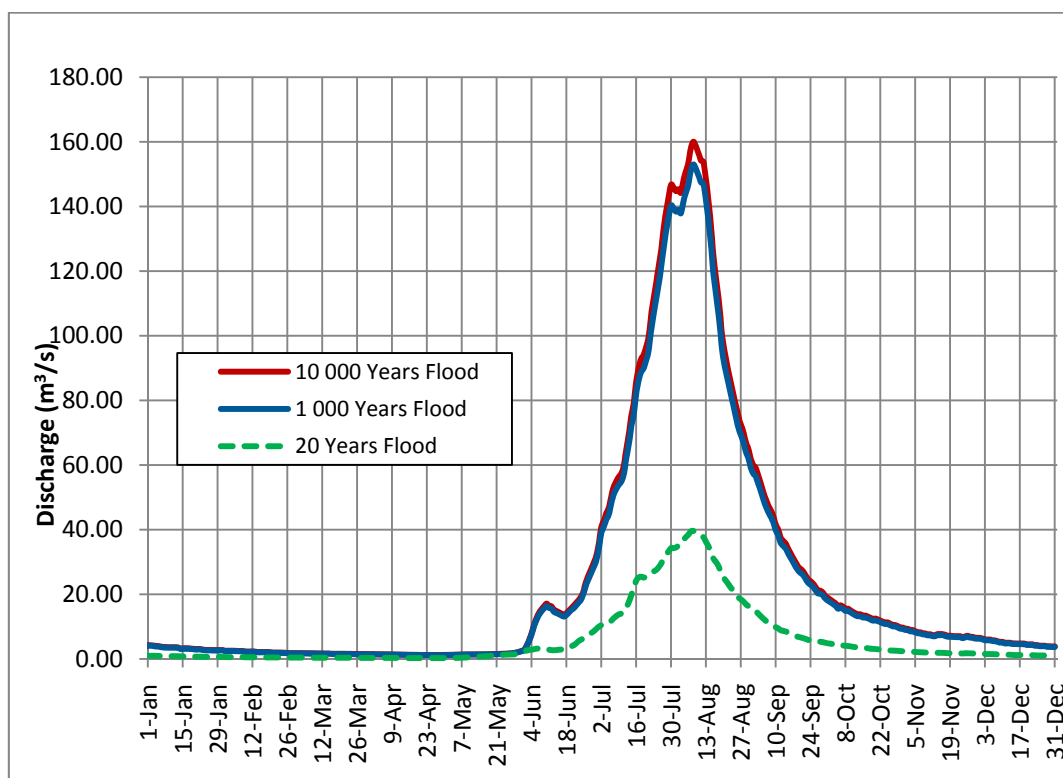


Figure 3.7 Synthetic hydrographs used for Lower Lake



3.4.2 Inflow Design Flood

The selected Inflow Design Flood (IDF) for the 6g structures is the 1:10 000 years flood. It is based on the discharges projected for the 2020 horizon, revised as indicated above.

3.4.2.1 Structures around Lower Lake: Dam 1, Dam 2, Spillway 1

The level of the Lower Lake is controlled by Spillway 1. The IDF is 160 m³/s, including the catchment of the Lower Lake, and the maximum discharge of Tunnel 1 (gate opened, Big Lake at maximum level). If Tunnel 3 is constructed, the southeast catchment flows to the Lower Lake, thus raising from the IDF of the Lower Lake to 166 m³/s.

3.4.2.2 Structures around Big Lake: Dam 3, Dam 4, Spillway 2, Dam 5

The level of the Big Lake is controlled by the Spillway 2. The IDF is 400 m³/s, including the catchment of the Big Lake and the northeast catchment transferred by Canal 4 and Canal 3.

3.4.2.3 Canal 3

Canal 3 is used as a spillway channel for Dam 5 to evacuate the floods coming from the northeast catchment. The IDF is 28 m³/s.

3.4.2.4 Canal 4

Canal 4 is designed to transfer to the Big Lake the discharges coming from the northeast catchment. Its IDF is 28 m³/s.

3.4.2.5 Tunnel 1

Tunnel 1 is regulated to supply water for the turbines. It is designed to allow the transfer of a discharge of 40 m³/s upon the most adverse conditions; minimum water level at the Big Lake and maximum water level at the Lower Lake.

3.4.2.6 Tunnel 2

Tunnel 2 is an equilibrium tunnel between Big Lake and Lake 682. Its dimension may be based upon constructability considerations. This tunnel is not part of the initial design but could be constructed to increase the power production.

3.4.2.7 Tunnel 3

Tunnel 3 is designed to transfer to the Lower Lake the discharges coming from the southeast catchment. Its IDF is 6 m³/s. This tunnel is not part of the initial design but could be constructed to increase the power production.

3.4.3 Inflows during construction

For the diversion structures, a risk of exceedance of 5% is allowed during the diversion period, estimated to one year. The flood selected is the 1:20 years flood.

The normal conditions during construction can be based upon the monthly average discharges. The flow-duration values are given in the tables 3.12 to 3.14 hereafter.

Table 3.12 Flow-Duration values at Big Lake

Exceedence probability	January	February	March	April	May	June	July	August	September	October	November	December
<i>Minimum</i>	0.49	0.44	0.41	0.40	0.41	0.48	21.09	28.23	7.04	1.91	0.81	0.55
95%	0.56	0.50	0.46	0.44	0.49	9.29	54.34	63.67	16.73	3.34	1.19	0.71
90%	0.59	0.53	0.48	0.46	0.53	14.21	61.57	75.38	20.92	4.06	1.34	0.77
85%	0.63	0.55	0.50	0.50	0.61	21.30	67.84	83.59	23.74	4.76	1.46	0.80
80%	0.65	0.58	0.53	0.52	1.29	24.44	75.92	89.76	26.04	5.45	1.58	0.84
75%	0.67	0.60	0.55	0.53	1.91	27.15	83.70	96.73	28.56	6.06	1.72	0.87
70%	0.69	0.61	0.57	0.55	2.36	29.10	90.68	103.37	31.25	6.68	1.86	0.92
65%	0.71	0.63	0.58	0.57	3.00	31.51	96.14	110.38	33.79	7.37	2.00	0.96
60%	0.72	0.65	0.60	0.59	3.92	34.79	101.02	116.54	37.09	8.18	2.16	1.00
55%	0.75	0.66	0.61	0.63	5.36	38.88	105.77	123.47	40.42	8.93	2.35	1.05
<i>Average</i>	1.26	1.44	1.59	3.03	12.16	50.21	118.27	135.70	50.06	12.23	4.32	2.04
50%	0.78	0.68	0.63	0.68	7.14	43.57	111.53	129.39	43.42	9.79	2.56	1.11
45%	0.82	0.70	0.66	0.79	8.57	48.43	118.18	135.93	47.09	10.65	2.77	1.17
40%	0.85	0.73	0.69	1.00	10.36	52.72	125.66	143.27	51.39	11.65	3.03	1.26
35%	0.88	0.75	0.72	1.44	12.91	59.07	132.47	151.30	55.71	12.68	3.38	1.38
30%	0.91	0.77	0.75	1.92	15.70	64.82	140.72	159.26	60.56	13.82	3.84	1.55
25%	0.95	0.79	0.80	2.50	18.10	69.52	148.35	168.53	65.54	15.24	4.41	1.81

Exceedence probability	January	February	March	April	May	June	July	August	September	October	November	December
20%	1.03	0.82	0.98	3.44	20.44	75.69	157.70	179.26	70.46	17.01	5.29	2.24
15%	1.16	0.86	2.46	4.64	24.27	83.07	167.85	190.08	76.26	19.26	6.85	3.05
10%	1.39	1.13	4.54	5.94	29.85	93.77	182.11	205.67	84.94	22.29	8.69	4.41
5%	3.31	5.69	7.53	9.95	41.08	109.45	202.12	226.49	102.57	29.56	14.45	6.99
<i>Maximum</i>	22.18	51.04	21.90	89.05	106.12	185.38	258.32	297.62	211.30	82.51	42.60	26.52

Table 3.13 Flow-Duration values at Lower Lake

Exceedence probability	January	February	March	April	May	June	July	August	September	October	November	December
<i>Minimum</i>	0.23	0.21	0.19	0.18	0.18	0.24	6.52	3.89	1.13	0.47	0.29	0.25
95%	0.27	0.23	0.21	0.22	0.23	2.53	8.77	6.94	2.22	0.73	0.40	0.31
90%	0.28	0.25	0.23	0.23	0.25	5.18	9.31	7.68	2.66	0.82	0.44	0.34
85%	0.30	0.26	0.24	0.24	0.27	6.67	9.81	8.29	3.04	0.91	0.46	0.35
80%	0.31	0.27	0.25	0.25	0.31	7.67	10.30	8.75	3.33	1.00	0.49	0.36
75%	0.32	0.28	0.26	0.25	0.46	9.00	10.74	9.25	3.65	1.09	0.51	0.37
70%	0.32	0.29	0.27	0.26	0.61	9.87	11.22	9.62	3.95	1.19	0.53	0.38
65%	0.33	0.30	0.28	0.27	0.76	10.70	11.64	10.00	4.28	1.29	0.56	0.39
60%	0.33	0.30	0.28	0.27	1.00	11.36	11.96	10.46	4.58	1.41	0.58	0.40
55%	0.34	0.31	0.29	0.28	1.34	12.17	12.35	10.88	4.90	1.52	0.62	0.41
<i>Average</i>	0.45	0.47	0.49	0.78	3.19	12.83	13.52	11.49	5.57	2.08	0.99	0.58
50%	0.35	0.31	0.30	0.29	1.61	12.68	12.77	11.23	5.17	1.65	0.64	0.43
45%	0.36	0.32	0.30	0.32	2.00	13.40	13.17	11.58	5.47	1.79	0.68	0.44
40%	0.36	0.33	0.31	0.35	2.45	14.14	13.58	11.91	5.79	1.94	0.72	0.46
35%	0.37	0.33	0.32	0.40	3.13	14.86	14.03	12.40	6.15	2.11	0.79	0.48
30%	0.39	0.34	0.33	0.51	3.87	15.67	14.51	12.94	6.51	2.29	0.87	0.50
25%	0.40	0.35	0.35	0.62	4.53	16.94	15.22	13.50	6.87	2.50	1.00	0.53
20%	0.41	0.37	0.36	0.81	5.53	18.17	16.01	14.24	7.40	2.74	1.19	0.59
15%	0.43	0.38	0.48	1.11	6.99	19.44	17.24	14.86	8.09	3.11	1.47	0.71
10%	0.45	0.41	1.17	1.52	8.63	20.91	18.67	15.61	9.04	3.71	1.93	0.95
5%	0.83	1.32	1.78	2.72	10.77	23.16	21.72	16.80	10.45	5.00	2.96	1.54
<i>Maximum</i>	5.84	12.18	5.32	15.86	23.00	26.22	34.50	22.54	16.97	11.90	7.93	5.11

Table 3.14 Flow-Duration values at northeast catchment (Dam 5)

Exceedance probability	January	February	March	April	May	June	July	August	September	October	November	December
<i>Minimum</i>	0.06	0.06	0.05	0.05	0.05	0.06	1.90	1.85	0.43	0.16	0.09	0.08
95%	0.07	0.07	0.06	0.06	0.06	0.50	4.47	3.34	0.92	0.24	0.12	0.09
90%	0.08	0.07	0.06	0.06	0.07	0.97	4.94	3.82	1.11	0.28	0.13	0.09
85%	0.08	0.07	0.07	0.07	0.07	1.69	5.24	4.24	1.25	0.31	0.14	0.10
80%	0.09	0.08	0.07	0.07	0.09	2.06	5.49	4.64	1.39	0.34	0.15	0.10
75%	0.09	0.08	0.08	0.07	0.13	2.37	5.75	4.90	1.52	0.38	0.16	0.11
70%	0.09	0.08	0.08	0.07	0.16	2.62	5.98	5.11	1.65	0.41	0.16	0.11
65%	0.09	0.09	0.08	0.08	0.20	2.89	6.16	5.30	1.77	0.45	0.17	0.11
60%	0.10	0.09	0.08	0.08	0.27	3.15	6.31	5.52	1.91	0.49	0.18	0.12
55%	0.10	0.09	0.08	0.08	0.35	3.52	6.49	5.80	2.04	0.54	0.19	0.12
<i>Average</i>	0.14	0.16	0.17	0.25	0.89	4.09	6.66	5.93	2.38	0.71	0.33	0.20
50%	0.10	0.09	0.08	0.09	0.45	4.02	6.69	5.98	2.18	0.59	0.21	0.13
45%	0.11	0.09	0.09	0.09	0.57	4.40	6.82	6.15	2.32	0.64	0.22	0.13
40%	0.11	0.10	0.09	0.11	0.73	4.71	6.97	6.37	2.49	0.69	0.23	0.14
35%	0.11	0.10	0.10	0.13	0.90	5.07	7.12	6.57	2.67	0.76	0.25	0.15
30%	0.12	0.10	0.10	0.17	1.07	5.49	7.30	6.76	2.86	0.82	0.28	0.16
25%	0.12	0.10	0.10	0.21	1.25	5.87	7.45	7.01	3.08	0.90	0.33	0.18
20%	0.12	0.11	0.11	0.27	1.51	6.21	7.65	7.24	3.29	0.99	0.41	0.21
15%	0.13	0.11	0.20	0.35	1.92	6.60	7.98	7.52	3.55	1.11	0.53	0.27
10%	0.15	0.13	0.40	0.49	2.27	7.05	8.26	8.02	3.90	1.27	0.71	0.37
5%	0.35	0.59	0.66	0.68	3.12	7.84	9.08	8.45	4.39	1.62	0.99	0.61
<i>Maximum</i>	2.23	4.47	1.94	6.08	7.14	9.98	12.24	9.96	7.28	3.33	3.09	1.91

3.5 Reservoir filling time

It is proposed to close the diversion tunnels at the end of November 2014 (4th year of construction). Since commissioning of the powerhouse is planned starting in March 2015, the Lower Lake will be filled first using part of the volume available in the Big Lake between levels 675 m (natural water level) and 669 m (planned minimum operating level of the reservoir). Tunnel 1 will be opened starting in December 2014 to transfer water from the Big Lake to the Lower Lake.

It would take two months to fill the Lower Lake if a steady flow of 40 m³/s is transferred through Tunnel 1, which meets the time constraint for the commissioning of the powerhouse. Considering that the initial water level of the Big Lake is at level 675 m, it could be possible to transfer a discharge close to 100 m³/s through Tunnel 1 if it becomes necessary to fill the Lower Lake in a shorter period of time.

Following the filling of the Lower Lake, the Big Lake reaches a low water level of 672 m. The filling of the Big Lake is then done during the summer 2015 season. It is simulated with synthetic hydrographs corresponding to:

- dry year, based on daily flow with exceedance probability of 80%;

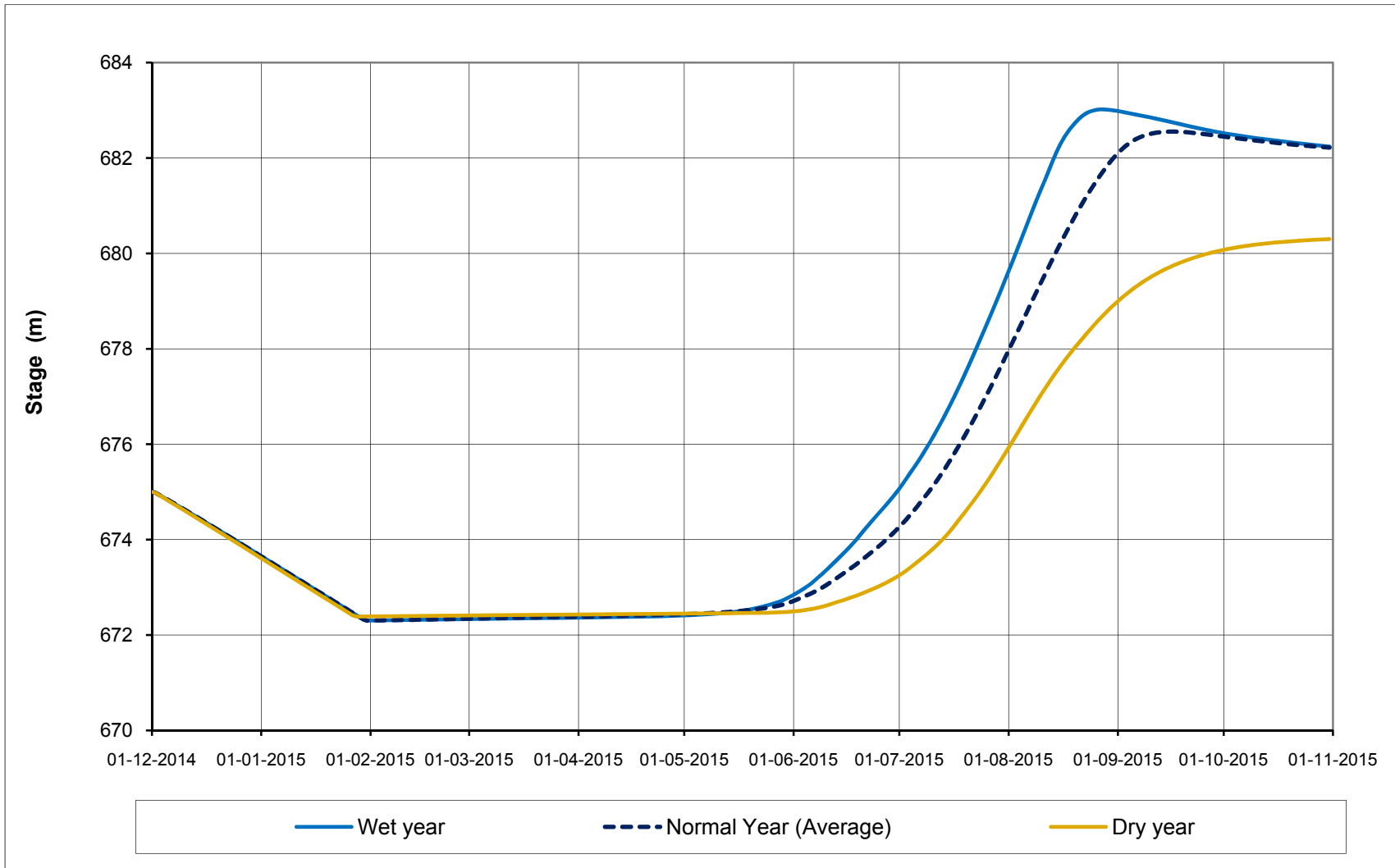
- normal year, based on daily flow average;
- wet year, based on daily flow with exceedance probability of 20%.

The intake gate is closed. Tunnel 1 is closed when lower lake is full. The simulations do not consider the volume that could be used for the commissioning tests of the powerhouse, i.e. additional water would need to be transferred from the Big Lake to the Lower Lake. However, the volume for the tests is planned to be small compared to the available volume (during the remaining of the winter season) and the incoming inflows (during the summer season).

For normal and wet years, the maximum operation level of Big Lake is reached between mid-August and the end of August 2015. In the case of a dry year, the filling of the Big Lake is not completed during the first year: the water level reaches the elevation of 680.3 m but it is expected that this condition will not affect the estimated firm power.

The filling curves of the Big Lake are presented in the Figure 3.8.

Figure 3.8 Filling of Big Lake Reservoir



The Big Lake can be filled during the summer 2015 season, except for a dry year for which the water level would only reach a level near 680 m at the end of 2015. In this case, the normal operations of the powerhouse could still start as the required volume for a reliable power production would be available. The reservoir would reach its maximum operating level during the following summer season. If starting the power simulations that are presented in section 5 of the current report with an initial water of 680 m for the Big Lake, no change is observed in the firm power guaranteed at the site since the inflows are above average for many years at the beginning of the synthetic series.

3.6 Sediments

The potential for sediment transport in the study area is limited since the surface is composed mostly of bedrock. Sand and gravel will be transported by the floods every year but are likely to be deposited at the bottom of the deep lakes used as reservoirs. Sediment deposition will not be an issue at the intake structures since they are planned to be constructed well above the deposition zones at the bottom of the lakes.

4 Power production and installed capacity

4.1 Smelter requirement

The firm power required at the smelter is 650 MW. this power should be guaranteed at all times over a 50 year time frame of operation.

4.2 Firm yield evaluation

For the Greenland Project, hydropower is the only energy source. The generation planning is based on the Smelter energy requirements for the electric power of 650 MW to be provided upon a constant basis, corresponding to 685 MW at both sites combined. The discharge used to generate this power must also be available on an ensured basis. Because of the repartition of the inflows inside a year and their variability between years, storage is needed to regulate the inflows.

The hydroelectric firm energy will be based on the energy output over the most adverse sequence of flows in the inflow series. This adverse sequence of flows is called the critical period.

The evaluation is based upon water resources expected upon the time of operation of the powerhouse: the discharge series used is the projected one for the 2020 horizon. Since the inflows come from numerous catchments at site 6g, the discharges in the series are split according to the percentage of inflows from every sub-catchments obtained from the gauging stations. Only the southeast catchment wasn't gauged, but the inflows from basin can be calculated by subtraction of the other sub-basin inflows from the total discharge for site 6g (for all of the catchments).

4.2.1 Module discharge

The average module discharge for the series is 37.4 m³/s, including the inflows from all of the sub-catchments. The northeast sub-catchment has an average module discharge of 1.9 m³/s while the southeast sub-catchment has an average module discharge of 1.0 m³/s (calculated). Table 4.1 summarizes the average module discharges for the potential development scenarios at site 6g.

Table 4.1 Average module discharges summary

Scenario	Module discharge (m ³ /s)
All catchments	37.4
Big Lake and Lower Lake, with the northeast sub-catchment only (without Tunnel 3)	36.4
Big Lake and Lower Lake, with the southeast sub-catchment only (without Canal 3 and 4, and Dam 5)	35.5
Big Lake and Lower Lake only (without Tunnel 3, Canal 3 and 4, and Dam 5)	34.6

4.2.2 Alternatives considered

Some alternatives were studied in order to optimize the power production at site 6g.

First, the presence of shoals at elevations between 666 and 668 m in the Big Lake upstream of the intake zone of Tunnel 1 limits the use of the full bathymetry of the reservoir, unless extensive dredging works are done. From the available bathymetry, the option to dredge the shoals was eliminated since it would require very costly excavation. The minimum operating level of the Big Lake is then limited to 669 m.

As for the maximum operating level of the Big Lake, preliminary estimations showed that the proposed maximum level of 682 m in FEL 1 was the optimal level to target. Indeed, raising the maximum water level above this value would necessitate raising the dams, which would be too costly compared to the firm power that can be gained.

As for the Lower Lake, its natural water elevation is around 654 m. The proposed operating level is 667 m, which is the water level required to have an adequate submergence of the intake structure with the intake invert above the natural water level of the lake. It will allow simplifying the construction and reducing the costs since no wet excavations will be required.

The operating level of the Lower Lake will be kept constant to maximize the head, since the potential gain in firm power by varying the operating level is minimal (less than 2 MW). Also, the current turbine design wouldn't allow the turbine to adequately operate with a lower water level in certain circumstance. In an emergency case (severe drought that would empty the useful storage in the Big Lake), it would possible to use some of the small storage volume available in the Lower Lake below elevation 667 m to produce additional energy. Nevertheless, the storage volume available is not large enough to increase the firm power at the site on a yearly basis.

The possibility to join the two reservoirs by raising the water level of the Lower Lake to 682 m, and create only one single large reservoir was also considered. Such an option would simplify the operation at the site by eliminating the transfer Tunnel 1 and would slightly increase the storage available. However, those advantages are overruled by the cost required to raise the dams and the spillway of the Lower Lake. This option was then rapidly eliminated.

4.2.3 Minimum and maximum operation level

Based on the above results, the Big Lake will operate between elevation 669 and 682 m, while the Lower Lake is planned to be kept at a constant elevation of 667 m. The useful storage volume at the site will be of 972 hm³ if Tunnel 2 is not constructed (bathymetry of Big Lake only) or 1 122 hm³ if Tunnel 2 is constructed (bathymetry of Big and Middle Lake).

4.3 Power production study

4.3.1 General methodology

The approach used to determine the energy potential of site 6g is the sequential streamflow routing method, which is the most viable method for evaluating storage projects regulated power or for multiple purposes including power.

The method uses the continuity equation to route streamflow through the project, and thus it accounts for the variations in reservoir elevation resulting from water inflow and outflow. The routing is done over the 50 years streamflow series, with a daily step. For a given time period, the water withdrawn is determined in function of generation needs, which are

constant in terms of delivered power. Water spills occur when the water level reaches the weir crest.

The level of firm power is determined by trial and error, and is defined as the output that will utilize the available storage completely once during the period of record.

The firm power is therefore dependant on:

- the drought characteristics;
- the net storage available;
- the net hydraulic head;
- the production device's efficiency.

4.3.2 Model

The simulations were conducted with version 3.0 of the HEC-ResSim model. This model has been designed and developed by the Hydrologic Engineering Center of the U.S. Army Corps of Engineers to perform Reservoir System Simulation's.

The program can be used efficiently for single reservoir or for complete reservoir systems on either critical period or period of record studies. It can handle multi-hourly, daily, weekly, or monthly intervals. It is designed to simultaneously meet flood control criteria and conservation requirements within other operating constraints defined by the user. Conservation requirements can be expressed in terms of seasonal flow requirements or seasonal generation requirements, at specific reservoirs or as seasonal flow requirements at downstream control points. Each demand may be served by one or more upstream reservoirs based upon input data. System operations may be performed for flood control, water supply, and hydropower, where more than one reservoir is operated for a common location.

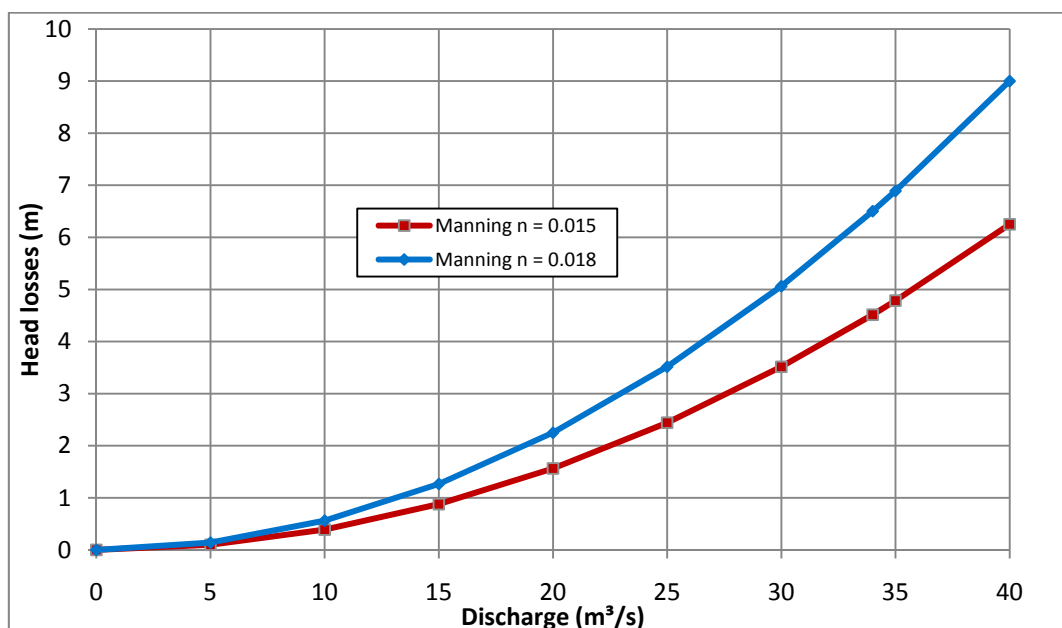
4.3.3 Net head

The gross head is taken between the pond surface and the level of the turbines nozzles, set at an elevation of 7.5 m. Such an elevation is chosen to ensure an appropriate clearance between the maximum tailwater elevation and the level of the flywheel. The clearance is set equal to 1.5 the diameter of the flywheel.

For evaluating the net head, head losses due to the intake structures and the power tunnel are taken into account. The characteristics of the power tunnel are:

- length: 9.99 km
- diameter: 5.1 m
- cross-section shape: Circular (TBM)
- cross-sectional area: 20.4 m²

Head losses - Discharge relationships are presented in Figure 4.1 below for Manning coefficient n equal to 0.015 and 0.018. However, the base case considered is a Manning's n of 0.015, which is likely for construction.

Figure 4.1 Friction head losses

4.3.4 Production devices efficiency

The following parameters were defined from the proposed turbines:

- turbines: 91.9%
- generator: 98.6%
- unit: 90.6%
- global efficient used: 90.1% to take into account possibility for high velocity oxygen fuel (HVOF) coating

4.3.5 Water losses

It is supposed that all of the inflows are reserved for hydropower generation.

Water losses may be of various kinds. In this stage, it is assumed that net evaporation losses are minimal. Preliminary values of head loss resulting from leakage through or around the spillway and the dams are considered. Since the water head at the dams in the Big Lake and Lower Lake don't vary a lot (maximum of 13 m difference), leakage is assumed constant over the whole range of operating level. Values of 0.1 and 0.25 m³/s are considered respectively for the Lower Lake and the Big Lake.

4.3.6 Power simulation results

Power simulations were run using the 2020 projected discharge series previously discusses to determine the firm power available at site 6g.

The base scenario that is currently considered operates the Big Lake between 669 and 682 m, while the water level of the Lower Lake is kept constant at elevation 667 m. The northeast catchment is diverted into the Big Lake with Canals 3 and 4, and Dam 5. Tunnel 2 (to use the storage of the Middle Lake) and Tunnel 3 (to divert the southeast catchment into the Lower Lake) are not constructed with this scenario. Such a scenario yields a firm power of 191 MW.

Additional simulations were run to quantify the possibility to add or eliminate both the northeast and southeast catchments from the project, to increase the firm power or lower the project cost.

The results of those power simulations are presented in Table 4.2, along with the differential construction cost associated with the various sub-components of the project which are considered.

Table 4.2 Minimum reservoir level (with zone near the intake only)

Scenario	Guaranteed power (MW)	Operating discharge (m ³ /s)	Construction cost (M\$)	Unit incremental cost (M\$/MW)
Base – Tunnel 1, Canals 3 and 4	183	32	-	-
With Tunnel 2 (Middle Lake)	186	32	5	1.67
With Tunnel 3 (southeast catchment)	187	32	13	3.25
With Tunnel 2 and 3	191	33	18	2.25
Without Canals 3 and 4, and Dam 5 (northeast catchment)	175	31	8	1.15

From the results presented in the above table, adding the inflows of the southeast catchment is not an interesting option, since the incremental unit cost of this catchment is higher than the unit cost per MW for the project. Moreover, this tunnel located next to the glacier margin presents some clogging risks by ice or snow accumulation which couldn't be economically settled. The northeast catchment has a low incremental cost and should be kept in the base scheme. Tunnel 2 would increase the storage volume with the bathymetry of the Middle Lake, but is not retained in the base scheme.

4.4 Available firm power at smelter location

The electrical power supply to the smelter will be provided by the two powerhouses of site 7e and site 6g.

The operational constraint is that a firm power of 650 MW has to be available anytime at the smelter. The power generated at the generating stations must be sufficient to cover this electricity demand as well as the electricity needs for the powerhouses, the stations service systems, the transmission line and the other different losses.

4.4.1 Station service requirements

Both powerhouses require power to operate, which amounts to a total of 6 MW for both sites. Table 4.3 outlines the needs at the 6g and 7e powerhouses.

Table 4.3 Power station energy requirements

Component	Site 7e (MW)	Site 6g (MW)
Powerhouse	1.6	1.3
Service Station	1.3	1.3
Intake structure	0.25	0.25
<i>Total</i>	<i>3.15</i>	<i>2.85</i>

4.4.2 Power losses

According to the information supplied by EFLA (transmission line subcontractor), the power losses through the transmission lines are estimated to a total of 22 MW for both sites combined. Finally, a 1% loss in the transformer is considered, which amounts to 7 MW for both sites combined.

4.4.3 Power needed at the generating stations

The power to be generated at sites 7e and 6g is estimated to 685 MW on a firm basis, as shown in Table 4.4.

Table 4.4 Total power production needs

	Power (MW)
Smelter requirement	650
Station service system	6
Transport losses	22
Generator losses	7
<i>Total</i>	<i>685</i>

5 Design criteria and assumptions

5.1 Purpose and Scope

This part of the design criteria defines the general and technical requirements for the following civil works:

- main water intake structures;
- power tunnel;
- powerhouse including service bay;
- transformer cavern;
- cable gallery
- access galleries;
- spillway.

5.2 Stability analysis

Classical stability analysis for the water intake structure, and dams is required.

5.3 Design criteria for tunnels

5.3.1 Geometry of the excavations

The following criteria regarding the geometry of the openings are considered when using drill and blast technique for the excavations:

- tunnel have a D-reverse shape;
- height to width ratio of the cross-sections of the power tunnels is at 1.3;
- the arch depth is equal to 25 to 30% of the width of the tunnel;
- the rock pillar between two galleries and/or the rock cover over a galleries is at least 1.5 times the width of the opening;
- the Manning's friction coefficient considered is $n = 0.033$ for head loss calculations;

Steel lining is placed in the tunnel near the intake and concrete is poured between the rock and the steel lined section. Elsewhere, the tunnels are unlined.

When a TBM is used for the excavations, the design criteria, with regard to the geometry of the openings, are the following:

- tunnels have a circular cross-section;
- the Manning's friction coefficient considered is $n = 0.015$ for head loss calculations;

Steel lining is placed in the tunnel near the intake and concrete is poured between the rock and the steel lined section. Elsewhere, the tunnels are unlined.

5.3.2 Excavation methods-Special requirements and restrictions

Well controlled drilling and blasting methods are generally used in all underground excavations to obtain relatively smooth, stable excavation rock faces with a minimum of overbreak and requiring minimum scaling and support.

Generally, no excavation sequences or restrictions on methods are imposed that would tend to reduce the contractor's flexibility in planning and add to his costs. However, special requirements are imposed in some zones considered critical and where a greater degree of assurance in the final results of excavation is needed.

In general, the excavation begins with a pilot tunnel on the first 10 meters, followed by slashing to the line within which excavation must be completed. The pilot gallery is excavated so that a layer of rock 2.5 m minimum thick is left in place inside the required excavation line of the walls and the arch of the tunnels. The maximum length of round for the pilot gallery generally does not exceed 2.5 m. Initially, the centre to centre spacing of the perimeter holes for the pilot tunnel is 60 cm. This spacing could be modified depending on the quality of the walls obtained and as to maintain the tolerances which will be specified in the technical specifications.

Pilot tunnel slashing as well as full face heading excavation shall be done using controlled perimeter blasting. Only cartridge type explosive will be used in the perimeter blast holes and in the buffer zone. Benching excavation in tunnel shall have the following specifications:

- maximum height of a bench: 10 m;
- maximum length of a bench: 10 m;
- maximum hole diameter of the perimeter, buffer and production holes: 70 mm;
- initial spacing of the perimeter holes (unless specified otherwise): 0.60 m c/c;
- loading of the perimeter holes: max. 0,65 kg/m² of presplit surface excluding bottom load;
- bottom load of the perimeter holes: 1.25 kg/hole.

Borehole grid in frozen rocks should be reduced and the explosive ratio increased compared to the same rocks in a thawed state.

5.4 Design Codes and Standards

European standards (EN/ENV) with Norwegian design guidelines shall apply.

Data processing, design and fabrication shall conform to the requirements of European Committee for Standardization (CEN) codes and standards.

Where an applicable EuroNorm (EN) or EuroNorm Vornorm (ENV) is not available, an appropriate ISO standard, ASTM or other internationally recognized standard may be utilized upon prior approval.

The following European standards (EN/ENV) are the principal standards, codes, guidelines and references to be used for the structural design:

The latest edition of a code or standard shall govern.

- EN 2004 Eurocode - Basis of structural design
- ENV2009 Eurocode 1- Actions on structures
- ENV2007 Eurocode 2- Design of concrete structures
- ENV2009 Eurocode 3 - Design of steel structures (both heavy and light gage)
- ENV1996 Eurocode 6 - Design of masonry structures
- ENV2009 Eurocode 8 - Design of structures for earthquake resistance

The following is the list of other codes to be used in the calculations (the latest edition of the following codes or standards shall govern).

- Rules BAEL 91, modified 99
- UK National Annexes to Eurocodes
- BS 8500-1: Concrete - Complementary British Standard to BS EN 206-1
- BS EN 10080: Steel for Reinforcement of Concrete - Weldable reinforcement steel
- BS EN 206-1: Concrete - Specification, performance, production and conformity
- UK National Application Document for Steel Structures
- BS 4449-2005: Specifications of concrete steel bars for reinforcement of concrete
- ONGC 41-GP-35M, type 2: waterstop in PVC (polyvinylchloride)
- USACE: Conduits, Culverts and Pipes, EM 1110-2-2902, Engineering and Design, March 1998
- USCAE: Shore protection Manual, Waterways Experimental station. Coastal Engineering research Center, 1984. (for tidal waves)
- Byngnings reglement 2006 - Greenland Building Code to be checked by Greenland Engineering Consultant
- Greenland specific Standard for Concrete - to be checked by Greenland Engineering Consultant

5.5 Material Properties

5.5.1 General

- Concrete :
 - Mass 2 500 kg/m³
 - Compressive strength at 28 days - cylinder 30 or 40 MPa
 - Concrete/rock adherence coefficient, c 300 – 1 000 kPa
 - Concrete/concrete friction coefficient 1.0
 - Thermal expansion coefficient 10x10⁻⁶/°C
- Lean concrete : compressive strength at 28 days - cylinder 15 MPa
- Porous concrete : compressive strength at 28 days - cylinder 10 MPa
- Reinforcing steel :
 - Mass 7 850 kg/m³
 - Yielding strength of regular rebars 500 MPa
- Compacted backfill :
 - humid density of sand and gravel 2 000 kg/m³
 - humid density of rockfill 2 000 kg/m³
 - saturated density of sand and gravel 2 150 kg/m³
 - saturated density of rockfill 2 200 kg/m³
 - concrete/rockfill coefficient of friction 0.70
 - active coefficient(sand and gravel) : K_a 0.33
 - at-rest coefficient (sand and gravel) : K₀ 0.50
- Rock:
 - allowable bearing pressure 1 000 – 4 000 kPa
- Rock/concrete: friction coefficient 0,85

Concrete cover:

- Concrete exposed permanently to soil and water (normal) : 60 mm
- Concrete exposed permanently to soil and water (minimum) : 50 mm
- Concrete not exposed permanently to soil and water (normal) : 40 mm
- Concrete not exposed permanently to soil and water (minimum) : 30 mm

5.5.2 Concrete

Greenland standard requirements

The classification of concrete classes is as per EN 206. The following strength classes are to be used:

Table 5.1 Concrete Class

Characteristic Compressive Strength f_{ck} at 28 days (cylinder) [MPa]	Usage
30	Structural concrete
15	Mass concrete, concrete plugs

5.5.3 Reinforcing Steel

Materials and workmanship shall comply with ENV 13670

Reinforcement shall be uncoated grade B500A or B500B, with characteristic yield strength of 500 MPa conforming to ENV 10080, except stirrups and ties, which shall be grade B500C, conforming to NS/Euro Standards.

5.5.4 Steelwork

5.5.4.1 Greenland standard requirements

Following are additions and modifications from Greenland and Danish standards.

The following material properties conforming to European standards are to be used:

Table 5.2 Structural Steel

Mechanical characteristics (t - steel thickness in mm)	Type		
	S.235	S.275	S.335
Elasticity limit f_y (Mpa)			
$t \leq 16$	235	275	355
$16 \leq t \leq 40$	225	265	345
$40 \leq t \leq 63$	215	255	335
Tensile resistance f_u (Mpa)			
$t \leq 3$	360/510	430/580	510/680
$3 \leq t \leq 100$	340/470	410/560	490/630
Minimum (average) elongation ϵ (%)			
$t \leq 3$	18	15	15
$3 \leq t \leq 100$	23	19	19

5.5.5 Bolts

Because bolts DIN931 cannot be tensioned on-site and thus are susceptible of stripping the threads, the bolts that will be used in the steel work are to be A325 or A490. These shall be used for all main site bearing-type and moment connections. Main connections shall include beam to beam, beam to column, column splices, bracings and all beams carrying non-vibrant equipment machines. The minimum size of bolts shall be M20.

5.6 Design loads

The codes and standards to be used in assessing dead and imposed loads are as listed under Section 5.4. Structures shall be designed for the worst-case loading combination.

5.6.1 Dead loads

Dead loads shall be calculated from the unit weights given in NS 3491-1: or from the actual known weights of the materials used. Where there is doubt as to the permanency of dead loads, such loads shall be treated as imposed loads. The self weights of the materials to be used are as follows:

- mass of reinforced concrete: 2 500 kg/m³
- mass of steel: 7 850 kg/m³

5.6.2 Hydrostatic pressure

The hydrostatic pressure is calculated from the upstream water level.

5.6.3 Wind and snow loads

The characteristic windload is independent of the surrounding terrain and height. For Maniitsoq the Annex specifies a windload of 1.2 kN/m², but because of the exposure of the Smelter plant, a load of 1.6 kN/m² is to be used (this value shall be used at the settlement Kangaamiut, which also is exposed to the ocean)

For snowloads the base values are influenced by the slope of the roof:

- characteristic load for slopes below 15° is $s_k = 1.8 \text{ kN/m}^2$
- characteristic load for slopes steeper than 15° is $s_k = 0.9 \text{ kN/m}^2$

5.6.4 Live loads (Imposed Loads)

Imposed loads consist of variable and/or transient load (operating or maintenance conditions, occupancy and/or due to storage of materials imposed on a specified area and/or on structural elements). Imposed load does not include the weight of fixed equipment, piping etc.

The design imposed floor loads shall be shown both in the calculations and on the design drawings. The minimum imposed floor loads shall be:

Table 5.3 Power House Floor loads

Specific using of areas	qk (kN/m ²) uniform	Qk (kN) concentrated
Generator floor and machine hall (excl. generators hatch covers)	50	9
Turbine floor	15	9
Bus bars tunnel	24	-
Other transversal tunnels	10	-
Scroll case, access floor and turbine pit	15	-
Penstock	10	-

Specific using of areas	qk (kN/m ²) uniform	Qk (kN) concentrated
Water intake crest - uniform loading	20	-
Water intake crest - Mobile crane	-	413
Water intake crest - load of a stabilizer	-	860/0.4 m ²

Table 5.4 Service Bay Floor Loads

Specific using of areas	qk (kN/m ²) uniform	Qk (kN) concentrated
Floor load	15	-
Oil hall - shells	10	196
Battery hall	35	-
Generator floor:		-
floor load	75	
the rotor	-	2 550
the wheel		510
Cone, support, pivot, winnowing circle		196

Table 5.5 Transformer, ventilation area and miscellaneous

Specific using of areas	qk (kN/m ²) uniform	Qk (kN) concentrated
Transformers area	15	1 275
Trackway downstream	20	To be confirmed
Ventilation area	10	-
Stairs, halls and interior pedestrian bridges	5	2.5

5.7 Crane and lifting appliance loads

Crane and monorails shall be designed in accordance with ENV 1993-6:2002 or where information is not available use AISC publication "Report #13". The crane classification, loads and dynamic effects shall be confirmed by the crane supplier. Crane and other lifting appliance vertical static loads shall be as specified by the manufacturer

Horizontal loads caused by off-vertical lifting shall not be less than 0.10 times the hoisted load.

Static vertical deflection of cantilever beams shall be evaluated allowing for rotation of the beam at the support.

Fatigue shall be checked in accordance with ENV 1993 and shall be based on the relevant number of cycles applicable to the beam or to the detail being designed and shall take into account the fabrication details of the beam and its components

The increase to be applied to the specified vertical static loads for cranes and other lifting appliances shall be calculated in accordance with code or as recommended by the manufacturer but shall be not less than the following:

Table 5.6 Impact

Loading case	Electric operation	Hand operation
Vertical loads – increase static wheel loads by	25%	10%
Horizontal force transverse to rails taken as percentage of wheel load	10%	5%
Horizontal force along rails taken as percentage of static driving wheel load	10%	5%

5.8 Load combination factors and crack width

Partial load safety factors for global analyze:

Table 5.7 Load combinations

Nb. of variable actions	ELU	ELS
1	$1.35 G_{\max} + G_{\min} + 1.5 Q$	$G + Q$
More than 1	$1.35 G_{\max} + G_{\min} + 1.35 \Sigma Q_i$	$G + 0.9 \Sigma Q_i$

Partial safety coefficients for materials properties:

Table 5.8 Safety factor

Combination	Concrete γ_c	Reinforcing steel γ_s
Fundamental	1.5	1.15
Accidental (without seism)	1.35	1.00

For the hydrostatic load the following factors shall be used:

- 1.25 for the max operating level. (MOL)
- 1.15 for the max flood level.

For crack control the following max crack width shall be used:

- 0.40 mm interior exposure.
- 0.33 mm exterior exposure.
- 0.28 mm exposure to water, structure under bending.
- 0.23 mm exposure to water, structure under tension.

5.9 Stability analysis

Classical stability analysis for the water intake structure, spillway and dams is required.

6 Works description

6.1 General layout

At site 6g, power will be produced from the underground powerhouse located near sea level at the end of a Godthabsfjord. The rapidly rising topography near the fjord made this site a logical choice like site 7e, with a large head available for power production

The proposed project layout takes advantage of the presence of two main lakes: the lake Tussaap Tasia (labeled Lower Lake) where the intake for the power tunnel is constructed and to the north, lake Imarsuaq (labeled Big Lake). The Big Lake is at a higher elevation than the Lower Lake, with a smaller lake (labeled Little Lake) in between.

The Big Lake will constitute the main storage for the installations and will be regulated, while the elevation of the Lower Lake will be kept constant to maximize the head. It is planned to operate the Big Lake between elevation 669 and 682 m, while the Lower Lake will be at a constant level of 667 m. Both lakes will be connected with Tunnel 1 that will transfer water from the reservoir (Big Lake) towards the intake structure. Two small canals (1 and 2) will be excavated downstream of the tunnel outlet, to ensure an adequate flow depth and eliminate the risk of freezing of the water passage during the winter season. The inflows from an adjacent subcatchment will also be diverted into the Big Lake with transfer structures, i.e. canals 3 and 4, along with Dam 6 used to close the natural outlet of this catchment.

The Big Lake will be closed by Dams 3 and 4, and Spillway 2, while the Lower Lake will be closed with Dams 1 and 2 and Spillway 1. A 10 km long power tunnel will connect the intake structure in the Lower Lake with the powerhouse.

The project will necessitate the construction of approximately 47 km of roads to access the various structures. The main road is the one that will climb the fjord from sea level to reach the intake structure in Lake Tussaap Tasia (Lower Lake). A water route through the Big Lake will also be used to reach the natural outlet of the lake to the north, where Dam 4 and Spillway 2 will be constructed, and the structures used to divert the adjacent catchment (Dam 5 and Canals 3 and 4).

6.2 Dams and Spillways

6.2.1 Dams and spillway locations

The topography and geology of site 6g gives very limited alternatives for dam alignments. In fact, no alternative sites have been considered for dam locations, which still the same as in previous studies. On the other hand, different locations of the spillways were considered in order to minimize concrete volume according to available information.

Dams 1 and 2 and Spillway 1 insure closure of the 6g lower reservoir. Dam 1 is located at the outlet of Lake Tussaap Tasia while Dam 2 is positioned approximately 300 m North-East of Dam 1. Spillway 1 is located about 4 km North of Dam 1 in a narrow valley having a thalweg at level 664 m. In order to minimize its concrete volume, the alignment of Spillway 1 is subject to slight changes upon reception of more information on the bedrock topography following additional investigations.

According to the available topography, an additional dam (identified Dam 7 in previous studies) is not required at a high valley located approximately 1.8 km South-East of Spillway 1. At this valley (\pm 538 150 E, 5 203 650 N), the ground level (>670 m) exceeds the maximum extreme level (668.3 m) of the 6g lower reservoir on a distance of over 250 m. The bouldery till and rock outcrop observed in this valley should impede the groundwater flow from the reservoir sufficiently over this distance.

Dams 3 to 5 and Spillway 2 insure closure of the 6g upper reservoir. Dam 3 is situated between Imarsuaq Lake and Little Lake. In combination with gated Tunnel 1, Dam 3 allows the desired water level control between the upper and lower reservoirs. This dam comprises two segments separated by a bedrock outcrop over a distance of approximately 70 m.

Dam 4 is located on the outlet river of Lake Imarsuaq located at its North-East extremity, about 200 m downstream of the natural sill of the lake. Despite located at a thalweg level about 7 m lower than the upstream natural sill, the selected site for Dam 4 offers a much more interesting topography. This dam comprises two parts slightly separated by a bedrock outcrop. On the left abutment, Dam 4 is terminated on a bedrock outcrop which is not more than 1.5 m lower than the crest elevation but higher than the maximum extreme level of the upper reservoir. This later bedrock outcrop separates Dam 4 from the adjacent Spillway 2 located across a small saddle valley. Spillway 2 is positioned at a location that should minimize its concrete volume according to available information. However, Spillway 2 alignment is still subject to slight changes upon reception of information on the bedrock topography following further investigations.

Dam 5 is located approximately 750 m North-East of Dam 4 on the outlet river of the Northeast sub-catchment. The selected alignment corresponds to the site where the topography clearly minimizes the dam volume.

6.2.2 Hydraulic design

6.2.2.1 Spillways capacity

Two spillways are projected for Site 6g:

- Spillway 1, to evacuate floods at Lower Lake;
- Spillway 2, to evacuate floods at Big Lake.

These spillways are weirs with uncontrolled crest, with discharge given by the equation:

$$Q = C_d \times L \times H \sqrt{2gH}$$

Where:

Q = discharge, in m³/s

C_d = variable discharge coefficient

L = effective length of crest, in meter

H = actual head on the crest

The crest elevations of the spillways correspond to the maximum operation level of the reservoirs, which are 667 m at Lower Lake and 682 m at Big Lake. The characteristics of the spillways are presented in Table 6.1.

Table 6.1 Characteristics of the spillways

	Crest elevation (m)	Length
Spillway 1	667	56
Spillway 2	682	72

The discharge coefficient, C_d , depends of the type of the weir, the approach height of the weir, and the head on the crest. The type of spillway considered is trapezoidal. For the design flood, corresponding to the 1:10 000 years flood, the discharge coefficient C_d is 0.41. This coefficient varies from 0.34 to 0.41 depending on the hydraulic head. The discharge capacity curve that is considered for the spillway is presented in the Figure 6.1 and Figure 6.2 hereafter.

Figure 6.1 Spillway 1 stage-discharge curve

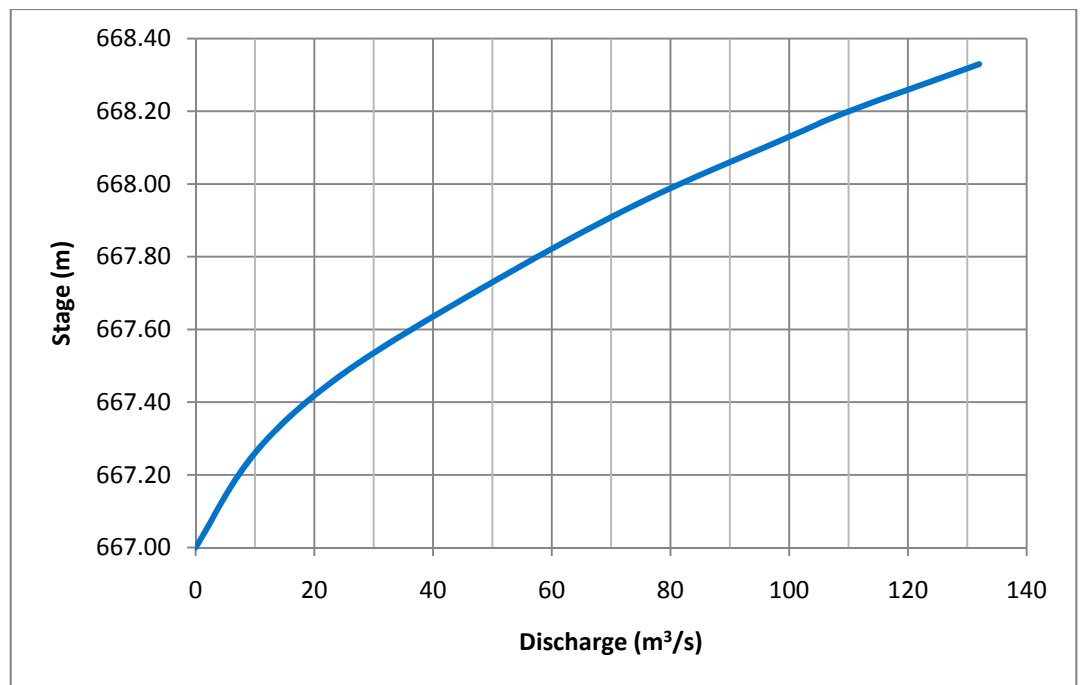
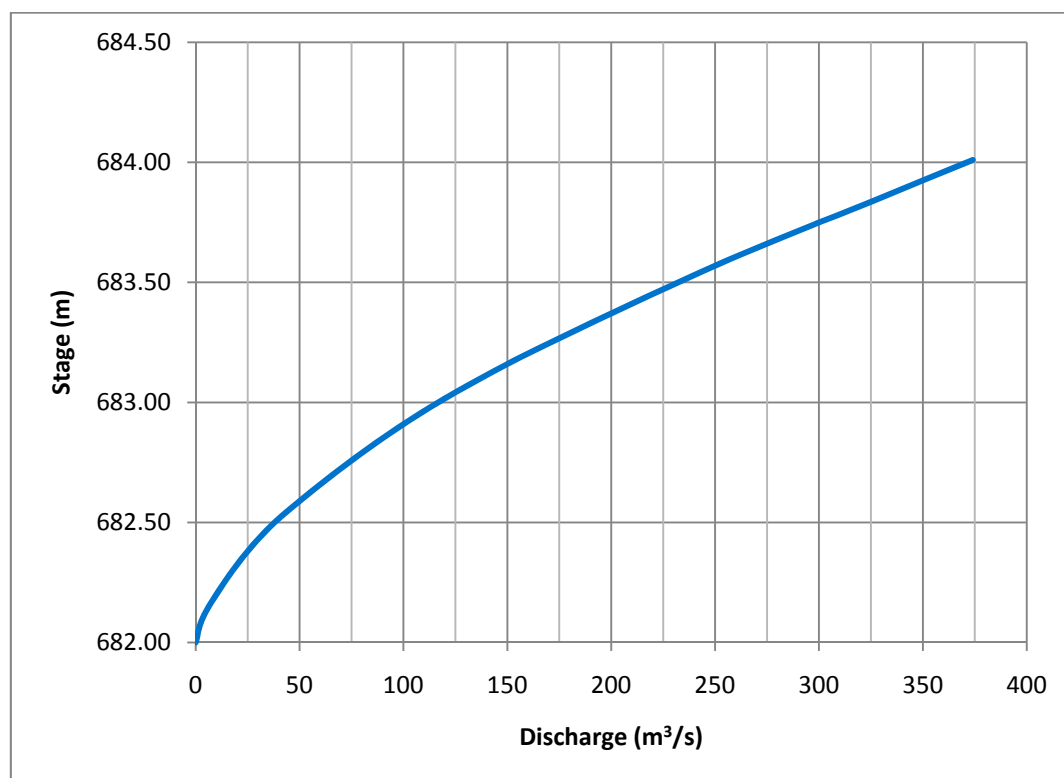


Figure 6.2 Spillway 2 stage-discharge curve

6.2.2.2 Water level during floods

Flood routing has been considered with the hydrographs presented in section 4, for the recurrence intervals 10 000, 1 000, and 100 years. At Big Lake, the calculations begin with a full reservoir (water level at 682 m) at the end of the previous water season: water is withdrawn for power generation during the winter, which lower the level of the reservoir before the arrival of the flood. At Lower Lake, the initial elevation at the arrival of the Flood is the maximum operation level (667 m). When the reservoir is taken full at the arrival of the flood with the powerhouse stopped, water level becomes of about 0.1 m.

The maximum water levels for the different floods are presented in the Table 6.2 and 6.3

Table 6.2 Lower Lake Reservoir (maximum operating level at 667 m)

Return period	Max inflow* (m³/s)	Max outflow at spillway (m³/s)	Max water level (m)
1 :10 000 years	167	132	668,33
1 :1 000 years	159	124	668,29
1 :100 years	151	117	668,24

* Include potential inflows from Tunnel 3

Table 6.3 Big Lake Reservoir (maximum operating level at 714 m)

Return period	Max inflow (m ³ /s)	Max outflow at spillway (m ³ /s)	Max water level (m)
1 :10 000 years	400	327	683.8
1 :1 000 years	354	244	683.5
1 :100 years	303	147	683.1

6.2.2.3 Freeboard and dam crest elevation

The normal freeboard is defined as the difference in elevation between the crest of the dam and the maximum operation level of the reservoir. Minimum freeboard is defined as the difference in elevation between the crest of the dam and the maximum reservoir water elevation that would result from routing the inflow design flood through the reservoir.

Both normal and minimum freeboard requirements should be evaluated to determine the required crest elevation. The freeboard resulting in the higher crest elevation is adopted for design. It is unlikely that maximum winds will occur when the reservoir surface is at its maximum elevation resulting from routing of the maximum design flood. Computations should incorporate the probability of pool level wind and appropriate durations.

The criteria used here to determine the crest elevations are:

- Criteria 1: Minimum Freeboard of 1.5 m above 1:10 000 years flood level;
- Criteria 2: 1:1 000 years flood level and wave run-up of 1:100 years winds;
- Criteria 3: Maximum operation level and wave run-up of 1:1 000 years wind;
- Criteria 4: Level of PMF.

In this stage, PMF has not yet been evaluated. At the moment, the spillway is designed to provide sufficient capacity to pass twice the routed 1:10 000 years flood below the crest of the dams.

The wave heights and freeboard calculations for the dams use the guidelines provided by different references including: Coastal Engineering Research Center of the United States (1984), SEBJ Guide for Rip Rap sizing (1996), Canada Dam Association, USBR Acer and ICOLD - selection of design floods.

The wind data used in the analysis come from the meteorological stations Sioralik (04242) and Nuuk (04220). Available data are maximum wind speeds (10 minutes average), without distinction of the direction. The wind characteristics are presented in the Table 6.4.

Table 6.4 Frequency of wind speed

Return period	10 minutes wind speed		Hourly wind speed	
	m/s	km/h	m/s	km/h
1:100 years	42	151	40	144
1:1 000 years	49	176	47	168

Preliminary estimations of wave heights are done with these winds for maximum fetches.

The wave run-ups, dam crest elevation and freeboards are presented in the Table 6.5 below.

Table 6.5 Crest elevation and freeboard of the dams at site 6g

	Lower Lake (Spillway 1)		Big Lake) (Spillway 2)		
	Dam 1	Dam 2	Dam 3	Dam 4	Dam 5
Max Operation Level	667.0	667.0	682.0	682.0	682.0
1:10 000 years flood	668.3	668.3	683.8	683.8	682.2
1:1 000 years Flood	668.3	668.3	683.5	683.5	682.2
1:1 000 years wind run-up	3.5	3.4	2.3	1.1	1.2
1:100 years wind run-up	2.9	2.8	1.9	0.9	1.0
Crest elevation	671.5	671.5	685.5	685.5	685.5
Minimum Freeboard (1:10 000 years flood)	3.2	3.2	1.7	1.7	3.3
Freeboard for 1:1 000 years flood	3.2	3.2	2.0	2.0	3.3
Normal Freeboard (Maximum operating level)	4.5	4.5	3.5	3.5	3.5

When the water level reaches the crest of the dams, the discharges are:

- for the Spillway 1 : 1 060 m³/s
- for the Spillway 2: 1 002 m³/s

These values are higher than 2 times the routed 1: 10 000 years flood at the structures.

6.2.2.4 Riprap protection

The riprap protecting the embankments resists the impact of waves by their own weight. The minimum and maximum weights are calculated with the following equations.

$$W_{min} = \frac{\rho_r H_s^3}{K(S_r - 1)^3 \cot g \alpha}$$

$$W_{max} = 3 W_{min}$$

With:

- W_{min} : Minimum weight of the riprap, kg
- W_{max} : Maximum weight of the riprap, kg
- ρ_r : Specific mass of the riprap, kg/m³
- ρ_w : Specific mass of water, kg/m³
- S_r : Riprap density
- $\cot g(\alpha)$: Embankment slope
- K : Stability factor

The parameter K can take the following values:

- K= 3.5 for the 1:1 000 years wave (acceptable damage)
- K= 1.75 for the 1:100 years wave (no damage)

The minimum thickness for the riprap is equal to 2.5 times the minimum diameter.

The preliminary riprap designs for the dams are presented in the Table 6.6.

Table 6.6 Riprap size for the dams of Site 6g

		Dam 1	Dam 2	Dam 3	Dam 4	Dam 5
W min	kg	820	730	200	30	40
D min	mm	800	800	500	300	300
D max	mm	1100	1100	700	400	400
Minimum thickness	mm	2000	2000	1250	750	750

6.2.3 Typical dam cross sections

6.2.3.1 Asphaltic concrete core rockfill dam

The dam type selected for dams 1, 2, 4 and 5 is an asphaltic concrete core rockfill dam (ACRD). Considering the arctic conditions and the scarcity of soils and the short period of time where unfrozen soils maybe found only over small depths makes very difficult the option of building earth dam. The ACRD's have proven to be economical and reliable and show excellent performance in all cases. Furthermore, the asphaltic concrete core construction offers interesting flexibility with respect to weather conditions; it should be interrupted only during heavy rain and can restart as soon as the rain stops. The placement of the asphaltic concrete core can be conducted without problem at temperatures slightly below 0°C. However, under colder temperatures special measures such as the insulation of the asphaltic concrete transportation/storage facilities and heating of aggregate are likely to be needed in order to respect the hot temperature criteria (140 to 155°C) required for placement of the asphaltic concrete core.

All ACRD's are to be founded on bedrock. The only exception is for part of Dams 1, 4 and 5 sited over the river where it is judged acceptable to leave in place the overburden (assumed thaw-stable material) present beneath the dam (upstream cofferdam) outside the limits of the 1H:1V slopes from the crest.

For all ACRD's, the width of the asphaltic concrete core is equal to the standard minimum value of 0.4 m. The asphaltic concrete core is made of crushed stone aggregate with a maximum size of 16 to 18 mm, containing 12% of filler and mixed with about 7% (by weight) of bitumen.

A 4 m wide concrete plinth connects the asphaltic concrete core to the bedrock foundation and serves as a grouting cap. The thickness of the concrete plinth varies according to rock surface topography and an average value of 0.55 m had been considered for cost estimate.

The width of the crest of the ACRD's is fixed at 6 m and the slopes of the upstream and downstream faces are respectively of 1.5H:1V and 1.4H:1V. The crest elevation of ACRD's varies according to water levels and waves as determined in the hydraulic design section. The crest elevation and length, the top elevation of the asphaltic concrete core, the maximum height and the total fill volume of each dam are presented in Table 6.7. The top of the impervious asphaltic concrete core is set at levels which are 0.5 m above the applicable extreme maximum level (1:10 000 years).

Table 6.7 Dams characteristics

	Crest elevation (m)	Top elevation of impervious element (m)	Total crest length (m)	Maximum height ⁽¹⁾ (m)	Total fill volume ⁽²⁾ (m ³)
Dam 1	671.5	669.0	295	21	78 400
Dam 2	671.5	669.0	485	15	97 300
Dam 3	685.5	684.5	480	17	120 000
Dam 4	685.5	684.5	170	18	40 200
Dam 5	685.5	684.5	285	28	123 000

⁽¹⁾ According to available subsurface information and assuming a 2 m thickness of overburden at rivers location.

⁽²⁾ Excluding the cofferdams volumes when present.

The asphaltic concrete core (zone 5) and the adjacent support/filter zones (2B) are all placed simultaneously by the specialized paving machine. The total width of these zones consequently depends on the width of the machine, which typically varies from 3.5 to 4.0 m. For all ACRD's, the combined width of the asphaltic concrete core and the support/filter zones had been fixed to 4.0 m. The support/filter material is made of crushed stone, max. diameter 60 mm.

Transition zones (3E) made of crushed stone, max. diameter 225 mm are placed next to the upstream and downstream support/filter zones. A random rockfill, max diameter 900 mm (zone 3D) completes the body of the dams upstream and downstream of the transition zones. For construction purposes, the width of the transition zones (3E) is set to 3.0 m. For the same reason, the random rockfill zones (3D) are stopped at the level where their width is equal 3.0 m in the upper part of the dams. At these locations, the transition zones (3E) are extended upstream and downstream.

An appropriate riprap (zone 4) is placed on the upstream face of the each dam up to the crest. In the case of Dams 1 and 2, the minimum level of the riprap is set 4.0 m (twice the height of the significant wave) below operating level of the lower reservoir (667.0 m). For Dam 4, the minimum level of the riprap is set 2.0 m below the crest elevation of the upstream cofferdam (kept in place after construction) since its presence constitutes an effective protection of the lowest part of the dam. In the case of Dam 5, the minimum level of the riprap is set 1.5 m (twice the height of the significant wave) below the minimum operating level of the upper reservoir. The crest elevation and length, the top elevation of the geomembrane liner, the maximum height and the total fill volume of Dam 3 are presented in Table 6.7.

For Dams 1 and 2, an additional zone (3F), made of selected rockfill, max. diameter 450 mm is required to support the upper part of the 800 to 1 100 mm rockfill riprap (zone 4) where the random rockfill (zone 3D) is not present. For Dams 4 and 5, the 300 to 400 mm rockfill riprap (zone 4) is compatible with the transition zone (3E) and there is no need of an additional zone in the upper part of these dams.

6.2.3.2 Lined Dam

The dam type selected for Dam 3 is a lined rockfill dam. As for the other dams, earth dam is not considered feasible for Dam 3 given the arctic conditions and the scarcity of soils, despite less rare in this area. For Dam 3, an ACRD appears less attractive in reason of the high mobilization cost of an asphalt plant compare to its relatively small asphaltic concrete core volume that would be required and its isolation. In theses conditions, a lined rockfill dam seems appropriate for Dam 3.

Given the observation of stepping permafrost (or solifluction phenomenon) at Dam 3 site, it is considered that the overburden material is not thaw-stable and the whole dam should consequently be founded on bedrock. The body of the dam consists in a random rockfill, max. diameter 900 mm (zone 3D) with a geomembrane liner located near the upstream face of the dam with appropriate protection and transition materials. On each side of the geomembrane, a geotextile protects the liner from puncture by the adjacent crushed stone, max. diameter 20 mm (zone 3C). Between this later cushion zone and the adjacent coarser materials, a transition made of crushed stone, max. diameter 225 mm (zone 3E) is required. This transition zone supports the 500 to 700 mm rockfill riprap (zone 4) and separates the random rockfill (zone 3D) of the dam body from the cushion material (zone 3C) placed below the geomembrane liner.

The geomembrane liner is connected to the bedrock foundation by the means of a narrow concrete sill casted into a cut-off trench (see also the foundation treatment section). The geomembrane liner is attached to the concrete sill with a watertight anchorage. Upstream of the concrete sill, the rock excavation is filled with low permeability till material to impede the flow through the bedrock foundation.

To decrease the riprap volume, it is projected to backfill the upstream toe of the dam with a random fill up to the initial natural ground level. Similarly, it is planned to backfill the downstream toe of the dam with a random fill up to the initial natural ground level in order to avoid the creation of a small pond at this location and to facilitate the dam access for inspection.

Different types of geomembrane could be selected for Dam 3 liner. According to current project status, it is not required to install the geomembrane liner under cold winter conditions. Consequently, it is not necessary to select particular geomembrane liner (such as "arctic liner" or "low temp PVC") suitable for installation during very cold temperatures that are generally more expensive. Considering its relative low cost, ease of welding and seams testing, a textured high density polyethylene (HDPE) geomembrane appears to be one of the most interesting liner for Dam 3. According to the information available, the friction angle between HDPE textured geomembrane and geotextile (product's specific) is so that the upstream slope of the dam maybe 2.0 to 2.5H:1V in order to insure its stability. At this stage of the project, the upstream slope of Dam 3 is set at 2.5H:1V.

The downstream face of the dam corresponds to a 1.4H:1V slope. Given the riprap thickness, the minimal 2.5 m width of zones 3E and 3C located upstream of the textiles and the geomembrane (for construction purposes) and the working surface required for the geomembrane liner installation, the crest width of the dam should be nearly 9 m.

6.2.4 Foundation treatment

The nature and extent of the foundation treatment of the dams are based on the existing data obtained from the previous field investigations: topography, geological/geotechnical conditions (soil and rock formations) and permafrost characterization. Since the requirements for foundation treatment depends on dam type, they are separated in the two following sections according to the typical cross sections selected. The foundation treatment for the spillways is presented in a third section.

6.2.4.1 Asphaltic concrete core rockfill dam

The selection of the typical cross section of asphaltic concrete core rockfill dam (ACRD) for Dams 1, 2, 4 and 5 reduces the extent of the foundation treatment compared to an earth dam. In fact, the foundation treatment is essentially concentrated below the concrete plinth which is 4 meters wide. Outside the concrete plinth area, rockfill and crushed stone are placed directly on the rock surface after stripping and removal of all organic materials,

without any special treatment. As mentioned previously, there is exceptions to that: assuming the presence of thaw-stable material, it is judged acceptable to leave in place the overburden present beneath Dams 1, 4 and 5 (upstream cofferdams) outside the limits of the 1H:1V slopes from the crest.

Table 6.8 shows the design criteria selected for the foundation treatment in the concrete plinth area. There are two main types of treatment: rock excavation and vertical curtain grouting. As the grouting cannot be conducted in frozen rock (defrosting required), the excavation of the top layer of rock surface, where rock is generally altered and more fractured, is prioritized. Therefore, the need to proceed with grouting operations, which besides being time consuming and considerably increasing construction costs, is greatly reduced. The choices made offer the advantage to treat the foundation adequately therefore reducing the need for further work after construction. However, in the worst case scenario, should this need arise (important water infiltrations through the foundation after reservoir impoundment), the ACRD offers the possibility to realize grouting after construction.

Table 6.8 Foundation treatment in the concrete plinth area

Water head, Hw (m)	Depth of rock excavation (m)*	Depth of curtain grouting (m)* (Holes 3 m c/c)	Length (m)				Total (m)	%
			Dam 1	Dam 2	Dam 4	Dam 5		
Hw < 5	1	0	78	287	90	98	553	45
5 < Hw < 15	2	0	185	199	28	131	543	44
15 < Hw < 25	5	0	0	0	0	7	7	1
Hw > 25 and at river bed area (including 6m on each side)	2	Hw / 3 (min 8 m)	32	0	52	52	136	10
<i>Total</i>			<i>295</i>	<i>486</i>	<i>170</i>	<i>288</i>	<i>1239</i>	<i>100</i>

Dams 1 and 2: constant operating level = 667 m, Dams 4 and 5: maximum water operating level = 682 m

* For preliminary estimation. To be revised according to further investigations and observations during construction.

This approach is valid with the assumption made that rock is sound and of good quality and it is mainly fractured and altered only in the first few (2-3) meters from the surface. The exact depth of the excavation however will be determined by senior geologist on the site once the overburden is excavated and the rock surface is cleaned by high air pressure.

In order to minimize the formation of addition cracks in the foundation bed, rock excavation in the concrete plinth area should be done using controlled drilling and blasting techniques with reduced charges.

If frozen rock is encountered (permafrost or active layer frozen when works to be conducted) prior to grouting, the foundation has to be thawed by injecting warm water (hydro-defrosting) or steam (steam defrosting) in holes drilled at the required depth for grout curtain. Defrosting holes has to be drilled upstream and downstream of the grout curtain line (at less 2 meters) to allow the grout to penetrate the surrounding rock mass. After grouting, all holes must be backfilled from the bottom up with a 0.74:1 water-cement grout by volume. Field tests and ground temperature monitoring are required to verify the effectiveness of both defrosting and grouting methods applied.

The presented criteria and associated quantities are used at this stage mainly for cost estimates. Although the table gives the impression that the decision making depends entirely on the water head of the dams, rock quality will govern the final decisions during

construction. Based on the preliminary 2009 investigation results, those criteria and associated quantities should be maintained. However, this should be later reassessed following any subsequent investigation.

It was determined from the 2007/2008 geological mapping that there are typically three orthogonal sets of joints in the rock throughout site 6g, one near horizontal and the other two near vertical. The predominant joint set is aligned with the foliation and tends to be steeply deeping (more than 45°). This conclusion was based only on surface geological mapping with no boreholes done in the dam site areas. Therefore, the presence of sub horizontal stress relief joints, result of elastic rebound following glaciations, typical for the northern hemisphere, will be reassessed when the 2009 investigation results will be fully integrated in the report. Should the subsequent investigations prove otherwise, other types of measures may be considered in order to reduce water losses through the foundation without treatment, such as the addition of an upstream till blanket over the exposed bedrock and a downstream reverse filter consisting of granular material (crushed stone) outside of the steep valleys.

6.2.4.2 Lined rockfill dam

Dam 3 requires different foundation treatment since it is a lined rockfill dam. In addition, Dam 3 has the particularity that leaking water through it and its foundation are not losses from power production point of view since the leakage water would reach the lower 6g reservoir. In this context, the foundation treatment at Dam 3 is limited.

The foundation treatment consists essentially in a small excavation (cut-off trench) into the most weathered bedrock at the upstream toe of the dam in order to connect the geomembrane liner to sufficiently competent rock and to increase the length of the seepage path. Based on the available information, the excavation into the most weathered bedrock is assumed to be in average 2.0 m deep. According to the borehole drilled at Dam 3, this depth maybe reduced. This assumption should be review according to further investigations. However, the exact depth of the excavation will be determined during construction once the overburden will be excavated and the rock surface cleaned by high air pressure.

For construction needs, it is required for the cut-off trench excavation into bedrock to be formed of straight segments at the upstream toe of the dam. Consequently, the geomembrane liner will be placed in contact with the rock surface on a distance that depends on the rock surface topography variation. In order to offer a suitable foundation for the geomembrane liner at this location, it is assumed that this rock surface will need to be treated. The treatment consist in small rock excavation and lean concrete placement in order to offers a smooth surface and avoid puncturing the geomembrane liner.

6.2.4.3 Spillways

The spillways are founded on bedrock of Good (Spillway 1) to Fair (Spillway 2) quality. The rock at Spillway 1 is mainly sound gneiss with consistently spaced joints, the rock at Spillway 2 is mostly greenschist (a metamorphosed basic igneous rock which owes its color and schistosity to abundant chlorite). It was assumed, in both cases, that at less one meter of rock on the surface has to be excavated (after removal of all boulders, top soil and cleaning by high air pressure), as to remove all fractured and weathered rock as well as for correction of unfavorable foundation slopes.

6.2.5 Spillway characteristic

Two spillways will be constructed at site 6g, Spillway 1 for the Lower Lake and Spillway 2 for the Big Lake.

Spillway 1 will close the Lower Lake to elevation 667 m on its western side. The spillway is a 50 m long concrete weir with a maximum height of 6 m. The crest of the weir has a trapezoidal profile.

Spillway 2 will close the Big Lake at elevation 682 m near Dam 4. The spillway is a 72 m long concrete weir with a maximum height of 7 m. The crest of the weir also has a trapezoidal profile.

The design flood for the spillways is the 1:10 000 years flood as mentioned in section 7.2.2.

6.2.6 Temporary works

6.2.6.1 Generalities

In order to build the dams in the dry, cofferdams and diversion works are required. At Dam 1 site, a diversion tunnel is projected at the right abutment to evacuate the water from adjacent Tussaap Tasia Lake during construction. An upstream cofferdam is necessary at Dam 1 while nearby Dam 2 does not require any cofferdam since located where ground level is above the 1:20 years water level during construction.

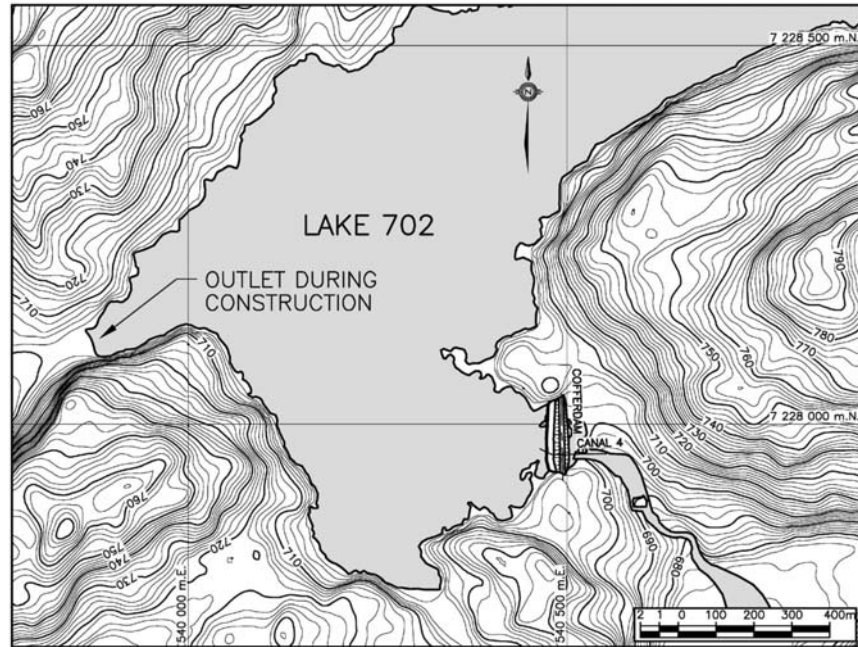
Spillway 1 is also located where ground level is above the 1:20 years water level of Tussaap Tasia Lake during construction and consequently does not require any cofferdam. However, dewatering works will be necessary for the Spillway 1 in reason of the presence of a small pond that intersects its alignment.

Dam 3 is located on a dry site where ground level is above the 1:20 years water level of the upstream Imarsuaq Lake during construction and consequently does not require any cofferdam.

At Dam 4 site, the water control during construction is realized by a combination of an upstream cofferdam and a diversion tunnel located on the right abutment to constitute an outlet for the adjacent Imarsuaq Lake. Spillway 2, adjacent to Dam 4, is located where ground level is above the 1:20 years upstream water level during construction and consequently does not require any cofferdam.

For Dam 5 construction, two small upstream cofferdams are projected. A first cofferdam is positioned at the entrance of Canal 4 in order to temporary divert the water of Lake 702 watershed (northeast sub-catchment) to a secondary natural outlet located South-West (see Figure 6.3). A second upstream cofferdam is required just upstream of Dam 5 in order to protect the site from flooding by inflows coming from the small intermediate catchment between Canal 4 and Dam 5. The inflows from this catchment may be controlled by pumping.

Figure 6.3 Temporary water diversion at Lake 702 (northeast sub-catchment)



6.2.6.2 Diversion tunnel at Lower Lake

The diversion works are designed for the 1:20 years flood during the construction period.

The diversion will be done by way of a tunnel built at Dam 1 location. The diversion tunnel will have an estimated length of 100 m with a reverse-D shaped cross-section. The base width will be 5 m with a maximum height of 6 m, for a cross-sectional area of 27 m². The invert of the tunnel will be respectively of 652 and 642 m at its upstream and downstream ends.

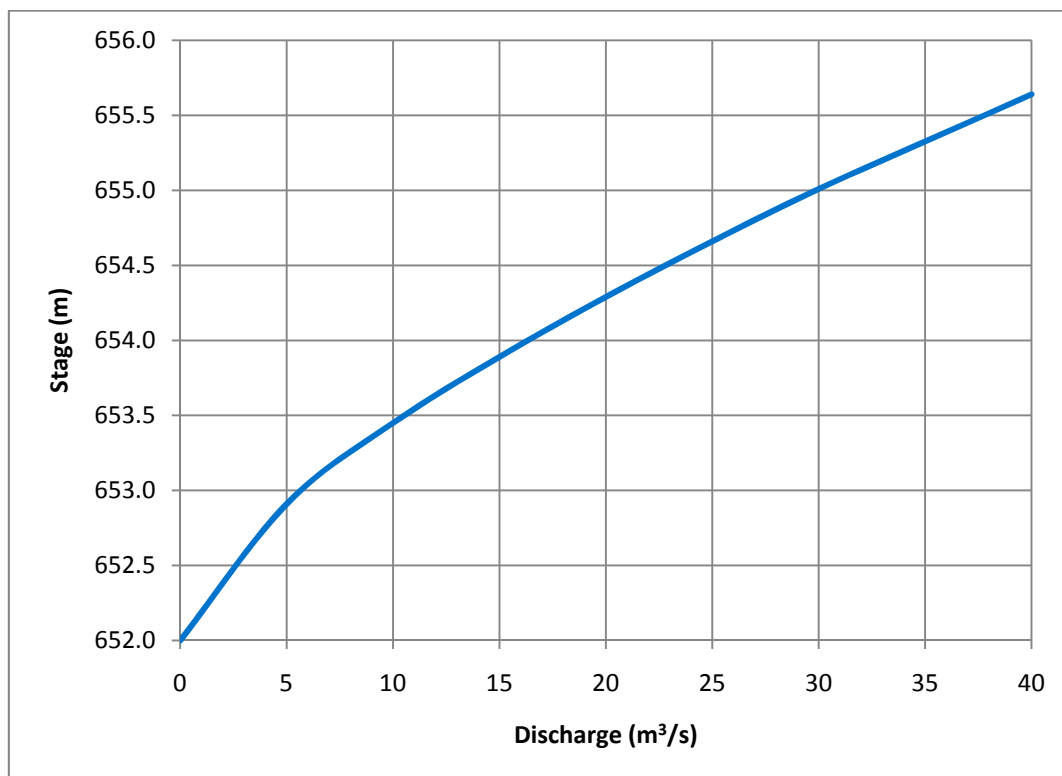
The natural outlet of Lower Lake is located at the site of Dam 1. In natural conditions, the water levels of the lake are controlled by this outlet, which is at an estimated elevation of 652 m according to the topography measured at the site. The capacity of the diversion tunnel will govern the water level in the lake during construction.

The Table 6.9 summarizes the water levels in the Lower Lake during the construction phase. A stage-discharge curve of the diversion tunnel at Dam 1 is presented in the Figure 6.4 hereafter.

Table 6.9 Site 6g – Water levels at Dam 1 site

	Discharge (m ³ /s)		Water levels during construction (m)
	Inflow	Tunnel outflow	
May average discharge	1	1	652.17
July average discharge	17	17	654.05
1: 20 years flood	40	34	655.28*

Figure 6.4 Stage-discharge curve of Diversion Tunnel of Lower Lake



6.2.6.3 Cofferdam at Dam 1 and diversion tunnel cofferdam

At Dam 1, the upstream cofferdam is positioned to be integrated as much as possible to the body of the dam. However, a minimal distance of 10 m is kept between the downstream toe of the cofferdam and the estimated limit of the cut-off trench excavation under the concrete plinth of the dam in order to limit the risk of conflict at this location.

From downstream to upstream, the upstream cofferdam comprises the following zones:

- a random rockfill, max. diameter 900 mm (zones 3 and 3D) that constitutes the body of the cofferdam;
- a transition zone made of crushed stone, max. diameter 225 mm (zone 3A);
- a geotextile filter;
- a low permeability till core (zone 1) that reaches the bedrock into a previously excavated cut-off trench;
- a random rockfill, max. diameter 900 mm (zone 3D) which protects and confines the till zone.

The crest elevation of the upstream cofferdam is set with a 2.0 m freeboard relative to the maximum water level during construction with a return period of 20 years⁵.

At the end of the construction of Dam 1 (Dam 2 being also complete or sufficiently advanced), a cofferdam is required at the entrance of the diversion tunnel located on the

⁵ The water levels during construction indicated on the drawings had not been updated according to the latest hydrologic data.

right abutment of Dam 1 for the construction of a concrete plug into this temporary tunnel. This cofferdam consists in dumped random rockfill, max. diameter 900 mm (zone 3) with a low permeability dumped till (zone 1A) upstream of it that reaches the bedrock. A geotextile filter separates the two fill zones. The crest elevation of the diversion tunnel cofferdam is set 1.5 m above the estimated level of Tussaap Tasia Lake resulting from water accumulation without outlet from December to mid-May.

6.2.6.4 Diversion tunnel at Big Lake

The diversion works are designed for the 1:20 years flood during the construction period. The diversion tunnel will be build at Dam 4 Location. The diversion tunnel will have an estimated length of 120 m with a reverse-D shaped cross-section. The base width will be 8 m with a maximum height of 7 m, for a cross-sectional area of 50 m². The invert of the tunnel will be respectively of 670 and 668 m at its upstream and downstream ends.

The natural outlet of Big Lake is located upstream of the proposed site of Dam 4. In natural conditions, the water levels of the lake are controlled by this outlet, which is at an estimated elevation of 674 m according to the bathymetry measured at the site. During the construction, it is estimated that the natural outlet will control the water levels in the lake up to a discharge of 90 m³/s. For higher discharges, the capacity of the diversion tunnel will govern water level in the Big Lake.

Stage-discharge curves of the Big Lake and the diversion tunnel at Dam 4 are presented the Figure 6.5 hereafter.

The Tables 6.10 and 6.11 summarize the natural hydraulic conditions at the sites of Dam 3 and Dam 4 and the expected conditions during the construction works (with the upstream cofferdams and the diversion tunnel in operation). It is expected that no downstream cofferdams will be required at both site.

Figure 6.5 Stage-discharge curve of Diversion Tunnel of Big Lake

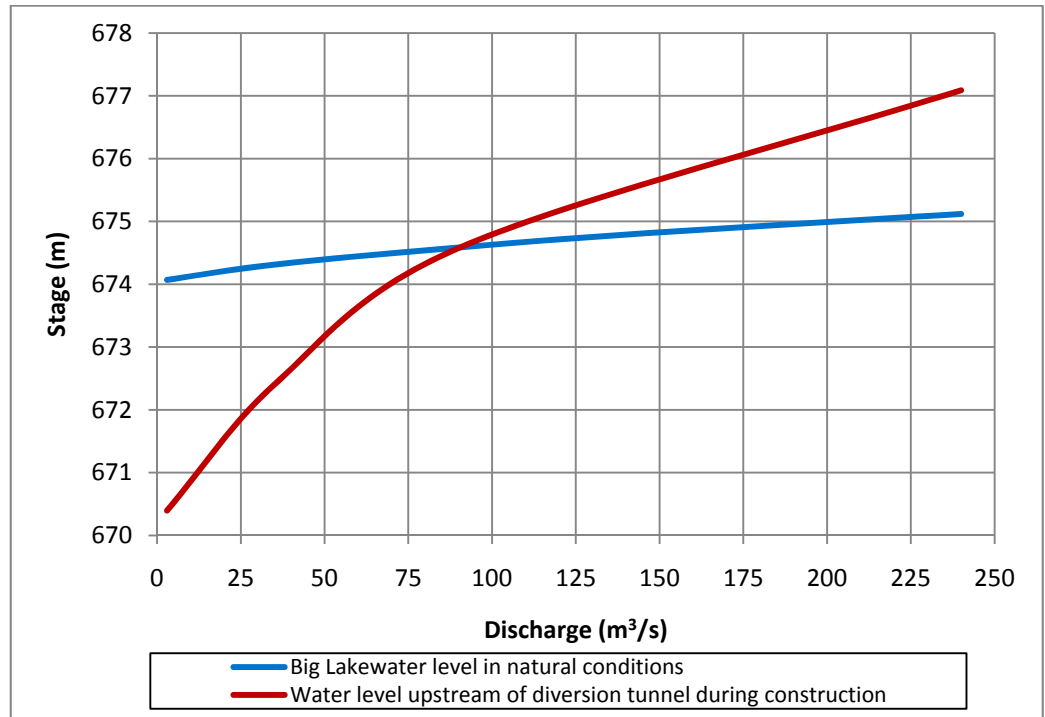


Table 6.10 Site 6g – Water levels at Dam 4 site

	Discharge (m ³ /s)		Water levels in natural conditions (m)	Water levels during construction (m)
	Inflow	Tunnel outflow		
May average discharge	3	3	669.25	670.40
June average discharge	36	36	669.89	672.45
August average discharge	91	91	670.61	674.60
1: 20 years flood	240	178	671.83	677.16

Table 6.11 Site 6g – Water levels at Dam 3 site

	Discharge (m ³ /s)		Water levels in natural conditions (m)	Water levels during construction (m)
	Inflow	Tunnel outflow		
May average discharge	3	3	674.07	674.07
June average discharge	36	36	674.32	674.32
August average discharge	91	91	674.59	674.59
1: 20 years flood	240	178	675.12	677.16

6.2.6.5 Cofferdam at Dam 1 and diversion tunnel cofferdam

At Dam 4, the upstream cofferdam is positioned to be integrated as much as possible to the body of the dam. However, a minimal distance of 10 m is kept between the downstream toe of the cofferdam and the estimated limit of the cut-off trench excavation under the concrete plinth of the dam in order to limit the risk of conflict at this location.

From downstream to upstream, the upstream cofferdam comprises the following zones:

- a random rockfill, max. diameter 900 mm (zones 3 and 3D) that constitutes the body of the cofferdam;
- a transition zone made of crushed stone, max. diameter 225 mm (zone 3A);
- a geotextile protection;
- a geomembrane liner that reaches the bedrock into a previously excavated cut-off trench;
- a confining material at the toe of the cofferdam made of crushed stone, max. diameter 20 mm (zone 3B).

This later 3B zone is required over the geomembrane at the toe of the cofferdam and on the abutments to confine the liner and to push it against the bedrock surface.

As shown on the drawings, a working platform made of random rockfill, max. diameter 900 mm (zone 3), is assumed to be required for the cut-off trench excavation, the geomembrane liner installation and the placement of the 3B zone confining material.

Different type of geomembrane maybe considered as the liner of the cofferdam. One of those is the EPDM liner, a synthetic rubber waterproofing membrane made of ethylene – propylene – diene terpolymer (also mixed with carbon black, oils, curing agents and others). This type of geomembrane has a high flexibility even at very low temperatures (down to –45°C) and is available in large seamless panel sizes resulting in fewer field joints made by wide overlaps. Theses properties make this inexpensive type of geomembrane very interesting for the cofferdams liner.

The crest elevation of the upstream cofferdam is set with a 2.0 m freeboard relative to the maximum water level during construction with a return period of 20 years.

At the end of the construction of Dam 4 (Dam 5 being also completed or sufficiently advanced), a cofferdam is required upstream of the diversion tunnel located on the right abutment of Dam 4 for the construction of a concrete plug into this temporary tunnel. In order to reduce its height and facilitate its construction, this cofferdam is positioned at the natural sill of Imarsuaq Lake located about 200 m upstream of Dam 4. The diversion tunnel cofferdam consists in dumped random rockfill, max. diameter 900 mm (zone 3) with a low permeability dumped till (zone 1A) upstream of it that reaches the bedrock into a previously excavated cut-off trench. A geotextile filter separates the two fill zones. The crest elevation of the diversion tunnel cofferdam is set 1.5 m above the estimated level of Imarsuaq Lake resulting from water accumulation without outlet from December to mid-May.

6.2.6.6 Dam 5 diversion works

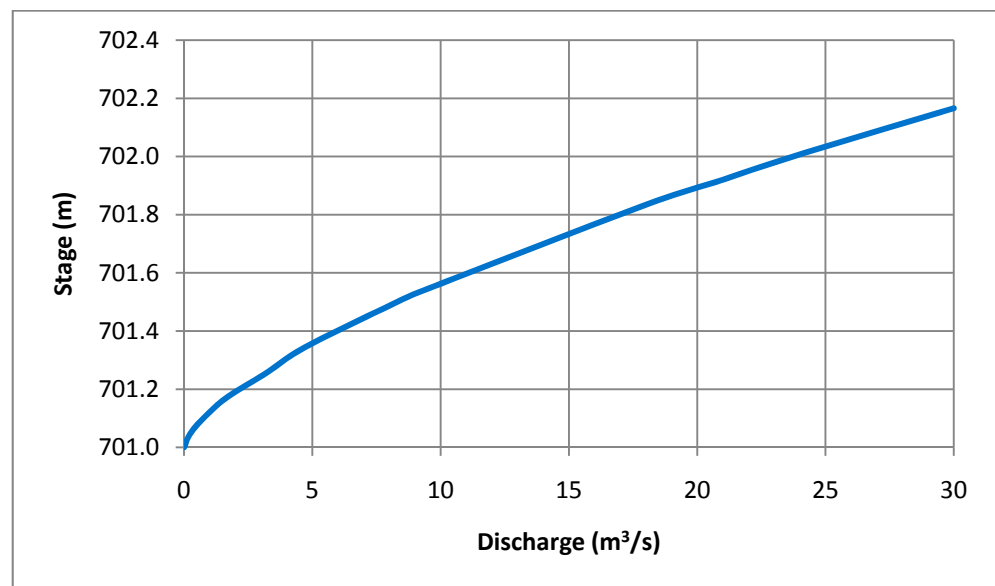
The water flowing from the northeast sub-catchment will be diverted into the Big Lake with Canal 3, Canal 4 and Dam 5. For the construction of those structures, a cofferdam will be built upstream of Canal 4 to divert the flow toward a second natural outlet. The low point of this outlet upstream of Canal 4 is measured at 701 m from the available topographical maps. A stage-discharge curve of the outlet is presented in the Figure 6.6 hereafter.

Table 6.12 summarizes the water levels expected at the cofferdam upstream of Canal 4 during construction.

Table 6.12 Site 6g – Water levels at cofferdam upstream of Canal 4.

	Discharge (m ³ /s)	Water levels during construction (m)
July and August average discharge	9	701.53
1: 20 years flood	21	701.92

Figure 6.6 Stage-discharge curve of the natural outlet upstream of Canal 4



The natural outlet at the site of Dam 5 is at elevation 660 m (measured from the topographical maps) and is lower than the invert of canal Canal 3. Dam 5 will then be built behind an upstream cofferdam. The inflow from the secondary basin (between the cofferdam upstream of Canal 4 and the site of Canal 3 and Dam 5) is evaluated to about 0.4 m³/s for a return period of 20 years. This discharge can be diverted by pumping or by way of a culvert.

6.2.6.7 Dam 5 cofferdams

As mentioned previously, two small upstream cofferdams are projected for Dam 5 construction. The first one is positioned at Canal 4 entrance located about 1.5 km upstream of Dam 5. The second cofferdam is situated just upstream of Dam 5 and partially integrated into the dam body.

From downstream to upstream, both upstream cofferdams comprise the following zones:

- a random rockfill, max. diameter 900 mm (zone 3) that constitutes the body of the cofferdam;
- a transition zone made of crushed stone, max. diameter 225 mm (zone 3A);
- a geotextile protection;
- a geomembrane liner that reaches the bedrock into a previously excavated cut-off trench;
- a confining material at the toe of the dam made of crushed stone, max. diameter 20 mm (zone 3B).

This later 3B zone is required over the geomembrane at the toe of the cofferdam and on the abutments to confine the liner and to push it against the bedrock surface. As mentioned previously, among the various geomembrane type that could be used as cofferdam liner, the EPDM liner appears to be one of the most interesting.

The crest elevation of the upstream cofferdams is set with a 2.0 m freeboard relative to the maximum waters level during construction with a return period of 20 years.

6.3 Canals and Tunnel 1

6.3.1 Tunnel 1

Tunnel 1 is a gated tunnel which transfers from the Imarsuaq Lake (Big Lake) to the Tussaap Tasia Lake, the inflows needed for generation at the powerhouse.

It is designed to pass the discharges of 35 m³/s in the worst conditions, that is during winter and when the Big Lake approaches its minimum operation level 669 m.

The Tunnel 1 has a length of 1 690 m, with a reverse-D shaped cross-section. The invert of the tunnel is sub-horizontal at the elevation of 660 m. The base width will be 5.00 m with a maximum height of 6.25 m, for a cross-sectional area of 29.4 m². The Manning coefficient used is 0.033.

The entrance channel is about 90 m long and has a slope of 10%. Its upstream sill is at the elevation of 665 m.

6.3.2 Canals 1 and 2

Canal 1 and Canal 2 are excavations required between Little Lake and Lower Lake, to insure appropriate conveyance of the flows from the Tunnel 1 to the powerhouse intake. The criteria used to design these canals are:

- flow during winter, with an ice cover of 2 m thickness;
- discharge of 35 m³/s;
- velocity limited to 0.65 m/s.

The bottom width of Canal 1 and Canal 2 is set to 20 m. The excavations are done for each channel with an elevation of 663 m for the upstream invert, and 662 m for the downstream invert.

During construction, shallow water conditions will permit to easily handle those excavations underwater.

6.3.3 Canal 3

Canal 3 is primarily planned to transfer water coming from the Northeast catchment to Big Lake. The average flows are in the range of 0.2 m³/s (in winter months) to 9.0 m³/s (in July or August). It is also designed as channel spillway for Dam 3, with a design discharge of 28 m³/s.

Canal 3 is 5 m wide at its bottom⁶ and about 725 m long. The level of the bottom of Canal 3 varies between 680 m at the entrance to 679 m at the exit. According to available geological information, both extremities of Canal 3 are sited entirely on overburden while the bedrock excavation could reach about 18 m in height into its middle portion. In order to avoid erosion problems, it is planned to protect the overburden excavated faces (canal bottom and slopes) with random rockfill up to level 684 m (2.0 m above maximum normal water level into the canal). Since the bottom of Canal 3 is entirely above the maximum water level of the adjacent water bodies during construction, it can be excavated in the dry without cofferdam.

6.3.4 Canal 4

Canal 4 is located at the main outlet of Lake 702, about 1.5 km upstream of Dam 5. It is excavated to force the total flow from the Northeast catchment toward the Big Lake. It is designed to transfer the discharge of the 1:10 000 years recurrence estimated to 28 m³/s, with a 2 m freeboard at the secondary outlet, located about 1.3 km northwest.

During Dam 5 construction and prior Canal 4 excavation, the flow through this main outlet is blocked by a cofferdam in order to divert water from Lake 702 to the secondary outlet of the lake. In case that this secondary outlet reveals to be deepened by erosion during the temporary diversion, the Canal 4 should be accordingly adjusted to insure the withdrawal of water from the northeast catchment as desired.

The downstream part of Canal 4 could be excavated in the dry in presence of the cofferdam required for Dam 5 construction while its upstream part could be easily excavated under shallow water when dismantling this later cofferdam.

⁶ This is the minimal width according to hydraulic needs. Excepted for its 2.0 m lowest part, the canal maybe widened into bedrock for construction purpose and as rockfill source.

6.4 Rock excavation and reinforcement

6.4.1 Open cut rock excavation

Well controlled drilling and blasting methods are used in all open-cut excavations to obtain relatively smooth, stable excavation rock faces with a minimum of overbreak and requiring minimum scaling and support.

Generally, no excavation sequences or restrictions on methods are imposed that would tend to reduce the contractor's flexibility in planning and add to his costs. However, special requirements are imposed in some zones considered critical and where a greater degree of assurance in the final results of excavation is needed.

Consequently, standard clauses covering definitions, special and performance requirements will be included in the technical specifications. The essential of these requirements is given below:

- use of controlled perimeter drilling and blasting, techniques such as presplitting, cushion blasting, smooth blasting and line drilling;
- reduction of presplitting hole spacing (usually 0.60 m c/c) in zones considered critical;
- maximum height of a bench: 10 m. The average slope of the final wall consisting of several benches should be vertical;
- diameter of perimeter; buffer and production holes: 70 to 100 mm max;
- maximum weight of explosive per delay period, including controlled perimeter blasting: 150 kg;
- installation of preset grouted rocks dowels before blasting to reinforce the periphery of some particular areas;
- seismic monitoring.

According to the study done by Bertsov et al. (1980), borehole grid in frozen rocks should be reduced by 13% and the explosive ratio increased by 28% compared to the same rocks in a normal (thawed) state.

In areas where concrete will be placed against the rock surfaces, financial penalties for excessive overbreak will be included in the technical specifications (for example, penalty will be applied when overbreak exceeds 0.15 m on walls and 0.30 m on horizontal surfaces).

Scaling, rock reinforcement and surface production, if needed, have to be done as soon as access to a freshly blasted face is available and before the subsequent blast.

6.4.2 Underground excavation-Excavation methods and sequence

6.4.2.1 Powerhouse complex

The excavation of the powerhouse complex including all access tunnels, penstocks, powerhouse cavern, transformer chamber, tailrace gallery and cable tunnel is achieved by using the drill and blast method. These excavations occur during a period of 18 months. This is achieved by using several access galleries allowing simultaneous progress on three headings at any time. Indeed, other than the main permanent access tunnel to the powerhouse cavern there are three additional accesses: one to the transformers cavern, a second one to the tailrace tunnel and a third one to the power tunnel.

The excavation begins from the access road close to the harbor with the simultaneous open-cut excavation of the two portals: access tunnel to the powerhouse and cable tunnel. Depending on the size of the galleries, once the excavation of the tunnel portals is completed, progress is made by full face heading or pilot tunnel breakthrough followed by slashing and bench excavation.

The excavation of the transformer chamber is firstly completed, and then followed by the simultaneous excavations of the access to the tailrace tunnel and the tailrace tunnel itself. The excavation of the powerhouse cavern begins after the access to the tailrace tunnel is completed.

The excavation of the transformer chamber will be done by excavating a pilot tunnel to clear the arch, followed by lateral slashing until the final limits of the walls. One bench will complete the excavation.

The excavation of the tailrace tunnel is done using both ends (2 headings) one from the access to the tailrace tunnel, a second one from the outlet. This excavation is realized simultaneously with the excavation of the powerhouse cavern and the cable tunnel.

Once the crown level of the powerhouse cavern reached, the excavation will be done in two phases:

1. excavation of the central section of the arch (approximately 1/3 of the full width) until approximate elevation of 24 m and subsequent lateral slashing on each side of the opening until the final limits of the walls;
2. benching excavation until reaching the button of the powerhouse cavern.

More details on some special requirements and restrictions to be included in the technical specifications are presented in the section "Design criteria for tunnels".

6.4.2.2 Power tunnel

The excavation of the power tunnel is achieved using a TBM's (Tunnel Boring Machine) allowing completion of the excavation in 17 months, excluding the time of delivery and erection of the TBM. Excavation begins at PM $\pm 10+800$ and it is completed at PM $\pm 0+150$.

6.4.3 Rock reinforcement and surface protection

The objective of rock reinforcement and surface protection is to ensure the security of personnel and equipment as well the stability of excavated or natural rock faces. Rock reinforcement consist mainly of grouted rock bolts and preinstalled grouted rock dowels. Shotcrete will be also used in zones of very fractured and altered rock, or fault and shear zones. If necessary, shotcrete with a welded wire mesh will be applied to increase the stability of the rock mass. Steel ribs will only be used if required in rock of very poor quality.

As a general approach to rock support, basic reinforcement is provided firstly by using pattern bolting for critical areas such as tunnel portals, vaults of large permanent underground openings and in enlarged intersections on the access galleries. This pattern bolting and other reinforcement material planned in advance will be shown on the drawings. Supplementary support and surface treatment elsewhere in the excavations will be determined as work advances. The extent of rock support will depend both on local geology and the degree of success of drilling and blasting methods used by the Contractor.

Based on the literature review of case studies of powerhouse projects in cold climates, the rock was found to be stable when frozen but when thawed, could not be maintained safely in an unsupported condition. Thawed rock required temporary lining of the arches.

In order to estimate the quantities of reinforcement material required, consolidation criteria were established depending on the rock quality. Those criteria are shown on Table 6.13 for both methods of excavation, Drill and Blast and TBM. Rock is classified using the Rock Mass Rating (RMR) method developed by Bieniawski, 1989. As the excavation progressed, the rock formations will be classified accordingly and the type of reinforcement will be determined as to meet the existing geological conditions. Moreover, Wedge analysis using version 3 software will be used to determine the reinforcement required in case of the formation of large wedges of rock.

Surface protection during the excavation will be provided by using flexible chain link mesh. The wire mesh is installed systematically as work progresses, generally after each blast. In open-cut excavations, the wire mesh is installed on all rock walls higher than 3 meters. In underground excavations, the wire mesh is installed on the entire vaults until the face of the excavation.

Table 6.13 Site 6g – Consolidation criterias

Rock class	Distribution %	Drill and blast				TBM			
		Rock bolts	Shotcrete (m ³)	Wiremesh (m ²)	Steel sets (m)	Rock bolts	Shotcrete (m ³)	Wiremesh (m ²)	Steel sets (m)
Class 1 RMR : 81-100	75	Occasional (1 rock bolt / linear meter) 2.5 m long	Local application 75mm (15% of the crown)	On crown	---	Occasional (1 rock bolt / linear meter) 2.5 m long	Local application 50 mm (15% of the crown)	Occasional (15% of the crown)	---
Class 2 RMR : 61-80	15	Pattern 2.25 m c/c 2.5 m long	Local application 75mm (15% of the crown+ walls down to 3 m from the floor)	On crown	---	Occasional (1 rock bolt / linear meter) 2.5 m long	Local application 50 mm (15% of the crown)	Occasional (15% of the crown)	---
Class 3 RMR : 41-60	7	Pattern 2.0 m c/c 3.0 m long	100 mm on 50% of crown + walls down to 3 m from the floor	On crown	---	Pattern 2.5 m c/c 2.5 m long	50 mm on crown	Occasional (15% of the crown)	---
Class 4 RMR : 21-40	2.5	Pattern 1.5 m c/c 4.0 m long	100 mm on 100% of crown 50 mm on 30% of the walls down to 3 m from the floor	On crown	6" on crown and walls spaced 1.5 m when required	Pattern 2.0 m c/c 3.0 m long	100 mm on crown and 50 mm on walls	On crown	Light ribs spaced 2 m when required
Class 5 RMR < 20	0.5	Pattern 1 m c/c 5 m long	100 mm on 100% of crown 50 mm on 30% of the walls down to 3 m from the floor	On crown	Heavy ribs spaced 0.75 m	Pattern 1.0 m c/c 4.0 m long	150 mm on crown and walls	On crown and walls	Medium to heavy ribs spaced 1.0 m
Adits + access tunnels		---	Local application 100 mm (15% of the crown+ walls down to 3 m from the floor)	On crown + walls down to 3 m from the floor	---	---	---	---	---

6.5 Excavation slopes in overburden

The slopes required to insure the stability of excavations in overburden greatly depends on the properties of soils and conditions associated to thawing of permafrost soils which may include ice lenses. Considering this later unfavorable condition associated to permafrost (assumed mostly present at 6g), the smooth slopes of 3 to 4H:1V are generally adopted for permanent excavation in overburden. Steeper slopes may be realized for small or temporary excavation, especially in granular material. On the opposite, smoother slopes might be required for excavation in ice lenses rich soils subject to thawing conditions.

6.6 Conveyance structure

6.6.1 General concept

Conveyance structures at site 6g include the headrace canal (discussed in section 6.7), the intake structure, the power tunnel, the tailrace tunnel, a transfer tunnel (Tunnel 1), two diversion tunnels and four transfer canals (Canals 1 to 4).

Water is transferred from the main reservoir (Big Lake) to the powerhouse through Tunnel 1 that empties into the Lower Lake, and then through the 10 km long power tunnel excavated into bedrock, eventually discharging into the 1.1 km long tailrace tunnel that empties in Godthabsfjord. At present study level, the power tunnel is planned to be excavated with a tunnel boring machine. All of the other tunnels are planned to be drilled and blasted.

The transfer Tunnel 1 is design to pass the operating flow of the powerhouse since the main storage for the project in the upstream lake. All other conveyance structures, except for the diversion tunnels, are design to transfer flow from adjacent catchments towards the main reservoir.

Two other transfer tunnels have been considered, i.e. Tunnels 2 and 3, but were rejected for reliability reasons and due to a low power gain compared to the cost of implementation (discussed in section 4). The reliability issue of constructing those tunnels is discussed hereafter

6.6.2 Ice issues

At site 6g, no permafrost was encountered during the investigations, as it was the case at site 7e. However, sporadic permafrost is still possible in the higher areas surrounding the proposed reservoirs (Big and Lower lakes).

The design of the conveyance structures has to prevent the risk of freezing of the water passages during the winter season, due to frazil ice and ice blocks on the water surface.

6.6.2.1 Canals

For the free surface works, like the headrace canal of both the intake and Tunnel 1, a stable cover has to form early in the winter season to reduce frazil ice formation that can eventually block water passages if it accumulates into blocks. The canals are then designed for a maximum flow velocity of 0.65 m/s at the minimum operating level, which allows the formation of a stable ice cover according to Hydro-Quebec standards. When a

stable ice cover is formed, water that is constantly flowing underneath will not freeze as it is the case in the headrace canals.

The canals proposed at site 6g (Canals 1 to 4) are also design according to the design velocity of 0.65 m/s for the formation of a stable ice cover. Free surface canals flowing too fast, thus impeding the formation of a stable ice cover, would likely freeze rapidly due to frazil ice that would eventually accumulate into large blocks and reduce the useful section of the canals. A minimum depth of 2 m of water under the ice cover is targeted at any time to reduce the risk of freezing. A 2 m ice cover thickness assumption is conservatively used.

Another concern is the excavation of parts of canals 3 and 4 into overburden. To reduce the risk in of slope stability problem in overburden to an acceptable level of confidence, the excavation slopes should be not steeper than 4H :1V due to unfavorable conditions associated to thawing of permafrost.

6.6.2.2 Tunnels

For the power and tailrace tunnels, there is no risk of freezing since they are excavated deep into the bedrock, and no permafrost was encountered in the proposed area. Even if there was sporadic permafrost, depths would likely be small, and the tunnels are excavated in depths largely greater than 100 m on most of the proposed longitudinal axis.

The only zone at risk for freezing would then be near the intake. Heating of the intake structure will be provided to ensure a reliable operation of the closing gate. Indeed, this gate will be open most of the time so heating is required to eliminate potential ice formation on the steel elements and in the embedded parts if it has to be closed (or opened, following maintenance works for example).

Tunnel 1 is also a key component in the project at site 6g, since it withdraws water from the main reservoir (Big Lake) and transfers it towards the intake. The tunnel is then designed to be submerged at its upstream and downstream ends, thus eliminating the risk of ice blocks or cold air entering the tunnel. The intake structure of the tunnel will be equipped with two gates to regulate the flow passing through the tunnel, allowing for one gate to be closed for maintenance or other purpose. The gate system will also be heated to prevent the risk of freezing as power production at site 6g relies on the operating of this regulating structure.

Finally, the two diversion tunnels (at Dam 1 and Dam 4) be dry during the winter period, since flow is intermittent. The tunnels will most likely flow only during the summer months, from May to September. Ice blocks may accumulate either at the entrance of or in the tunnels, but the presence of numerous workers and machinery at the sites can allow for a fast intervention in case of a problematic situation.

6.6.2.3 Discarded tunnels

As mentioned earlier, two other tunnels have been considered and been discarded of the base scheme at site 6g: Tunnel 2 to increase the storage volume of the reservoir with the Middle Lake (equilibrium tunnel) and Tunnel 3 to divert an additional catchment towards the Lower Lake.

Tunnel 2 was considered safe regarding the risk of freezing, as the tunnel would have been excavated in bedrock free of permafrost and it would have been submerged at both ends for the full range of operating conditions (low-setting tunnel). However, Tunnel 3 would present risks of freezing as it would have to be a high setting – free surface flow

tunnel. The tunnel would be located in a remote area near the margin of the Greenland glacier, approximately 15 km from the Lower Lake. The presence of permafrost in this area is more likely. Also, the tunnel would experience almost no flow during the winter, which would likely end up freezing parts of the tunnel. Since the tunnel would have been constructed far from the other facilities, inspection and intervention in case of a problematic situation would have been difficult. These issues along with the low gain in power production with the inflows from this catchment justified not to include this tunnel in the base scheme.

6.7 Headrace canal and intake

6.7.1 Location and sizing

6.7.1.1 Intake structure

At site 6g, it is possible to construct the intake structure above the natural water level of the Lower Lake (654 m), since it is planned to operate the lake at a constant water level of 667 m. Another intake structure has to be constructed at the entrance of Tunnel 1 in the Big Lake. The intake structure for the power tunnel will have only 1 gate, while the intake structure of Tunnel 1 will have 2 gates.

The location of the intake structure was chosen to minimize the excavation quantities of both the intake structure and headrace canal, and the power tunnel. The options that were studied targeted zones where the rock was observed at the surface on the orthophotos for stability purpose of the structure.

The cross-sectional area of the water passage at the intake structure is of the same size as the power tunnel, which is 20.4 m². The cross-section is set rectangular with a width of 3.75 m and a height of 5.5 m, to minimize the size of the gates.

It is necessary to provide an adequate submergence at the intake structure to eliminate the risk of vortex formation that could reduce the performance of the turbines due to potential air entrainment, and cause debris entrainment towards the thrashracks. The well-known Gordon's law⁷ is used to estimate the required submergence. The minimal submergence required is calculated as follow:

$$h = C_a v d^{0.5}$$

Where:

H = minimal submergence (in m)

C_a = coefficient relative to the approach flow conditions (0.54 for symmetric approach and 0.73 for asymmetric)

V = water velocity through the intake structure (in m/s)

D = height of the intake structure (m)

Since a small headrace canal will be excavated in the reservoir to ensure adequate flow conditions upstream of the intake, it is assumed that the approach flow conditions will be mostly symmetric. A minimal submergence of 2 m (rounded to the higher integer) was

⁷ Gordon, J.L. (1970). Vortices at intake, *Water power*, No. 4.

calculated for a height of 6.1 m at the entrance of intake structure, which is set to make the entrance smoother and reduce head losses. The minimal invert elevation of the intake would then be 657 m to ensure safe operations at the Lower Lake operating level of 667 m, with including a 2 m thick layer of ice. Since the water level of the lake during construction will be approximately 654 m, the invert elevation of the intake structure is set at elevation 655 m which increase even more the submergence of the intake.

Another intake structure has to be constructed at the entrance of Tunnel 1 in the Big Lake. The same criteria did apply for the design of the intake structure. The invert elevation of the structure is set at elevation 660 m to ensure safe operation at the minimum operating level of 669 m of the reservoir.

6.7.1.2 Headrace canal

The design criteria for the headrace canal is to have a maximum flow velocity of 0.65 m/s upstream of the intake to ensure the formation of a stable ice cover for the winter season. This criterion is based on the current practice employed by Hydro-Quebec in the northern region of James Bay in Quebec. Without a stable ice cover, there is a large risk of clogging of the thrashracks with ice due to constant frazil ice formation.

Since the shores of the Lower Lake at the intake structure location are quite steep, the headrace canal is short. The cross-sectional area needed for the headrace canal is calculated for a constant operating level of 667 m. Since the average operating discharge at the site will reach close to 35 m³/s, a minimum cross-section of 55 m² allows to meet the design criteria. The section near the intake will be excavated into dry bedrock at elevation 657 m, since the water level during the construction period will be close to 654 m only.

6.7.2 Heating

Heating will be provided at the intake structure and at the intake of tunnel 1, to prevent freezing and to ensure an adequate operation of the gates. Both of these structures will be exposed to cold conditions, especially the upstream wall of the structure that can be exposed to very low temperatures due to the fluctuations of the water level in the reservoir. A surface electrical line will bring power to the intake.

The proposed scheme is to provide round tubes embedded in the periphery of the concrete walls of the intake shaft. Most of these pipes are located in the upstream wall of the shaft. These pipes will cover the full height of the shaft from the shelter floor down to 2 m below the minimum water level. Electric heating elements with 150 watts/m will be inserted in the pipes.

6.7.3 Lined section of power tunnel and manifold

This section describes the lined portion of the power tunnel upstream of the powerhouse including the manifold.

The lined portion of the power tunnel represents only a small length of the predominantly unlined power tunnel.

Generally the surrounding rock in the region consists largely of composite gneisses. Preliminary geologic mapping confirmed that the rock is predominantly hard and sound. There was no permafrost encountered in the area of site 6g during the investigations.

The height of the rock covers 200 m upstream of the 6g power station varies from 700 m to 600 m.

The hydrostatic head in the tunnel in the case of the 6g power station is about 660 m. The rock cover would be adequate assuming a normal distribution of rock stresses.

For final design in-situ testing, hydraulic jacking and door stopper test should be done on the rock in the vicinity of the powerhouse in order to evaluate the ability of the rock to withstand the designed internal pressure.

In order to prevent excessive leakage through the tunnel in the region close to the powerhouse and in absence of three dimensional seepage analyses, ASCE recommends using a length of watertight liner equivalent to 25% of design head for preliminary evaluations.

Part of the designed length of steel lined tunnel can be replaced with reinforced concrete liner, the amount of which will be determined based on actual permeability parameters and detailed seepage analysis through the liner.

For this preliminary analysis about the in-situ rock properties, the value of the concrete liner was chosen conservatively to be 50 m for the power tunnel at site 6g.

The tunnel diameter was optimized based on an economical analysis with an actualization rate of 4%, however for the final design this actualization rate needs to be reviewed for a more accurate value which can range between 3% up to 6%. The lined tunnel diameter was calculated to be 3.0 m for the 6g power station.

The steel lined portion of the tunnel should be designed to resist internal and external hydrostatic pressure and the liner should also be designed to resist buckling using the Amstutz formula.

In the zone where sound rock is available partial rock participation should be considered in the analysis.

The concrete cover that will be used around the steel liner and between the liner and the rock excavation is 750 mm.

The manifold is designed to withstand the full internal hydrostatic pressure plus the hydrodynamic pressure.

The diameters of the manifold sections were designed to maintain a constant speed taking into consideration a possible shut down of one unit for maintenance.

Steel liner material should be made out of pressure vessel quality steel with grades corresponding to the European standards (EN/ENV) with accompanying Danish National Annex Documents (NAD) and Danish Building regulation.

The steel liner should be designed to conform to ENV2009 Eurocode 3 - Design of steel structures and based on the latest edition of design codes and standard as listed in section 7.3 of the design criteria. For material properties see section 7.4 of the design criteria.

The elastic limit of the steel (f_y) can vary between 235 MPa up to 500 MPa. In addition to steel work specified in section 7.4.4 of the design criteria, high yield strength structural

steel (S460Q and the S500Q) can also be utilized but must be certified as pressure vessel quality. The recent trend is towards using a higher strength steel.

6.8 Power tunnel

6.8.1 Tunnel axis and longitudinal profile

The overall length of the tunnel is 11 km and has a diameter of 5.10 m (circular shape). Compared to the former power tunnel axis, the inlet and outlet remain at the same locations, while the powerhouse complex was moved some 2 km downstream and 500 m west. The powerhouse was displaced so to avoid a major fault zone which was almost parallel to the previous axis.

The powerhouse has a 6.1% slope so to eliminate the need of a shaft about 500 m high and at the same time it facilitate the drainage.

6.8.2 Transient regime and surge chamber analysis

The main objective is to analyse if the transient regime conditions are acceptable along the headrace tunnel without the installation of a surge chamber. The three independent phenomenon of the transient regime to be analysed for the site 6g power plant includes the following:

- Pressure rise “water hammer” due to the turbine load rejection.
- Speed rise influence by the water hammer effect,
- Governing stability regulation.

Detailed analysis of the transient regime as well as the decision related to the necessity of providing a surge chamber is presented hereafter. The basic data used for the calculations is the following:

• Tunnel length (m)	9 990
• Tunnel cross section (m ²)	20.4
• Tunnel mean velocity (m/s)	1.5
• Friction head loss (m)	3.4
• Net head (m)	655.3
• Generator output (MW)	88.9
• Electrical output (MVA)	98.75
• Turbine output (MW)	90.1
• Synchronous speed (rpm)	500
• Reservoir water level (m)	667
• Elastic pressure wave in the roc (m/s)	1 415

The design criteria used in the transient regime analysis are defined as follow:

- The pressure rise ($\Delta H/H_0$) resulting from a load rejection of the Pelton unit is accepted within a range of 10 to 15%;
- The speed rise ($\Delta n/n_0$) shall be less than 35%;
- Both Routh-Hurwitz and Seeberger criterion are used to check against the governing regulation stability.

6.8.2.1 Water hammer analysis

Both methods used to determine the pressure rise during load rejection are: i) the method of characteristics based on the numerical resolution using explicit scheme, and ii) the Allievi inter-lock series resolution of the pressure wave propagation along the headrace tunnel.

As the characteristic of the headrace tunnel (ρ) < 1, it could be expected that the maximum pressure rise associated to the pressure wave propagation (a) will occur at the end of time interval equal to $2 L/a$.

The pressure rise ($h = \Delta H/H_0$) is presented in the table below as a function of the time of closure (T_f).

Table 6.14 Pressure rise

Time of closure (sec)	Pressure rise (%)
20	22.3
30	14.3
40	10.6
50	8.4

As previously stated, the allowable pressure rise for the Pelton unit is within a range of 10 and 15%. It corresponds to a closure time varying between 30 and 40 sec. It results that the water hammer effect due to load rejection is not a major concern.

6.8.2.2 Speed rise analysis

During load rejection, the deflectors can be activated within a short period of time: generally between 2 to 6 sec. Considering the maximum allowable pressure rise equal to 15%, the speed rise ($\Delta n/n_0$) influenced by the water hammer during load rejection is presented as a function of the deflector closure time in table below.

Table 6.15 Speed rise

Deflector closure time (sec)	Speed rise (%)
2	13
4	25
6	36

It requires activating the closure of the deflector between 4 and 6 sec to maintain the speed rise within a permissible value of 30%. It thus results that the speed rise effect is not a major concern for the Pelton turbine equipped with a deflector device.

6.8.2.3 Governing stability

The characteristics of the generator including the power output (P_o), the flywheel inertia (PD^2), the accelerating time of the flywheel (T_m) as well as the accelerating time of the water column (T_w) are presented below:

- P_o 88.9 MW
- PD^2 1106 t x m²

- T_m 8.54 sec
- T_w 2.34 sec
- T_m/T_w 3.64

The governor regulation system is controlled by the servomotor speed mechanism. A proper adjustment of speed - responsive element according to small oscillations will provide an adequate governing stability. The following criteria can be used to verify the governing stability conditions:

i) Seeberger criteria

Based upon the Seeberger criteria, for the accelerating time of water column (T_w) in the headrace tunnel equal to 2.34 m/s, it requires a minimum accelerating time (T_m^*) equal to 7.54 sec to ensure the governing stability condition. As shown previously, the parameter T_m corresponding to the natural flywheel inertia is greater than T_m^* : $T_m/T_m^* = 3.65$. The governing stability is then adequate.

ii) Routh-Hurwitz criteria

The table below presents the different combination of the governing regulation setting - type PID for ensuring the governing stability associated to small oscillations.

Table 6.16 Required inertia of the flywheel for stability

$T_r = 8 \text{ sec}$ and $\sigma = 40\%$	$T_m \text{ (sec)} = 7.40$	$PD^2 \text{ (t x m}^2\text{)}=958.66$
$T_r = 8 \text{ sec}$ and $\sigma = 35\%$	$T_m \text{ (sec)} = 8.46$	$PD^2 \text{ (t x m}^2\text{)}=1095.61$
$T_r = 8 \text{ sec}$ and $\sigma = 30\%$	$T_m \text{ (sec)} = 9.87$	$PD^2 \text{ (t x m}^2\text{)} = 1278.21$

Notes: T_r = relaxation time of the dashpot and σ = statism = $-d\omega/dx$

From the above figures, for $T_r = 8 \text{ sec}$ and $\sigma = 35\%$, both parameters T_m and PD^2 are respectively 8.46 sec and 1 095.61 t x m², i.e. values practically similar to the values as determined based upon the natural weight of the flywheel rotor ($PD^2 = 1 106.0 \text{ t x m}^2$), it results that the regulation stability is adequate.

Considering for example the oscillation of the power output ($\Delta N/N_o$) being equal to 5% and for given parameters: $\sigma = 35\%$ and $T_r = 8 \text{ sec}$, the frequency oscillation ($\Delta f/f$) estimated based upon the method of Esscher Wyss - bulletin 52/53 is approximately equal to 4%.

6.8.2.4 Conclusion

From the standpoint of the transient regime, the results obtained in the above analysis showed that a surge chamber is not required at site 6g.

6.8.3 Penstocks and manifold

The geometry and the sizing of penstocks and the manifold were based on the general criteria presented in section “Design criteria for tunnels”. The manifold has a circular final shape with the following dimensions: length of 17 m with variable diameter from 3.1 to 1.8 m. The penstocks (number of 2) have equally final circular shape (inside diameter of 1.8 m) with an overall length of 22 m.

6.9 Turbine-generator units

6.9.1 Pelton turbine selection

6.9.1.1 General criteria for turbine type selection

Before selecting Pelton rather than Francis units, both being theoretically feasible, the main issues which were considered included:

- efficiency at full load;
- facility to deal with silt erosion and long term performance preservation;
- turbine stability;
- manufacturing difficulties of extreme high head Francis runners and precision;
- availability of competent suppliers and repair capability at site;
- cost and schedule;
- sensitivity to tailwater level variations;
- cold region operation;
- space requirements;
- consequences on transients;
- high speed generator reliability.

The very large majority of high head hydroelectric projects are equipped with Pelton turbines. The Pelton turbine has no real limitation in head, size, operating constraints and hydraulic design of the conveying system.

On the borderline, the Francis turbine design favors a higher specific speed which results in a more economical rotational speed with cost and space reductions on both the generator and turbine at the border between both designs.

The comparison made for 7e is definitely true for 6g, Pelton turbines have to be preferred to Francis. Indeed, it would be even more difficult to manufacture the smaller units precisely and to protect them against erosion.

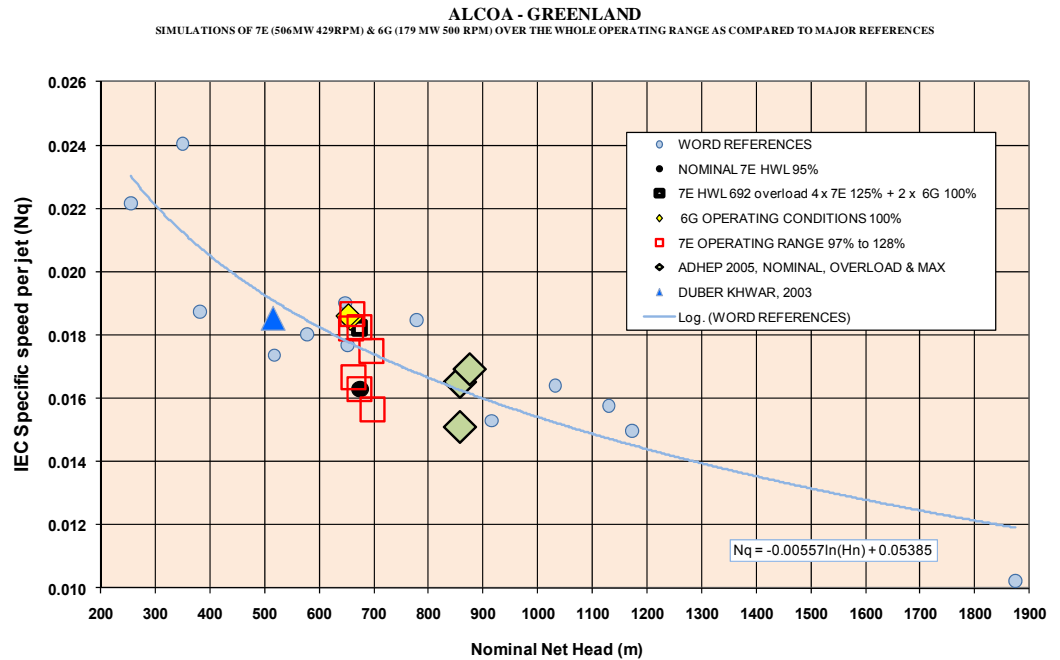
6.9.2 Pelton turbine final selection and sizing - Overload capability

Two options are presented in the 7e powerhouse report:

1. Optimized 6 jets units at 7e (429 rpm) and 6g (500 rpm) of complete different sizes and designs (our dwgs.)
2. Same 429 rpm generators and same runners (to be confirmed) with a special 5 jet spiral distributor for 6g. This option would reduce spare costs and simplify maintenance with a comparable global cost.

6.9.2.1 Different unit design - Overload capability ensured by 7e only

Figure 6.7 Specific speed per jet at 7e and 6g as compared to major references



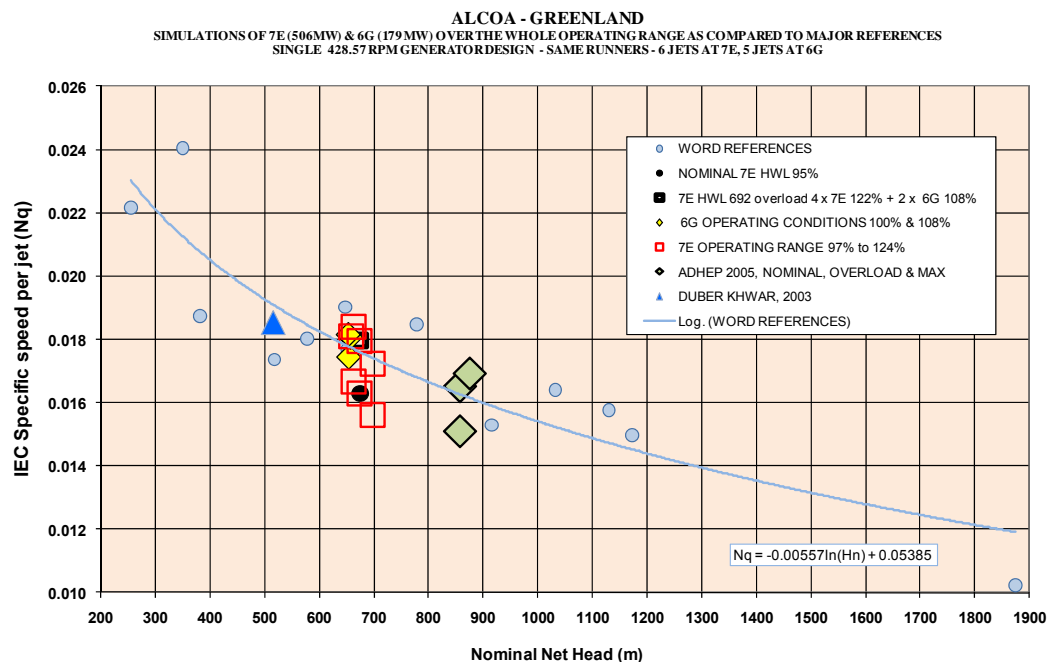
Considering the already high specific speed per jet of the 6g turbines (to be confirmed by manufacturers in view of silt content and general operating conditions), overload operation is excluded and 7e units will have to absorb the necessary overload during the maintenance or unplanned outage of anyone of the units.

6.9.2.2 Same generator design as 7e using 5 jets and 429 rpm rotational speed at 6g

This solution is better balanced in terms of specific speed. However, to be able to use the same runners at 6g, with a nominal net head at 6g being slightly less than the minimum net head of 7e, some compromise will be necessary on the Pelton diameter.

On the graph below, 6g operating conditions are limited to two yellow lozenges corresponding to the normal headwater level and either normal operation (just below the trend line) or 108% overload (above the trend line) when anyone of the 6g or 7e unit is out of service for inspection or maintenance. We can expect the overload situation to occur at least 6 times in a year, assuming units are inspected yearly, at least during the guarantee period.

Figure 6.8 Specific speed per jet at 7e and 6g as compared to major references



Overload capability was balanced between 6g and 7e units in order to attain the same severity degree when the 7e reservoir is at its 95% probability of occurrence. If unplanned outages of 7e units occur simultaneously with deep draw down after sustained low inflows, the remaining 7e units might have to operate at up to 124% overload, the reason why their nominal operating condition was selected sufficiently below the statistical trend line. This is also the reason why the number of units was fixed to 5, the overload percentage for a four unit powerhouse becoming excessive.

Table below lists typical 6g operating conditions, with or without one unit out of service, either from 6g or 7e, and headlosses corresponding to a smooth headrace tunnel (TBM).

Table 6.17 Typical 6G operating conditions

Operating condition	500 rpm optimized 6g			429 rpm common generator and runner		
	Nominal	1 6g unit Out of serv.	1 7e unit Out of serv.	Nominal	1 6g unit Out of serv.	1 7E unit Out of serv.
Number of units in operation	2	1	2	2	1	2
HWL	667	667	667	667	667	667
Powerhouse discharge	30.7	15.4	30.7	30.7	16.6	33.1
Generator output	89.4	89.8	89.4	89.4	97.0	96.4
Turbine model efficiency	91.80%	91.81%	91.80%	91.90%	91.88%	91.87%
% of nominal discharge	100%	100%	100%	100%	108%	108%
Turbine discharge	15.4	15.4	15.4	15.3	16.6	16.6
Turbine output	90.6	91.1	90.6	90.6	98.3	97.7
Turbine net head	653.4	656.8	653.4	653.4	656.5	652.5
D nozzle opening	0.216	0.215	0.216	0.236	0.245	0.245
nq/nq opt	1.232	1.227	1.232	1.156	1.197	1.203
ns jet	18.60	18.53	18.60	17.46	18.08	18.16
Nq jet	0.019	0.019	0.01859	0.017	0.018	0.018
Unit Efficiency	90.56%	90.57%	90.56%	90.66%	90.64%	90.63%

Unit common characteristics and sizing corresponding to above performances appear at table below:

Table 6.18 Unit characteristics for both options

Output at generator terminals 7e + 6g = 684.5 MW	Units	500 rpm optimized 6g	429 rpm common generator and runner
Synchronous speed	rpm	500	429
Nozzle level	m	7.500	7.500*
# jets		6	5
Pelton diameter Dp	m	2.068	2.411 *
Bucket width B	m	0.540	0.621 *
Dp/B		3.83	3.88 *
Runner weight	kg	6252	9788 *
Efficiency corr for HVOF coating		-0.4%	-0.4%
"Zero" cavitation ¼ Annex A IEC 60609	kg	0.18	0.24
High tide for full load MSL	m	2.63	2.63
Future Ocean rise provision	m	0.30	0.30
# Pole pairs		6	6
Cos Phi		0.9	0.9
Generator Eff.		98.65%	98.65%*

* Dimensions to be adapted for a common speed and runner design with 7e

6.9.3 Summary of extreme operating conditions

Option	6 jets 500 rpm	5 jets 429 rpm
Maximum generator output MVA at 0.9 power factor	100 MVA *	108 MVA *
Maximum power at 6g generator terminals	178.8 MW **	194 MW **
Maximum power at 6g switchyard	175 MW **	190 MW **
Maximum discharge of 6g powerstation	30.7 m ³ /s	33.1 m ³ /s **
Nominal net head	653.4m	653.4m
Maximum static head	658.7 m	658.7 m
Maximum net head at full load	656.8 m *	656.5 m *
Minimum net head	653.4 m	652.5 m **

* One 6G unit out of service

** One 7E unit out of service

6.9.4 Unit configuration: Generator foundation combined with Pelton housing

The arrangement shown on drawing A-403 is for a two bearing compact unit, which combines the advantage of a reduced cavern height and of generator foundations less sensitive to rock relaxation after excavation. Now that most major turbine and generator suppliers are integrated in single companies such as (Voith-Siemens (Riva), Alstom (Neyrpic, ABB), Andritz Hydro (Va Tech EscherWyss Ellyin GE Kvaerner), as well as suppliers from Asia, it is no longer necessary nor commercially valuable to separate the turbine and generator contracts in order to combine the best price and best technology, once a reason for separate foundations.

La Batiatz (Switzerland), San Carlos (Colombia) operate satisfactorily using this arrangement at similar heads as well as a number of high head powerhouses, old or new.

Irrespective of cavern compression sensitivity, the operator also benefits of the two bearing arrangement, which is much easier to commission and maintain than a long shaft three bearing unit.

The only inconvenience is the reduced working space in the deflector and jack/ brake zone. These equipments require very little maintenance with the extensive use of stainless steel, self lubricating materials and LVDT feedbacks. Furthermore units attached to a smelter are less exposed to premature wear as compared to Pelton peaking units combined with pump turbines which may start and stop many times a day.

6.9.5 Turbine and valve design and manufacturing

As mentioned previously, these units shall operate with the highest possible availability factor and require the minimum possible maintenance:

- a) Runners: All runners shall be 100% machined out of a solid forged stainless steel disk from low carbon, low sulfur CA6NM grade. Most probably, all runners will have to be hard coated with a Cobalt base Tungsten carbide layer using the HP-HVOF process. However, we are afraid that the first runner to be commissioned cannot be fully protected against impacts, whatever the care given to the tunnel cleaning and the importance of rock traps. It could be of interest to commission the first unit with a non coated smooth runner whose initial performance would be optimal and repair easier.

It will serve as a test for erosion rate and hard coating profitability. This first runner will be delivered with a coated spare runner for early replacement in case erosion puts the first runner at risk of irreversible damage. Conservative stress amplitude will be specified.

- b) Nozzle tips and needle tips: All nozzles shall be equipped with superfinished Co-W-C hard coated nozzle and needle tips.
- c) Cut-in deflectors, nozzle roofs and nozzle heads shall be forged stainless steel for sustained operation with deflected jets.
- d) Turbine housing liners, gratings and air admission pipes shall be designed for sustained operation with deflected jets.
- e) Spiral distributor shall be delivered in three sections with numerically machined weld preparations for minimum works at site and flange connection to the test head first and spherical valve makeup pipe later. Radiography should be avoided on the site circumferential joints. All spiral construction shall be made of plates and forging.
- f) All tooling inside the turbine housing, trolleys, runner coupling, needle attachment and all lifting devices shall be designed for minimum maintenance downtime and skill requirement.
- g) Double counterweight, double servomotor spherical valve with service and maintenance seal and seat, fully replaceable using turbine HPU for opening only.

6.9.6 Generator design and manufacturing

Characteristics given for the nominal power factor:

- a) stator winding Class F T. rise 75°K, Max T 115°C at maximum overload and maximum water temperature;
- b) rotor winding Class F T rise 80°K, Max T 120°C at maximum overload and maximum water temperature;
- c) stator core T rise 70°K, Max T 110°C at maximum overload and maximum water temperature, 85dB max;
- d) accelerated aging test for all series of Roebel bars;
- e) ripple spring radial packing;
- f) bolted stator frame with stator core stacked at site;

- g) single piece fans;
- h) hot air drainage capability for powerhouse heating with filtering of air makeup;
- i) partial discharge detectors;
- j) closed loop stainless steel cooling system;
- k) self equilibrated thrust bearing;
- l) stabilization of bearings at maximum overspeed (corresponding to full load rejection);
- m) runaway site test;
- n) short circuit test.

6.9.7 Governor, Excitation system and auxiliaries

The overall responsibility for the T/G unit, main inlet valve, governor, excitation system and associated auxiliaries including control, command and protection, man-machine interface, compressed air for the units, cooling water system shall be the responsibility of the main Contractor and include signal and power cables, batteries, chargers and inverters, and all embedded air and water piping.

6.10 Powerhouse, transformer cavern and annex building outside

6.10.1 General layout

Powerhouse and transformer gallery are located inside of two main caverns.

The overall length of the powerhouse (including the service bay) is 67.05 m at level 13.80 m, its width is 15.25 m and it has a height of 30.80 m. The overall length of the transformer gallery is 84.45 m, its width is 13.50 m and it has a height of 12.55 m at its centerline. The sizing of both caverns was established with accordance with the criteria presented in section "Design criteria for tunnels". The distance between the powerhouse and the transformer galleries is 1.5 times the width of the powerhouse.

The powerhouse is subdivided in two sections: the first one includes all the energy producing equipments; the second one is located on the left hand side of the main cavern, which is the service bay area. Access to these caverns is possible through two tunnels located at their extreme left.

The dimensions and locations of the energy production areas and the service bay area of the powerhouse are conforming to the requirements of all disciplines. The main level of the Powerhouse, service bay, bus bar galleries, transformer cavern, electro-mechanical tunnel and escape tunnel is 13.8 m.

The service bay includes several locals at different levels including the service building, one assembly area and unloading area with the same level as the main level of the powerhouse namely 13.8 m.

The transformer cavern is located downstream of the powerhouse and parallel to it. The overall length of the transformer cavern is 84.45 m, its width is 13.5 m and it has a height of 12.55 m at its centerline. Its floor level is 13.8 m, same as the principal level the Powerhouse.

The annex building is located outside; it has two floors and a flat roof. The overall dimensions of this building are: 68 m length, 30 m width and 7.5 m height.

6.10.2 Powerhouse layout

The principal floor level of the powerhouse is 13.8 m, the generator floor level is 8.85 m and the turbine floor level is 4.14 m.

The powerhouse is comprised of two bays, one for each turbine-generators unit. The distance between the centerline of each unit is 14.0 m. The unit number 1 is adjacent to the service bay and the unit number 2 is located at the far right of the cavern.

An overhead crane, with 265 t capacity, located at the elevation of 22.6 m, is serving the whole length of the powerhouse and major part of the service bay. This crane covers the whole length of the service bay, up to column line 10, during construction. After construction, the overhead crane serves part of the service bay, up to column line 7.

A suspended ceiling at the elevation 28.0 m is located above the powerhouse and the service bay area.

Table 6.19 Different levels of the powerhouse

Level (m)	Description
1.55	Discharge basin
4.14	The valve floor with valve and servomotor bases
7.3	Centerline of the turbine, distributors and the penstock
8.85	The generator floor with generator housing, oleopneumatic assembly and turbine wheel hatch
13.8	The powerhouse main floor with turbine wheel hatch, generator cover, electrical control panel. The bus bar galleries, contains the electrical equipments, is at the same level.

6.10.3 Service bay layout

The service bay is located at the extreme left side of the powerhouse.

One elevator and one stair case provide access from level 4.14 m to 26.6 m.

Service bay, at the main floor elevation of 13.8 m, is composed of the assembly area and the unloading area. The five other floors, namely elevations 4.14 m; 8.85 m; 18.3 m; 22.3 m and 26.6 m, are used for different locals.

Table 6.20 Floor description

Level (m)	Description
4.14	Pump room; Hydrocarbon room
8.85	Potable water room; Sanitary waste system room; Oil room; Compressors room; Washrooms
13.8	Accumulator /battery room; Charger (battery) room
18.3	Control room; Computer room; washrooms

Level (m)	Description
22.3	Telecommunication room; Battery Télécom; Substation protection & control center room; Technician office; Engineering office; Lunch room (command room); Computer room; Meeting room; Lunch room; washrooms
26.6	Elevator mechanical room (lift well); Mechanical and ventilation room (HVAC)

The overhead crane and suspended ceiling as described above are also located in this area.

6.10.4 Transformer cavern layout

The transformer gallery is located downstream and parallel to the powerhouse in a separate cavern. The overall length of the transformer cavern is 84.45 m, its width is 13.5 m and it has a height of 12.55 m at its centerline. Its floor level is 13.8 m, which is the same as the principal level the Powerhouse. It has two floors at 13.8 m and 22.3 m.

The cable gallery tunnel and access gallery are connected to its extreme right and its extreme left respectively.

Two bus bar galleries, one escape tunnel, one electro-mechanical tunnel at the level at the level 13.8 m and one ventilation tunnel at the level 28 m connect the transformer cavern to the powerhouse cavern. An underground oil separator at 10.3 m level is located at extreme left side of the cavern.

The transformer gallery accommodates two power transformers, two auxiliary transformers and one reserve power transformer. A gantry crane and two draft tube gates are also located in this cavern.

6.10.5 Annex building outside

The annex building is located outside of the caverns. It has two floors and a flat roof. The overall dimensions of this building are 68 m in length, 30 m in width and 7.5 m in height.

This building consist of living spaces, offices, maintenance area, mechanical and electrical rooms.

6.10.5.1 First floor

- Workshop: civil / electrical/ mechanical
- Mechanical warehouse
- Electrical warehouse
- Civil warehouse
- Electrical room
- Wood workshop
- Welding workshop
- Handling maintenance room
- Visitor room office
- Transmission room
- Mechanical and ventilation room

6.10.5.2 Second floor

- Infirmary room
- Technician office
- Powerhouse chef office
- Maintenance chef office
- Administrative office
- Documentation / reproduction office
- Engineering office
- Meeting room
- Lunch room
- Men shower room and bathroom
- Men washroom
- Women shower room and bathroom
- Women washroom

6.10.6 Structure

Concrete floors are mainly used for the Powerhouse, Service bay area, Transformer Cavern and Annex building. The overhead crane in the powerhouse and service bay area is supported by the steel girders. These steel girders are designed as simply supported beams. Steel columns are used to transfer the crane loads to the floor level 14.7 m. Vertical bracings provide stability for the crane supporting steel structures.

The suspended ceiling at the elevation 28.0 m is designed with the light weight steel channels and plates. This ceiling is hanging from the roof by means of cables. These cables are attached to the rock anchors.

6.11 Tailrace tunnel

The overall length of the tailrace tunnel is 1.1 km, its width is 5 m and it has a height of 8.25 m. It's a free surface flow tunnel with a reversed-D shape. The cross-sectional area is 39.4 m². In order to allow the excavation in the dry, a rock plug is left in place in the extreme end of the tunnel. This rock plug is excavated only after the gates of the draft tubes are installed. There is no gate required at the extreme end of the tailrace tunnel.

6.12 Access tunnels

There are several access tunnels available that allow not only the transportation of the construction materials and personnel required for the construction, but also allow the simultaneous excavation of the powerhouse complex on at least tree headings. These access tunnels are: main access tunnel to the powerhouse, access tunnel to the transformer chamber, access tunnel to the power tunnel and access to the tailrace tunnel.

The sizing of the access tunnels shown on the drawings and their sections were established depending of the dimensions of the equipments required for the construction.

The layout of all access tunnels was designed as to allow the drainage by gravity.

The crown of all access tunnels will be covered by a wire-mesh so to ensure the safety of the personnel and the equipment.

6.13 Auxiliary services

6.13.1 Electrical

6.13.1.1 Equipment at the Stator voltage (10.3 kV)

6.13.1.1.1 General

The nominal voltage at the generator terminals is 10.3 kV. The equipment between these terminals and the terminals on the LV side of the unit transformers are the following;

6.13.1.1.2 Excitation transformer

The excitation transformer shall be dry type, and shall be able to operate at its nominal rating on a permanent basis

Insulation level shall be class F, resin encapsulated. The transformer shall be encased in an IP 31 (minimum) cubicle, as for the standard IEC 60529 and shall be equipped with bars and all accessories for direct connection to the phase isolated bus bars by tee connection close to the generator terminals.

6.13.1.1.3 Static Excitation

The static excitation system includes mainly:

- the rectifier bridge;
- the field circuit breaker (contactor);
- the field flashing contactor from the battery;
- the field discharge resistor;
- the local / remote control devices;
- the protection relays;
- the power stabilizer.

6.13.1.1.4 Current transformers

The 10,3 kV input at the excitation transformer shall be equipped with a current transformer for the generator differential protection and a second current transformer with a ratio for the excitation transformer overload protection.

6.13.1.1.5 10.3 kV natural air cooled isolated phase bus bar system components and accessories

Isolated phase bus bar

The 10.3 kV isolated phase bus bar shall be the natural air cooled type. The design shall incorporate cubicles and connection points for voltage transformers, excitation transformers, lightning, arrestors, capacitors and cable connections.

Generator voltage transformer cubicles

These cubicles shall be metal clad, air insulated, entirely factory fabricated, and shall contain two voltage transformers with fuse protection on their primaries. The transformers, with their fuses, shall be installed on drawers, withdrawable from the energized 10.3 kV section.

Capacitor and lightning arrester cubicles

These cubicles shall allow the connection of the capacitors and lightning arrestors to the 10.3 kV main bars.

Generator isolating cubicles

In order to be able to feed the auxiliary services of the power station from the network, an isolating switch is required between each unit transformer and its generator. We can thus isolate each generator from its unit transformer and the 10.3 kV board can be fed from the HV network (refer to the single line diagram). Each switch shall be metal clad, air insulated, with isolating material between the phases.

Generator neutral cubicles

The neutral cubicle for each generator shall include:

- a set of bars, forming the neutral;
- a neutral transformer;
- a neutral resistance, connected to the neutral transformer secondary;
- three protection CTs on each of the three phases.

The neutral cubicle shall be installed close to the generator neutral zone.

Generator phase cubicles

For each generator set, a set of cubicles including, for each phase:

- one CT and one VT for the voltage regulator;
- a two winding VT for:
 - the speed regulation;
 - metering, measuring and protection;
- three CTs for:
 - measuring and metering;
 - over-current and unbalance protection;
- an outgoing section for excitation.

The isolated phase bus bar shall be equipped with provision (connections) so that short circuit tests are possible. One set of bars is foreseen for the five generating sets.

6.13.1.2 Power transformers

6.13.1.2.1 General requirements

The three phase transformers 100 MVA, 10.3 kV – 225 kV. (The dimensions can change after the brought modifications to compensate a group in maintenance).

The winding connections are dYn11. The HV windings are star connected with the neutral solidly grounded.

The direct sequence impedance is 12% and the transformers shall be supplied with 2 x $\pm 2.5\%$ off-load tap changers.

Cooling shall be OFWF.

All the transformers shall be equipped with removable wheel sets. The transformers in their bays will be installed on rails via shimming plates. The rails shall be connected to the transformer concrete base.

The 225 kV bushings shall be SF6 type.

Insulated cable at 245 kV shall make the connection between the power transformer and the substation.

At the power transformer SF6 bushing there shall be the SF6 transformer termination module a small section of GIB, gas insulated bus and the GIS cable termination module.

The cable shall terminate at the transformer with its GIS sealing end and in the substation with its outdoor sealing end.

The tanks shall be rectangular with a bolted cover.

The material shall be in accordance to the most recent IEC recommendations; the principal recommendations to consider are:

- IEC 60060 High voltage test techniques;
- IEC 60071 insulation coordination;
- IEC 60076 Power transformers;
- IEC 60137 Insulated bushings for ac voltages above 1 000 V;
- IEC 60296 Specifications for new mineral insulating oils for transformers;
- IEC 60076-5 Loading guide for oil-immersed power transformers;
- IEC 60076-10 Noise level determination for transformers and inductances.

Other internationally accepted standards, which are either as or more demanding, such as the American ANSI, IEEE or the German DIN/VDE will also be accepted.

The transformers shall be designed to minimize harmonics and to avoid deforming the sinusoidal wave which may hamper the telecommunications circuits. The neutral points shall be made available and appropriately grounded.

6.13.1.3 Alternating current sources for the 400 V auxiliary services

6.13.1.3.1 10.3 kV switchgear

Two 10.3 kV principal switchgears shall be energized directly from the main bars; they can be fed from one generator or another, the supply source can be switched from one to the other via a circuit breaker.

The section feeding the 400 V auxiliary services shall include the following:

- one 10.3 kV circuit breaker cubicles (only PSG1);
- two 10.3 kV circuit breaker cubicles (only PSG2);
- one transformer feeder;
- one 10.3 kV switchgear feeder (PSG3);
- one measuring cubicle.

The incoming cubicles (10.3 kV circuit breaker) shall also include a 50: 1A current transformer, whose role is to protect the bus bars and the auxiliary service transformer.

The measuring cubicle shall include two voltage transformers, $11 \text{ kV} / \sqrt{3}: 110 \text{ V} / \sqrt{3}$; the role of one shall be to measure the voltage on one generator and the role of the other shall be to measure the voltage on the adjacent generator.

The 10.3 kV principal switchgear (PSG1) supplied by the groups 1 and 2 constitute the source for Auxiliary Transformer no. S1, and the 10.3 kV principal switchgear (PSG2) supplied by the groups 1 and 2 constitute the source for Auxiliary Transformer no. S2. An appropriate interlock associated with an automatic transfer system will guaranty that there is no inadvertent paralleling of the groups on the 10.3 kV bar. Transformer no. 1 can be energised from either group 1 or group 2; likewise transformer 2 can be fed either from group 1 or group 2. As a single transformer is sized to carry the entire alternating current load from both the power station, it is sufficient to have a single group running for all the auxiliary loads to be fed.

The 10.3 kV/400 transformers S1 and S2 are each rated at 1 500 kVA. They are oil type, air cooled (ONAN). The transformers are fitted with on-load tap changers, with 2 x 2.5%.

The 10.3 kV switchgear (PSG3) supplied by the 10.3 kV principal switchgear (PSG1) and (PSG2) constitute the source for Auxiliary Transformer no. S3 and S4.

The 10.3 kV/400 transformers S3 is rated at 1 500 kVA. They are dry type, natural cooled (ANN). The transformers are fitted with on-load tap changers, with 2 x 2.5%.

The 10.3 kV/400 transformers S4 is rated at 300 kVA. They are dry type, natural cooled (ANN). The transformers are fitted with on-load tap changers, with 2 x 2.5%.

6.13.1.3.2 400 V switchgears

Two main switchgears and seven secondary switchgears are previewed for the auxiliary services of the power house, intakes and services building.

The main switchgears shall be metal clad, self standing cubicles, with withdrawable circuit breakers as well as control and measuring.

The secondary switchgears power the auxiliary services in the areas located in proximity to the switchgears. The motor control centers are integrated in the same switchgear. The incoming breakers are of the withdrawable unit design equipped with automatic switching system, such that if the normal power source is lost, the auxiliary services are not lost.

Two of the ten secondary switchgears have a circuit breaker serving to relieve the non essential loads.

6.13.1.3.3 Auxiliary power sources

The cubicle auxiliary power sources shall be as follows:

- control, interlock and signaling circuits: 125 V d.c.;
- motor cranking circuit: 125 V d.c.;
- heater resistances : 230 V a.c.;
- alternating current for auxiliary electric loads.

400 V Distribution network

The neutral is not distributed, except for the lighting and socket outlet circuits, for which a neutral is created in the 400 V/230 V transformers.

The auxiliary loads are distributed via 400 V panels, as follows:

- Power plant:
 - one panel per turbine-generating set, to feed the set's auxiliary loads;
 - one panel for the backed-up (essential) general auxiliary loads (hydraulic set or diesel generating set);
 - one panel for the non backed-up (non essential) auxiliary loads.

The circuit breaker which sheds the loads on the non essential panel can be manually closed, if there is a requirement to feed certain non essential auxiliaries during a prolonged power outage. Care must then be taken to not overload the power source.

6.13.1.4 Emergency Generating Set (EGS)

The EGS constitutes the primary 400 V back-up source for the auxiliary loads.

The EGS, which has an automatic electric starter, has the following main characteristics:

- it is designed for continuous service.

Its principal components are as follows:

- diesel engine and generator on a common chassis, with an elastic coupling between the engine/generator and chassis;
- slave pump on the engine lubricating circuit, forced air cooling the water/oil coolant;
- exhaust circuit;
- diesel tank, and 500 liter day tank in the EGS room;
- preheating;
- electric starting;
- WOODWARD electric speed regulator;
- brushless generator, self-cooled, rotating diode excitation, static voltage regulator;
- high mechanical inertia;
- control and protection panel;
- local room ventilation;
- load resistance, with automatic start and two-step load contactors, all installed on the roof of the EGS room.

6.13.1.5 Direct current feeds

6.13.1.5.1 125 V dc source

The equipment is as follows:

- two sets of battery/chargers, 125 V, each connected to a set of busbars, connected in parallel and each sized to feed all the auxiliary loads;
- one sets of battery/chargers, 125 V, for the generator field flashing

The battery will be composed of 60 lead acid elements.

The chargers will be of the silicon diode type. The chargers will feed the battery to which it is associated in maintenance mode. They are fed from the essential services (backed-up) 400 V panel.

A 125 V dc distribution board, comprising:

- two sets of busbars, each fed from a battery/charger set;
- outgoing circuits, connected in parallel to the 2 sets of busbars, with protective fuses and diodes to eliminate circulating currents between the batteries, and equipped with a two pole circuit breaker.

6.13.1.5.2 48 V dc source

This equipment is used for the telecommunication systems.

It is made up of a 48 V battery/charger and a distribution panel. The battery is a 60 Ah lead-acid type, and is installed in the battery room. The charger has a 25 A rating.

6.13.1.6 Lightning and socket outlets

Plant lighting will be fed from two auxiliary service panels (normal or backed-up) via 400 V/230 V transformers.

Battery pack fixtures will light exit ways until power is restored.

The plant's socket outlet circuit will be fed from the normal auxiliary service board, via a 400 V/230 V transformer.

6.13.1.7 Telephone

Telecommunications will be via a telephone system with a private PABX internal to the plant. There will be telephones in the various rooms in the plant and connections to the 225 kV substation, to the water intake at the dam and to the workers' town.

A line from the HF telecommunication system over the 225 kV network is dedicated to the plant.

6.13.1.8 Fire detection

The power plant is equipped with a fire detection system including:

- fire detectors spread out in the various rooms;
- a fire alarm panel in the control room.

6.13.2 Mechanical

6.13.2.1 Piping

6.13.2.1.1 General

This chapter describes the piping systems that are not supplied by the units' supplier. The systems are the following one:

- raw water;
- fire protection;
- potable water;
- service water;
- drainage of clear water;

- drainage of waste water;
- drainage of oily water;
- compressed air for general service.

The following systems are supplied by the unit's supplier:

- cooling water;
- filtered water;
- oil handling and storage;
- compressed air for regulation;
- compressed air for breaking;
- units dewatering.

6.13.2.1.2 Raw water

This system supply water to the fire protection system and to the service water system.

The equipment of this system are self cleaning filters and piping networks.

6.13.2.1.3 Fire protection

This system insures the protection of the alternators, the transformers as well as the rooms in the powerhouse that may present a fire hazard.

The system uses water as the extinguishing media.

The water is distributed in a network of pipes to the locations where automatic fire protection is required. For alternators and transformers, deluge valves and dry sprinklers are used. For the rooms, wet sprinklers are used.

There are also sufficient fire hoses and manual extinguishers installed on every floor to complement these systems of automatic protection. The fire hoses are also fed by the same piping network that feed the automatic systems previously described.

Two main pumps are provided to feed the water in the system, taking their water from the raw water system. One pump is able to supply the full flow required. The other is a standby pump in case of failure of the first one. A jockey pump is also provided to maintain the pressure at a suitable value when there is no need for fire protection.

Facilities are also provided to allow for the periodic testing of the pumps and the sprinklers systems.

The fire pumps are located in the service area at level 8.85.

6.13.2.1.4 Potable water

This system insures the supply of potable water suitable for use in lavatories and sinks.

To save on the required flow, the sanitary apparatus like the toilets and the urinals flushes are fed with service water. This system is described hereafter.

Water heaters are provided to supply warm water to the lavatories and sinks.

The water comes from a well (to be confirmed) and is distributed in the powerhouse by a piping network. A storage tank is provided to avoid too frequent starts and stops of the well pump as well as to provide a reserve in case of a pump failure.

The tank and all equipments are located in the service area at level 8.85, except for the well pump which is installed in the well.

6.13.2.1.5 Service water

This system insures the supply of water to the service stations located in the powerhouse and in the transformers gallery as well as to the sanitary apparatus located in the service area.

It takes its water from the raw water system and distributes it, by the means of pumps and a piping network, to the service stations.

The pumps and their control panel are located in the service area at level 8.85.

6.13.2.1.6 Drainage of clear water

This system insures the drainage of clear water from all locations in the powerhouse. All waters that may contains contaminants are drained by dedicated systems like the waste water and oily water described hereafter.

Whenever possible, the water is drained by gravity toward the units' downstream channels.

For locations that are lower than these channels, the water is directed toward a sump where pumps are provided to move the water in the closest unit's channel.

6.13.2.1.7 Drainage of waste water

This system insures the drainage of waste water from all sanitary apparatus.

The wastes are directed to a septic tank whose effluent is transferred to a pumping pit. From there, a pump move the liquids from this pit to a leach field located outside the powerhouse. A second pump acts as a standby for the first.

The septic tank, the pit and the pumps are located in the service area at level 8.85.

6.13.2.1.8 Drainage of oily water

This system insures the drainage of oily water from equipment that may leak oil, namely, the transformer and the units governors and oleo pneumatic systems.

The water is directed to a oil/water separator whose effluent is transferred to the system insuring the drainage of clear water described here above. The oil is confined in the separator from which it can be removed by a vacuum truck or any suitable pumping apparatus.

There is one separator in the service area of the powerhouse in the service area at level 4.14 and another in the transformers gallery at level 13.8.

6.13.2.1.9 Compressed air for general service

This system insures the supply of compressed air to the service stations located in the powerhouse and in the transformers gallery.

The compressors, the storage tank and all accessories are located in the service area at level 8.85.

6.13.2.1.10 Compressed air for the surge chamber

This system is provided to maintain an adequate pressure in the surge chamber by compensating the leaks through rock fractures and also the loss of air due to dissolution in the water.

Two compressors are provided with each one able to supply 100% of the required flow. The required pressure is calculated as 6.9 MPa.

6.13.2.2 HVAC

6.13.2.2.1 Introduction

This section describes conceptual engineering for heating, ventilation and air-conditioning of the hydroelectric power plant.

The HVAC systems have for objective to keep a contaminant free atmosphere for the safety and comfort of personnel. These systems, as per specifications, have the following functions:

- air change;
- evacuation of contaminated air;
- maintain of temperature and relative humidity;
- evacuation of smoke and pressurization of emergency exits in order to facilitate the evacuation of personnel and fire control.

6.13.2.2.2 Principles of ventilation

The principles of ventilation of major spaces are shown on the drawings.

In normal operation, the outside air required for the ventilation of the complex comes from ventilation units located in a mechanical room near the exit of the cable tunnel. In the mechanical room, the outside air is mixed with inside air from the complex to adjust the supply air temperature in order to cool the powerhouse, the transformer gallery and the cables tunnel. The supply air is delivered to the powerhouse and the transformer gallery through a plenum located in the cables tunnel.

For the needs of ventilation and safety exit, the cables tunnel is divided in two sections by a firewall:

- one section is reserved as a supply air plenum for the undergrounds installation and as emergency exit. This section is divided in two: a lower and an upper part. In normal operation, the lower and upper part are used as a supply air plenum. In case of fire, the upper part is pressurized with outside air for the pressurization of the lower part, which becomes an emergency exit, and for the pressurization of the galleries (see smoke exhaust chapter);

- the other section is used as a return air plenum from the undergrounds installations to the ventilation units and for the passage of the power cables.

A part of the air supplied to the underground is evacuated through the main access tunnel to assure a minimum air change and the evacuation of combustion gas from vehicles.

6.13.2.2.3 Design criteria

Temperature and ventilation

The temperature and ventilation criteria of inside rooms are given in Table 7.18

Table 6.21 Temperature and ventilation criteria

	Heating T.S. °C	Cooling T.S. °C	Outside Air Change*
Powerhouse	16	30	0.25
Transformer gallery	16	30	0.25
Office	21	25	1
Cables gallery	10	35	0.25

* Calculated for a height of 3 600 m

Climate

The outside temperatures used for the preliminary design are presented in the section “Site description”.

Rock temperature

The rock face is an important source of cooling which must be considered in the conception of HVAC systems. To establish the criteria, the heat transfer coefficient presented in the ASHRAE manual (Application Handbook, edition 1999, chapter 26) is used. Two coefficients have been retained for the conception of systems in heating and cooling mode. These coefficients take into account the evolution of the surface rock temperature. The stabilization period is estimated at three years:

- for the sizing of HVAC systems in the cooling mode, the heat transfer coefficient used is 0.57 W/m²°C;
- for the sizing of HVAC systems in the heating mode, the heat transfer coefficient used is 1.2 W/m²°C.

The rock temperature, at the depth of powerhouse, is estimated at 15°C.

Heat loss of production equipments

The permanent heat losses of the production equipments are estimated as follow:

Table 6.22 Major equipment heat loss – Powerhouse

Identification	Heat loss (kW)		
	Quantity	Per unit	Total
Lighting	---	---	25
Excitation cabinet	2	15	30
Excitation transformer	2	20	40
Bus duct	---	1,3 kW/m/3ph.	80
<i>Total heat loss</i>			<i>175</i>

Table 6.23 Major equipment heat loss – Transformer cavern

Identification	Heat loss (kW)		
	Quantity	Per unit	Total
Lighting	---	---	20
Power transformer	2	25	50
<i>Total heat loss</i>			<i>70</i>

Table 6.24 Heat loss – Cable Tunnel

Identification	Heat loss (kW)		
	Quantity	Per unit	Total
Lighting	---	---	10
Cables	7 200 m	0.015 kW/m/ph.	108
<i>Total heat loss</i>			<i>118</i>

Smoke exhaust

The principles of smoke evacuation are shown on the drawings.

In case of fire detection, the ventilation systems are used for the exhaust of smoke and the pressurization of emergency exits. According to the location of detection, the operation is as follow:

1. Fire in the transformer gallery

In case of fire in the transformer gallery, the emergency exit and the powerhouse are pressurized with outside air.

Following the fire alert, the smoke exhaust system is started, to remove the smoke from the transformer gallery. To compensate the air evacuated with smoke, the main access door and the transformer gallery door are opened.

2. Fire in the powerhouse

In case of fire in the powerhouse, the emergency exit and the transformer gallery are pressurized with outside air.

Following the fire alert, the smoke exhaust system is started to remove the smoke from the powerhouse.

To compensate the air evacuated with smoke, the main access door and the powerhouse door are opened.

3. Fire in the main access gallery

In case of fire in the main access gallery, the emergency exit, the transformer gallery and the powerhouse are pressurized with outside air.

Following the fire alert, the smoke exhaust system is started to remove the smoke from the main gallery.

To compensate the air evacuated with smoke, the main access door is opened.

6.13.2.2.4 Heating

The heating of major space and make-up air from outside is principally assured by the heat rejected by the electrical production equipments. Locally, in certain rooms, the heating is completed by the use of forced flow heaters or baseboards.

When required, the heat loss from one generator may also be used.

6.14 Hydro-mechanical equipment

This section describes the hydro mechanical equipment that will be provided for site 6g and gives the summary of the technical characteristics of this equipment.

6.14.1 Intake

6.14.1.1 Gate and Hoist

The water intake is equipped with one intake gate.

The Intake gate is of the fixed wheel type, with wheels mounted on taper roller bearings. It is fabricated from carbon steel welded construction with upstream skinplate and seals. It is designed to close on its own weight against the full incoming flow to the turbine. The seals are of the elastomer music note type with fluorocarbon cover that reduces friction.

The gate structure is designed and the number and location of wheels are selected in order to withstand the full pressure corresponding to the maximum upstream water level of 667.00 m, the downstream side of the gate being considered empty.

The gate guides are comprised of one upper light section from elevation 670.00.00 m down to about elevation 662.00 m that serves to guide the gate to the heavy lower section from elevation 662.00 to the sill elevation 655.00 m. This heavy section is comprised of one lintel beam, one sill beam and two heavy side guides that are designed to resist the full load of the wheels of the gate and transfer these loads to the concrete structures. The gate guides are made of carbon steel with a high strength machined wheel rolling path and machined stainless steel sealing surfaces.

Opening and closing of the gate the gate are made by a cable drum hoist located at the upper end of the gate shaft at elevation 670.00 m. The hoist mechanical components are designed to withstand the full maximum motor torque that will be limited to 210% of the motor nominal torque.

The hoist is equipped with an electro-mechanical brake that holds the gate in any position but mainly in the normal open position at 300 mm above the lintel.

Normal opening and closing of the gate are set at 1.20 m/minute but the hoist is also equipped with a fan brake that enables emergency closing of the gate, without electric power and by its own weight at a speed of twice normal closing speed.

The hoist is protected by a heated shelter

In order to avoid ice formation mainly on the inside surface of the upstream wall of the gate shaft, electric heating elements are inserted in tubes embedded in the wall of the shaft of the gate over the exposed height of the shaft above the water level at elevation 667.00 m

6.14.1.2 Trashracks

The trashracks are intended to prevent rocks and debris from entering the intake tunnel and eventually damaging the turbine.

The upstream guides are used mainly for the installation of the trashrack but they are also designed to enable the insertion of a set of stoplogs whenever maintenance is required on the intake guides, lintel and sill and the adjacent liners and concrete structure.

6.14.1.3 Stoplogs

One set of stoplogs is provided. The set of stoplogs is kept outside in a storing area provided on the downstream side of the shelter at elevation 670.00 m. The stoplogs are of carbon steel welded construction, with a downstream skinplate and seals. They are about 1.50 m high and are all identical and interchangeable except for the upper one that is equipped with a filling valve to fill the space between them and the gate. The stoplogs are designed to be put in place or removed only under dead water condition.

Handing of the stoplogs is made by a mobile crane and a lifting beam.

6.14.1.4 Summary of the characteristics of intake equipment

The following table gives the general characteristics of intake equipment.

Table 6.25 Summary of the characteristics of intake equipment

<i>Reference drawings</i>	A 401 A 402
<i>Basic data</i>	
W/L	667.00 m
Sill elevation	655.00 m
<i>Trashracks</i>	
Quantity	1 set
Nominal dimensions	W= 4 300 mm, H= 6 100 mm, t= 900mm
Number of sections	3
Total mass	22 000 kg
Lifting beam	1 500 kg

Stoplogs		Quantity	1 set
	Type	upstream skin plate downstream seals	
	Nominal dimensions	W= 4 300 mm, h= 6 100 mm, t= 900 mm	
	Number of stoplogs	4	
	Total mass	10 000 kg	
	Lifting beam	1 000kg (different from that for the trashrack)	
Embedded parts <i>(common for the trashrack and the stoplogs)</i>		Quantity	1set
	Total mass	12 000 kg	
Intake gate		Quantity	1
	Type	Fixed wheel , upstream skinplate and seals	
	Nominal dimensions	W= 3 750, H= 5 500 mm, t= 600 mm	
	Total mass	12 000 kg	
	Embedded parts	12 500 kg	
Gate hoist		Quantity	1
	Type	cable drum	
	Lifting capacity	160 kN	
	Power	4 kW	
	Mass	7 500 kg	
Concrete		Intake and shelter	760 m ³
Intake steel liner			8 000 kg
Gate shaft & shelter heating			(approx)100 kW

6.14.2 Powerhouse

6.14.2.1 Machine Room Crane

6.14.2.1.1 General Description

The machine room of the powerhouse is equipped with one crane which nominal lifting capacity is selected to lift the heaviest load to be handled in the powerhouse that is the alternator rotor.

The crane is a double girder, overhead electric travelling crane. It is equipped with one main hoist and one auxiliary hoist. The auxiliary hoist is overhanging on the side of one of the crane girder and it travels independently from the main hoist and along the bridge main girder.

The crane is operated either using a pendant control station or remote control by the operator standing and walking on the alternator floor at elevation 13.80 m.

6.14.2.1.2 Summary of the technical characteristics

The following table gives the general characteristics of the machine room crane:

Table 6.26 Machine room crane – General Characteristics

<i>Reference Drawing</i>	A 403
<i>Quantity</i>	1
<i>Type</i>	Double girder- Overhead Electric travelling
<i>Lifting capacity</i>	Main hook 265 metric ton Auxiliary hook 25 metric ton
<i>Class of service</i>	Bridge and main hoist Light duty- infrequent use Auxiliary hoist Heavy duty- frequent use
<i>Span</i>	14 350 mm
<i>Overall travelling distance</i>	41 500 mm (approx)
<i>Travelling speed</i>	Bridge 0-30 m/m Man hoist trolley 0-25 m/m Auxiliary hoist trolley 0-25 m/m
<i>Lifting Speed</i>	Main hook 0-1 m/m Auxiliary hook 0-10 m/m
<i>Lifting Height</i>	Main hook 18.00 m Auxiliary hook 18.00 m
<i>Elevation</i>	Top of rail 22.80 m Operator's floor 13.80 m
<i>Control</i>	Variable speed drives Pendant station moving along bridge girder Remote Radio control
<i>Installed Horsepower</i>	Bridge 45 kW Man hoist trolley 15 kW Auxiliary hoist trolley 4 kW Main hoist 56 kW Auxiliary hoist 45 kW

6.14.2.2 Tailrace gate

6.14.2.2.1 Description

One gate is provided to isolate either one of the turbine discharge tunnels in order to protect the powerhouse and the turbines from the high tide when the portion of the discharge tunnel in the powerhouse has to be emptied for maintenance purpose.

The tailrace gate is of a simple bulkhead type. It is fabricated from carbon steel welded construction with upstream skinplate, seals and bearing pads. The seals are of the elastomer music note type.

The gate is designed to be put in place and removed only under dead water condition. It is designed to withstand the hydrostatic load corresponding to the high tide water level, the upstream side of the discharge tunnel being considered empty.

One set of embedded guides is provided for each gate shaft. Each set of guides is made of two sections. One light section runs from the floor level of the transformer cavern at elevation 13.80 m and guides the gate down to the heavy lower section from elevation 5.1 m to the sill elevation 1.10 m. This heavy section is comprised of one lintel beam, one sill beam and two heavy side guides. The gate guides are made of carbon steel with a machined bearing path and machined stainless steel sealing surfaces.

The gate is fabricated in two sections and is handled, transferred from one gate shaft to another, and put in place by means of the gantry crane travelling the length of the transformer cavern at elevation 13.80 m.

The gantry crane hoist is of the fixed position and cable drum type, and is equipped with a lifting beam.

When not used, each section of the tailrace gate is stored at the upper end of the gate shaft, under the floor elevation.

6.14.2.2.2 Summary of the characteristics of tailrace equipment

The following table gives the general characteristics of tailrace equipment.

Table 6.27 Summary of the characteristics of tailrace equipment

<i>Reference drawing</i>		A 403
<i>Basic data</i>	W/L	5.05 m
	Sill elevation	1.10 m
<i>Tailrace gate</i>	Quantity	1
	Nominal dimensions	W= 3 000 mm, H= 3 200 mm, t= 300 mm
	Number of sections	2
	Total mass	2 500 kg
	Lifting beam	750 kg
<i>Embedded parts</i>	Quantity	1 set per turbine unit
	Mass	5 000 kg per set

<i>Gate lifting equipment</i>	
Quantity	1
Type	Gantry crane- fixed position, cable drum hoist
Lifting capacity	50 kN
Travelling speed	0-30 m/mn
Lifting speed	0-3 m/mn
Mass	3 000 kg

6.14.3 Tunnel No 1 Intake

6.14.3.1 Gates and hoists

The flow passing through the intake of tunnel No1 is controlled by a control structure containing two flow regulating gates.

Since the reliability of this regulating work is very important, two small gates have been provided rather than only one big one, so that if failure of one gate occurs, the flow can be regulated by the adjacent gate.

The gates are of the fixed wheel type. They are fabricated from carbon steel welded construction with upstream skinplate and downstream seals. The seals are of the elastomer music note type with fluorocarbon cover that reduces friction

The gate guides are comprised of one section from the sill elevation 660.00.00 m up to 668.00 m. This heavy section is comprised of one lintel beam, one sill beam and two side guides that are designed to resist the full load of the wheels of the gate and transfer these loads to the concrete structures. The gate guides are made of carbon steel with a high strength machined wheel rolling path and machined stainless steel sealing surfaces.

The gates are designed to close under its own weight and under the full flow passing through the tunnel when the water level is at elevation 682.00 m, the downstream side of the gate being considered empty.

When totally open, the gate are kept about 300 mm above the lintel beam.

Opening and closing of the gates are made at a speed of 0.600 m/min by a means of valve actuators with rising stem. These two gate operators are located in a heated shelter at the upper end of the gate shafts at elevation 683.00 m. The hoist structure and mechanical components are designed to withstand the full dead weight of the gate and frictions and the hydraulic load corresponding to the maximum water level of the tunnel intake canal.

6.14.3.2 Stoplogs

To complete the reliability of this control structure, two sets of stoplogs are provided, one upstream set that can be put in place in either one of the upstream shaft and one downstream set complete with embedded guides. Also to prevent ice formation in the gate shafts, the upstream shaft portion down to 2 m below the minimum water level is heated by means of electric heating elements inserted in tubes that are embedded in the concrete walls of the shaft.

Each set of stoplogs is comprised of two stoplogs that, when not used, are stored in the shelter at elevation 683.00 m and are transferred from one shaft to the adjacent shaft by a upstream and a downstream monorail equipped with a lifting beam.

6.14.3.3 Summary of the characteristics of Tunnel no 1 equipment

The following table gives the general characteristics of Tunnel no 1 equipment.

Table 6.28 Summary of the characteristics of Tunnel no 1 equipment

<i>Reference drawing</i>	A 404
<i>Basic data</i>	
Maximum W/L	682.00 m
Min water level	669.00 m
Sill elevation	660.00 m
hoist floor elevation	683.00 m
<i>Gates</i>	
Quantity	2
Type	fixed wheel, upstream skinplate downstream seals
Nominal dimensions	W= 2 500 mm, H= 3 750 mm, t= 600 mm
Number of sections	2 per gate
Mass of gate	8 500 kg each (total 17 000 kg)
Mass of embedded parts	2 sets @ 11 000 kg each (total 22 000 kg)
<i>Gate actuator</i>	
Quantity	2
Type	Electric valve actuator with rising stem
Lifting capacity	170 kN
Power	3 kW
Mass	3 000 kg
<i>Stoplogs</i>	
Quantity	2 sets
Type	Upstream: upstream skinplate and downstream seals downstream: downstream skinplate and seals
Nominal dimensions	W= 2 500 mm, H= 3 750 mm, t= 600 mm
Number of sections	2 per set
Mass	5 000 kg per set (total 10 000 kg)
Mass of embedded parts	4 sets @ 9500 kg each (total 38 000 kg)
<i>Monorail</i>	
Quantity	2
Type	Electric cable drum hoist
Lifting capacity	50 kN
<i>Concrete</i>	
Total volume	900 m ³
<i>Gate shaft and Hoist shelter heating</i>	100 kW

6.15 Harbor

6.15.1 Need for Harbor

It is foreseen to have three different phases for the use of harbor facilities at site 6g:

- Initial Phase: for unloading of initial personnel, preliminary camps and civil works equipment for building of camp(s), harbor structure(s), roads, tunnels etc.
- Construction Phase: for unloading personnel for civil works construction, consumables including arctic diesel, additional civil works equipment.
- Operation Phase: for consumables, personnel, spare parts and equipment necessary during the Operation Phase.

Initial phase

It is assumed that all equipment necessary for the initial civil works will be brought to the sites by large transatlantic ships, which will unload their cargo onto barges; which will then be beached on the shore, to transfer the equipment to the shore.

This is common practice for civil works of this nature.

Construction Phase

During this phase, it is expected that heavy equipment will be beached as above during the entire period. The harbor structure will be built and will be used for personnel and light goods when completed. The heavy civil works equipment will be landed by barge as in the Initial Phase.

Operation Phase

During the Operation Phase, it is foreseen that most equipment, personnel and consumables will be unloaded directly to a quay structure. This quay shall be designed for a vessel similar to the Pajuttaat, operated by Royal Arctic Line.

Large type equipment, which is brought to the site(s) by sea-going vessels or which is heavier than the allowable load on the quay, shall be unloaded to the beach by use of barges.

Oil products, like arctic diesel can be brought to the site either in barrels or directly to the quay and pumped to a tank farm.

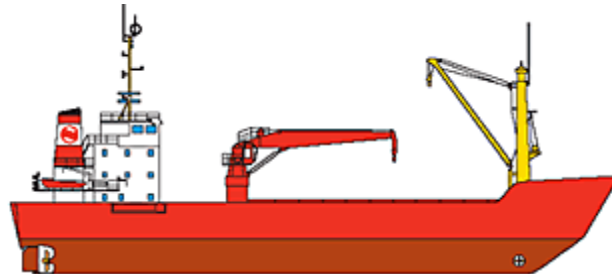
6.15.2 Design ship

During operation of the hydro power facilities equipment, personnel and consumables can be transported by ship to the quay, as mentioned above.

We have been in contact with the operations department of Royal ArcticLine (RAL), which has informed us that RAL is considering modernizing its fleet, serving the smaller towns. The "Design ship" for serving the hydro power facilities in Evighedsfjord and in Godthaabsfjord should be a type similar to the existing Pajuttaat, which can carry a number of 20 ft. containers and other general cargo.

The features for the Pajuttaat are shown on Figure 6.9.

Figure 6.9 Design ship for harbor (Pajuttaat)



Pajuttaat Specs (source: Royal Artic Line)

• ship type:	General Cargo ship with container capacity
• length (m):	63
• beam (m):	12
• draught (m):	3.71
• service speed (knots):	13
• number of containers (TEU+FEU) / loading capacity (m³):	18+4 / 1349
• reefer slot / Cold store capacity (m³):	12 / 396
• loading capacity (ton):	887
• cranes:	1 x 20 SWL + 1 x 30 SWL
• year:	1979

6.15.3 Water levels

Information on water levels and tidal variation is taken from reports published in 2008 by Asiaq for both site 7e and 6g. It is presented in Table 6.29.

Table 6.29 Water levels in Anavaik (Ujarassuit) – Site 6g

	Anavaik (Ujarassuit) Site 6g
Highest Astronomical Tide (HAT)	2.07 m
Mean High Water of Spring Tide	1.64 m
Mean Sea Level	-0.75 m
Mean Low Water of Spring Tide	-2.96 m
Lowest Astronomical Tide (LAT)	-3.46 m
Delay of the tidal wave, Maniitsoq (mean value)	Approx. 10 minutes

6.15.4 Proposed design basis for the quay structure

The design basis proposed for the conceptual/preliminary design of the quay structures in connection with the hydro power facilities for the Operation Phase is presented in tables 6.30 and 6.31.

Table 6.30 Water depth at quay at mean sea level

	Site 6g
LAT (from table above)	2.8 m
Extra over for wind setup/low pressure	0.5 m
Maximum draught	4 m
Keel clearance	1 m
<i>Necessary water depth</i>	<i>8.3 m</i>

Table 6.31 Quay height above mean sea level

	Site 6g
LAT (from table above)	2.8 m
Extra over for wind setup	0.5 m
Quay level above high water	1.4 m
<i>Necessary quay depth</i>	<i>4.7 m</i>

The quay length is set to 50 m, with bollard of 50 tons. The uniform load on the quay apron is 3 ton/m². The proposed quay plan is shown on drawing no. 104 of the set of drawings.

6.15.5 Beaching and harbor facilities

The proposed location for the harbor at site 6g in Godthabsfjord is shown on figures 6.10 and 6.11.

Figure 6.10 Proposed location of harbor at site 6g

Figure 6.11 Close up view of proposed location of harbor at site 6g



The photo on Figure 6.11 confirms the statements that the fjord – Ujarassuit (location of many stones) - is shallow and visibly turbid. The bottom of the fjord was described by the field team as very shallow with stones and rocks and possibly with soft sea bed materials.

There is moreover an indication of a possible threshold marked “Limit for icebergs” at which the icebergs apparently ground, again indicating shallow water.

Two possible locations have been studied for the quay structures. Each position has features for and against the location. One location has been shown for location of beaching.

It is essential for locating the harbor at the most feasible position that water depths are known in the area both in relation to the quay structure and for the navigability for ships using the quay. It is therefore essential that an in-depth bathymetric survey will be carried out in the area. These information were received shortly before issuing this report.

Figure 6.12 is shown on the following page.

6.15.5.1 Beaching and barge transport

Initial beaching of barges is envisaged on the beach at the bottom of the fjord. It is at this point of the project not possible to determine the most feasible location, because there is still missing information on the actual water depths in the bottom of the fjord.

Location for beaching during the construction will be determined based on the most feasible location for the quay structures.

6.15.5.2 Potential location for the quay

The potential locations for the quay are presented in tables 6.32 and 6.33. For all cases, the quay front shall be located at 8.0 m water depth, approximately 250 m from the shore. For location 6g-2, the contour lines have been assumed to be similar to the contours at location 6g-1, since no bathymetry is available at that site. All of the locations are shown on Figure 6.12.

Table 6.32 Position 6g – 1

Position 6g - 1	Pro	Con
Navigability	Easy access, bathymetry available	
Access to camp site and main roads for tunnel and dam work	Reasonable access	Possible road from quay to hinterland shall be cut in the steep rock side.
Access to storage area in valley		Small back area for quay can be established near the quay, by blasting a shelf.
Hinterland for camp	Reasonable conditions for establishing camp facilities on plateau.	

A harbor structure at location 6g-1 is considered acceptable, but back areas for storage near the quay shall be established on reclaimed land and access between quay and hinterland shall be established on the steep mountain slope.

Table 6.33 Position 6g - 2

Position 6g - 2	Pro	Con
Navigability	Acceptable because of the assumptions on water depth at the quay front.	Bathymetry not available
Access to camp site and main roads for tunnel and dam work	Easy access	Major reclamation for quay structure
Access to storage area in valley	Easy access to the sandy valley at the bottom of the fjord Back area for quay can be established near the quay	
Hinterland for camp	Reasonable conditions for establishing camp facilities on plateau above harbor	

Location 6g-2 is considered the most favorable location for the harbor structure and adjacent storage areas, provided that the ships will have access to the quay by having 8 m of water at the bottom of the fjord.

6.16 Access roads

6.16.1 General design basis

6.16.1.1 Road Types

Two types of roads are considered: primary roads and secondary roads. The primary roads will be used both in the construction phase and later on in the operation phase. All primary roads are shown on the drawings.

The secondary roads will mainly be used in the construction phase and are not shown on the drawings. The design of the secondary roads will be the responsibility of the contractor.

6.16.1.2 Phases

Two different phases for the use of the roads are envisaged:

- **Construction Phase:** The roads will be used for transportation of heavy loads, containers, equipments for civil works construction, fuel for vehicles, generators, materials and consumables for the camps and personnel, from the harbor or landing areas to tunnel adits, camps and dam areas.
- **Operation Phase:** The roads will be used for transportation of spare parts and equipment necessary during the operation phase. During the Operation Phase it is foreseen that most equipment, personnel and consumables will be unloaded at the harbor. It is expected that helicopters will be used in both phases for transport of light spare parts and personnel as a supplement to road transport. The helicopters shall operate from approved heliports.

6.16.1.3 Geotechnical conditions

There are no records of permafrost in the area of site 6g, but sporadic permafrost may be expected at high altitude. The roads are designed accordingly to the expected permafrost occurrence.

6.16.1.4 Traffic volume

No traffic forecasts are available. The following design criteria have been used.

During the Construction phase

- The primary roads are recommended to be designed for ten passages of heavy vehicles every day or 4 000 passages per year.

During the Operations phase

- The primary road between the harbor and the power station is recommended to be designed for two passages of heavy vehicles everyday or 500 passages per year.
- The primary road beyond the power station is recommended to be designed for two passages of heavy vehicles every week or 100 passages per year.

6.16.1.5 Vehicles and Axle Loads

The roads are expected to be used by ordinary heavy vehicles, and the pavement is planned for 10 ton axle load with two passages per passing vehicle, according to Danish Standards.

The bearing capacity of the roads must be sufficient for a heavy truck like a CAT 740. The empty weight is 33.1 tons and 72.6 tons when loaded. The truck has 6 wheels. It is expected to carry excavation materials at the construction sites outside the primary roads. The calculated pavement for ordinary heavy vehicle is sufficient for one passage per day with a loaded CAT 740. If the primary roads are used for heavy traffic with the CAT 740 each layer of the pavement and the total thickness has to be increased.

6.16.1.6 Road geometry

The road surfaces will be gravel-paved and shall be carried out with a transverse slope of 4% one-sided or double-sided. According to the literature⁸, the maximum longitudinal slope of roads is 10% at a length of 100 m only. As all roads are constructed outside urban areas and mainly as construction roads, the roads may have a maximum longitudinal slope of 10 % for longer distances and steeper slope, up to about 15%, for shorter distances.

Primary Roads

Primary roads will have one lane, 4-5 m wide with a 0.75 m wide shoulders, for a total of 5.5-6.5 m width. The 5 m wide roads will be used for permanent roads and for roads from the harbor to the main construction sites. The width is increased at narrow bends and in cut-sections to allow for transport of long units and 8 m wide units. Lay-bys are located at maximum distances of 500 m and with free sight. The road width at lay-bys is 8.0 m.

Secondary Roads

Secondary roads run from the primary roads to minor construction sites and borrow areas. They are designed by the contractor.

6.16.1.7 Cross sections

Typical cross sections for the primary roads are shown on drawing no- 103 of the drawings set.

In soil areas where permafrost is expected, the excavation is limited to a minimum to avoid thawing of the frozen soil. In these road sections, there will be no balance of cut and fill. Large quantities of NFS material must be brought in from borrow areas, unless the excavated soil is of NFS quality.

A study of the orthophotos has provided an indication of the surface and cross-section to use along the alignment.

In steep sloping areas the roads will have ditches (refer to section 6.15.1.9).

⁸ Greenland Home Rule, Roads, in Greenlandic towns, Directions for design and execution, Oct. 1987 (in Danish only)

6.16.1.8 Pavement

Based on the traffic volume and axle load mentioned above and a calculation of bearing capacity, the recommended pavement thickness is shown in Table 6.34 below. The material for the wearing course is mechanically stabilized gravel size 0-32 mm. Base course can be the same or screened gravel size 0-50 mm. The sub-base and filter can be selected gravel size 0-80 mm or screened gravel 16-80 mm. The fill below the pavement is selected gravelly soil with less clay and silt.

Rock fill is used at steeper rock surfaces. Dents in the surface are filled with minor stones or screened material before the pavement is placed. In areas with heavy traffic with the CAT 740 truck, each layer of pavement and the total thickness have to be increased.

6.16.1.9 Water control

Where the surrounding terrain is rising from the road side, water control measures are included with ditches at a short distance from the road embankment. Where the road is located on fill and the surrounding terrain is sloping away from the road side, no water control measures are specified.

Embankment slope shall be 1:2, if possible. Where the roads cross springs, streams, rivers, etc., different kinds of structures are used:

- bed-level causeways constructed by stones or concrete at a width of 8 m, when the water flow is limited and the water depth can be limited to 0.15 m;
- vented causeways with culverts and concrete superstructure (Irish bridges) where the stream and water flow are larger than above, and where the water flow over the structure can be limited to 0.15 m;
- regular bridges or rock-fill embankments with large diameter culverts where the road crosses major streams or rivers.

Culverts and bridges shall be designed for axle and vehicle loads as mentioned in section 6.16.1.5.

6.16.1.10 Maintenance

Autumn maintenance should be carried out just before the winter season. The cross slope at a gradient of 4% and road shoulders are graded even, in order to allow water to flow from road surface into the ditches. The maintenance also includes cleaning out any poorly working ditches and checking culverts for debris etc.

Spring maintenance should be carried out in early spring and consists mainly of de-icing of culverts and ditches filled with ice. During the spring, dust binding may be done using salt (CaCl₂). When the snow has melted the gravel roads are reshaped using a grader. Bed-level causeways are inspected annually, every spring and maintained when necessary. Other culverts and bridges are inspected regularly and maintained when necessary.

During the construction phase, all roads should be maintained as described above.

During the operation phase, only a minor part of the primary road (from the harbor to the power station) is maintained regularly.

The remaining primary and secondary roads will be maintained only if required for new construction or repair works.

6.16.2 Preliminary Design

The design of the roads is based on the alignments shown on drawing 102 of the drawing set. A 3D-model is also received and is prepared for calculation of fill and cut along the planned roads for site 6g.

Longitudinal sections of each part of the roads are prepared based on the information in the 3D-model and orthophotos, and cut and fill quantities are calculated at site 6g using the proposed cross-sections (drawing no. 103).

The alignment is divided into eight parts, for the various structures. For each part, longitudinal sections have been prepared to fit the terrain. The number of water body crossings has been evaluated with orthophotos.

Typical cross sections are shown on drawing no. 103 of the drawing set. The most suitable cross-section was then applied to every stretch of the road, as presented in Table 6.34, along with the cut and fill quantities for every stretch. The number of water body crossings is also presented in Table 6.34.

Table 6.34 Road design at site 6g (by Niras)

Stations	Length (m)	Road type	Width (m)	Pavement (m)	Soil excavation (m ³)	Fill (m ³)	Rock cut (m ³)	Rock fill (m ³)
<i>Harbor to intake</i>								
0+000	0+330	330	C3	5 (6.5)	0.25		18.943	0
0+330	0+500	170	F1	5 (6.5)	0.65	236	3.593	
0+500	1+150	650	C3	5 (6.5)	0.25		37.312	0
1+150	1+300	150	F2	5 (6.5)	0.65	87	3.170	
1+300	2+500	1,200	F2	5 (6.5)	0.65	693	25.364	
2+500	2+900	400	F2	5 (6.5)	0.65	231	8.455	
2+900	3+450	550	F3	5 (6.5)	0.65	445	3.240	
3+450	4+000	550	F2	5 (6.5)	0.65	318	11.625	
4+000	5+100	1,100	F3	5 (6.5)	0.65	889	6.480	
			VC	3	nos.			
5+100	5+990	890	C2	5 (6.5)	0.25		7.093	5.859
5+990	6+100	110	F2	5 (6.5)	0.65	64	2.325	
			VC	2	nos.			
6+100	6+340	240	C2	5 (6.5)	0.25		1.913	1,580
6+340	6+370	30	Bridge	5 (6.5)				
6+370	6+950	580	F3	5 (6.5)	0.65	469	3.416	
			VC	2	nos.			
6+950	7+100	150	C2	5 (6.5)	0.25		1.195	988
7+100	7+600	500	C3	5 (6.5)	0.25		28.702	0
7+600	7+800	200	F2	5 (6.5)	0.65	116	4.227	
7+800	8+230	430	C3	5 (6.5)	0.25		24.684	0
8+230	9+300	1,070	C2	5 (6.5)	0.25		8.527	7,044
9+300	10+000	700	C1	5 (6.5)	0.25		2.911	1,860

Stations	Length (m)	Road type	Width (m)	Pavement (m)	Soil excavation (m ³)	Fill (m ³)	Rock cut (m ³)	Rock fill (m ³)
		VC	1	nos.				
10+000 10+500	500	F2	5 (6.5)	0.65	289	10.568		
10+500 10+950	450	F3	5 (6.5)	0.65	364	2.651		
10+950 11+250	300	F2	5 (6.5)	0.65	173	6.341		
11+250 11+450	200	F3	5 (6.5)	0.65	162	1.178		
11+450 11+900	450	C1	5 (6.5)	0.25			1.871	1.195
		VC	1	nos.				
11+900 13+200	1,300	F1	5 (6.5)	0.65	1.802	27.477		
13+200 13+650	450	F2	5 (6.5)	0.65	260	9.511		
13+650 14+300	650	F3	5 (6.5)	0.65	526	3.829		
14+300 14+700	400	F1	5 (6.5)	0.65	554	8.455		
14+700 15+500	800	F3	5 (6.5)	0.65	647	4.712		
		VC	1	nos.				
15+500 15+900	400	F1	5 (6.5)	0.65	554	8.455		
15+900 16+100	200	F3	5 (6.5)	0.65	162	1.178		
16+100 16+600	500	F2	5 (6.5)	0.65	289	10.568		
		VC	1	nos.				
16+600 16+950	350	F1	5 (6.5)	0.65	485	7.398		
16+950 18+200	1,250	F3	5 (6.5)	0.65	1.011	7.363		
18+200 18+500	300	F1	5 (6.5)	0.65	416	6.341		
		VC	1	nos.				
18+500 18+650	150	F2	5 (6.5)	0.65	87	3.170		
18+650 18+754	104	C2	5 (6.5)	0.25			829	685
	<i>Subtotal</i>	<i>18,754</i>			<i>11,329</i>	<i>191.090</i>	<i>133.980</i>	<i>19.211</i>
<i>Intake to Lake Imarsuaq</i>								
0+000 0+300	300	F3	4 (5.5)	0.45	254	1.670		
		VC	nos.					
0+300 0+520	220	F1	4 (5.5)	0.45	186	3.887		
0+520 0+610	90	C2	4 (5.5)	0.25			610	468
0+610 1+420	810	F3	4 (5.5)	0.45	686	4.508		
1+420 1+550	130	C2	4 (5.5)	0.25			881	676
1+550 1+750	200	F1	4 (5.5)	0.45	169	3.533		
1+750 1+810	60	C2	4 (5.5)	0.25			407	312
1+810 1+910	100	F3	4 (5.5)	0.45	85	557		
1+910 2+200	290	C1	4 (5.5)	0.25			1.053	597
2+200 2+470	270	F3	4 (5.5)	0.45	229	1.503		
2+470 2+550	80	C1	4 (5.5)	0.25			290	165
2+550 3+000	450	F1	4 (5.5)	0.45	381	7.950		
		VC	1	nos.				
3+000 3+390	390	C1	4 (5.5)	0.25			1.416	802

Stations	Length (m)	Road type	Width (m)	Pavement (m)	Soil excavation (m³)	Fill (m³)	Rock cut (m³)	Rock fill (m³)	
3+390 3+500	110	F3	4 (5.5)	0.45	93	612			
3+500 3+880	380	C1	4 (5.5)	0.25			1.379	782	
3+880 4+000	120	C2	4 (5.5)	0.25			813	624	
4+000 4+940	940	C1	4 (5.5)	0.25			3.412	1,934	
4+940 5+010	70	Bridge	4 (5.5)						
5+010 5+800	790	C1	4 (5.5)	0.25			2.868	1,625	
5+800 5+900	100	F3	4 (5.5)	0.45	85	557			
		VC	1	nos.					
5+900 6+840	940	C1	4 (5.5)	0.25			3.412	1,934	
6+840 8+300	1,460	F3	4 (5.5)	0.45	1.237	8.126			
8+300 8+460	160	C1	4 (5.5)	0.25			581	329	
8+460 8+640	180	F3	4 (5.5)	0.45	152	1.002			
8+640 9+000	360	F2	4 (5.5)	0.45		6.360			
9+000 9+390	390	F1	4 (5.5)	0.45	330	6.890			
9+390 10+300	910	C3	4 (5.5)	0.25			43.604	0	
		VC	1	nos.					
10+300 10+400	100	F1	4 (5.5)	0.45	85	1.767			
10+400 11+000	600	F2	4 (5.5)	0.45	0	10.600			
		VC	1	nos.					
11+000 12+630	1,630	F3	4 (5.5)	0.45	1.381	9.073			
		VC	2	nos.					
12+630 12+800	170	C1	4 (5.5)	0.25			617	350	
		VC	1	nos.					
12+800 12+950	150	F3	4 (5.5)	0.45	127	835			
12+950 13+100	150	C2	4 (5.5)	0.25			1.016	780	
13+100 13+930	830	F3	4 (5.5)	0.45	703	4.620			
13+930 14+100	170	F3	4 (5.5)	0.45	144	946			
		VC	1	nos.					
14+100 16+150	2,050	F3	4 (5.5)	0.45	1.736	11.410			
		VC	1	nos.					
16+150 16+730	580	C1	4 (5.5)	0.25			2.105	1,193	
16+730 17+070	340	F3	4 (5.5)	0.45	288	1.892			
17+070 18+100	1,030	F2	4 (5.5)	0.45	0	18.196			
		VC	1	nos.					
18+100 18+350	250	F2	4 (5.5)	0.45	0	4.417			
18+350 18+510	160	C2	4 (5.5)	0.25			1.084	832	
18+510 18+800	290	F1	4 (5.5)	0.45	246	5.123			
18+800 19+000	200	C2	4 (5.5)	0.25			1.355	1,041	
19+000 19+600	600	F3	4 (5.5)	0.45	508	3.340			
19+600 19+805	205	C2	4 (5.5)	0.25			1.389	1,067	
<i>Subtotal</i>					<i>19,805</i>	<i>9,105</i>	<i>119.374</i>	<i>68.292</i>	<i>15.511</i>

Stations	Length (m)	Road type	Width (m)	Pavement (m)	Soil excavation (m ³)	Fill (m ³)	Rock cut (m ³)	Rock fill (m ³)	
<i>Access road to Canals 1 and 2</i>									
0+000	0+400	400	F3	4 (5.5)	0.45	339	2.226		
0+400	0+750	350	F2	4 (5.5)	0.45	0	6.183		
0+750	1+070	320	F3	4 (5.5)	0.45	271	1.781		
1+070	1+200	130	F2	4 (5.5)	0.45	0	2.297		
1+200	2+710	1,510	F3	4 (5.5)	0.45	1,279	8.405		
			VC	1					
2+710	3+100	390	C2	4 (5.5)	0.25		2.643	2,029	
3+100	4+280	1,180	F3	4 (5.5)	0.45	999	6.568		
			VC	2					
4+280	4+800	520	F2	4 (5.5)	0.45	0	9.186		
4+800	5+197	397	F1	4 (5.5)	0.45	336	7.013		
<i>Subtotal</i>		<i>5,197</i>				<i>3,224</i>	<i>43.659</i>	<i>2.643</i>	<i>2.029</i>
<i>Access road to Dam 5</i>									
0+000	0+878	878	C2	4 (5.5)	0.25		5.949	4,568	
<i>Access road to Canal 4</i>									
0+000	0+500	500	C1	4 (5.5)	0.25		1.815	1,029	
0+500	1+000	500	C2	4 (5.5)	0.25		3.388	2,602	
1+000	1+500	500	C1	4 (5.5)	0.25		1.815	1,029	
1+500	2+047	547	C2	4 (5.5)	0.25		3.706	2,846	
<i>Subtotal</i>		<i>2,925</i>	<i>0</i>				<i>16.673</i>	<i>12.074</i>	
<i>Bed level causeways</i>		<i>122</i>	<i>nos.</i>						
<i>Grand total</i>		<i>46,681</i>				<i>23,658</i>	<i>354.123</i>	<i>221.588</i>	<i>48.825</i>

6.17 Construction Camps

6.17.1 Camp capacity

The projected capacity of the camps is estimated and is listed in Table 6.35. They are planned to be used during all of the construction period. Space for the transmission line workers is included in the projected capacity of the camps.

Table 6.35 Projected capacities of camps

	Camp 1	Camp 2	Camp 3	Camp 4
Workers	306	162	111	132
Staff	102	54	37	44
<i>Total</i>	<i>408</i>	<i>216</i>	<i>148</i>	<i>176</i>

The work will be carried out in shift work. The accommodation buildings are based on 100% capacity, while the camp facilities are based on 70% capacity, corresponding to the working day shift. The service buildings must then accommodate respectively 286, 152, 104 and 124 persons at the same time for camps 1, 2, 3 and 4.

The camps shall be self supporting with power and water supply while the sewage treatment plant (STP) and waste disposal are common for the camp and the construction site. The camp will consist of a number of accommodations blocks, service facilities such as kitchen, dining and laundry (KDL), recreation hall (RH), offices, storage facilities and workshops. A helicopter pad is included in the layout.

At the beginning of the first year of construction, a “starter” camp will be constructed at camp 1, which shall accommodate 70 people (60 workers and 10 staff).

All buildings and other facilities shall be designed to a Greenlandic standard observing the Greenland Building Code.

6.17.2 Description of camps facilities

6.17.2.1 Camp area layout

At site 6g, four camps will be constructed:

1. Camp 1 near the powerhouse and the tailrace tunnel exit at sea level, where workers for the powerhouse and adjacent tunnels will live.
2. Camp 2 near the intake structure, near the shores of Lake Tussaap Tasia. Workers for the intake structure, Dam 1, Dam 2 and Spillway 1 will stay at this camp,
3. Camp 3 near the Tunnel 1 on the shores of the Little Lake. Workers for Tunnel 1, Canals 1 and 2, and Dam 3 will stay at this camp,
4. Camp 4 at the northern end of Lake Imarsuaq, only accessible by water route. Workers for Dams 4 and 5, Canals 3 and 4 and Spillway 2 will live at this camp.

The layouts for the camps are shown on drawings no. 105 to 108. The layouts of the camps consider a minimum of ground leveling. The long accommodation buildings are located along the topographical contours and the larger service buildings are located on flat areas.

The material from rock excavation for the accommodation buildings are used for leveling the ground for the service buildings.

Each camp consists of four areas:

1. central area with RH, KDL, offices;
2. accommodation area north;
3. accommodation area south;
4. industrial area with storage, workshops and power station.

The distance between the buildings must comply with the distance criteria in the Greenland Building Code. They are shown in Table 6.36.

Table 6.36 Distance criteria for camp buildings

Distance criteria in meters	Wooden facing	Steel facing
Accommodation buildings with more than 10 beds	10	5
Buildings more than 600 m ² or with more than 50 persons assembled	10	5
Buildings containing inflammable storage, power stations, fire stations etc.	10	5
Other buildings one storey	5	2.5
Other buildings, more than one storey	5	3.5

For calculating the distance between two buildings the distance criteria for both buildings must be added.

6.17.2.2 Accommodation buildings

The accommodation buildings are constructed of two story modules.

Each building comprises:

- sleeping rooms having an area of 9 m²;
- living rooms;
- private bathrooms, toilets and washbasins;
- washing machines and dryers;
- two modules for staircase.

Each building contains a total of 58 modules, each of them 2.9 m × 8.4 m (24.36 m²). The modules are constructed with timber, with a wood facing and insulation for arctic climate. The foundations are steel beams on gravel pads. The beams are tied to anchors in the gravel pad or rock anchors.

The buildings for workers will be equipped with two bed sleeping rooms, while the buildings for the management staff will have single rooms.

The distance between two accommodation buildings is not less than 20 m for fire safety. Each sleeping room must be a fire cell and each wing with 20 rooms is considered a fire section. Each room is equipped with fire alarm system, and a fire hose with water pressure is installed in each corridor

All rooms are heated with electrical radiators. Hot water is produced in water heaters in each module. Water installations are limited to the central common rooms with bath, toilets and laundry facilities.

6.17.2.3 Service buildings

Office for camp administration and infirmary

One two storey building, constructed by the same type of modules as the accommodation buildings, contains offices and rooms for the infirmary. Foundation, heating, fire safety and other installations are as for the accommodation buildings.

Kitchen, dining and laundry (KDL)

The dining hall is designed to accommodate the number of workers during the day shift all at the same time (286, 152, 104 and 124 persons respectively for camps 1 to 4). The necessary area is estimated to 1.2 m² per seat. The kitchen area including day storage is estimated to 50% of the dining area. The laundry area is estimated to one module (2.9 x 8.4 m) per 200 people.

The KDL is arranged in one large building constructed by modules like the accommodation buildings or as a steel frame building with insulation and corrugated steel facing. The foundation is steel beams on a gravel pad. A small KDL is first established at the “starter” camp.

All of the buildings are heated with electrical heaters.

Recreation hall

The recreation facilities comprise gymnasiums, meeting hall, bar, shops, internet suite, etc. The facilities are included in one or two buildings next to the KDL. The area is estimated to 1.5 m² per person. Construction and heating of the buildings are similar to the KDL.

6.17.2.4 Storage facilities

The supply of goods will take place by ship to the harbor in Godthabsfjord.

The container storage area at the harbor is limited and the containers for the camp must be moved to the camp area shortly after the ship arrival. Between ship arrivals, the containers are stripped to the storage buildings or directly to the kitchen storage rooms. The storage facilities in the camp comprises container yards and three storage buildings for cold storage (unheated), heated storage and refrigerated storage respectively.

Local storage of gasoil for the power station, heating and vehicles is established.

The storage buildings are constructed by steel frames with corrugated steel facing. The foundation is steel beams on gravel pads and ground anchors may be necessary. The heated storage and refrigerated storage buildings are insulated, and the heating of the buildings is with oil fired unit heaters.

6.17.2.5 Workshop and garage

One building similar to the storage buildings comprise workshops for camp maintenance, mechanical, carpenter etc. The electrical workshop is included in the power station building. A garage for vehicle repair and firefighting equipment is also established in a steel building similar to the storage buildings.

6.17.2.6 Power supply

The design criterion for the power supply is to provide 8 kW of power per person. Diesel generators to meet the required power at each camp are provided. The generators are placed in an insulated steel building which also holds switchboard and operation panel for the power station, electrical workshop and water treatment plant.

6.17.2.7 Water supply

The water supply comprises a reservoir, a raw water pipeline, water treatment plant, storage tank for treated water and water mains to consumers. The water consumption is estimated to 200 liters per person per day.

The reservoir will consist of an insulated storage tank designed for a 1 month capacity of water. The water will run from the reservoir to all the buildings by gravity through a siphon and pipelines. The pipelines are pre-insulated and heat traced, placed on sleepers on the ground. The pipelines will cross the roads in culverts.

At the site of camp 1, it is planned to pump water from a small temporary reserve created by damming the small river near the camp, while for camps 2 to 4, it is planned to pump water directly from the adjacent lakes.

The water treatment plant is located in the same building as the power station. Water quality from the lakes varies over the year. It must be taken into consideration when dimensioning the water treatment that in the snow melting period the level of organic matter will rise (called "Flom").

The water is treated to obtain a water quality accordingly to Greenlandic standards.

6.17.2.8 Waste water

All waste water is collected in a sewer system and led to a sewage treatment plant (STP). Sewer mains consist of insulated pipelines from all buildings to the STP. The sewer mains will follow the same alignment as the water mains. The daily volume to be treated corresponds to the projected water consumption of 200 liters per person per day.

6.17.2.9 Waste disposal

The waste from the camps comprises:

- combustibles material, wood, garbage, waste oil, etc.;
- metal scrap;
- hazardous and toxic waste;
- STP sludge.

The production of waste in Greenlandic towns is estimated to 1.5-2 kg per person per day. A value of 2 kg per man is used for the estimation of the waste volume to be produced at each camp. The proposed solution is a containerized incinerator.

6.17.2.10 Roads

The roads include:

- primary road between the construction site and the central part of the camp;
- secondary roads between camp centre and buildings.

The primary road is design for heavy traffic with two lanes, for a total width of 6 m, and 1.5 m shoulders. The pavement is a thickness of 0.65 m of gravel. The distance between the centerline of the roads and buildings shall be not less than 10 m.

All accommodation buildings are connected with the primary road by secondary roads. They are used during construction of the buildings, but shall be maintained for light vehicle traffic for the fire engine and in connection with repair, waste removal etc. The width of the secondary roads is 4 m and 1.5 m shoulders.

Outside entrances to the buildings and level area of gravel fill serves as parking area and turning area at dead end roads.

A gravel area with a diameter of 22.2 m is also established near the camp for the heliport. It is equipped with wind sock and fire extinguishers.

6.17.2.11 Telecommunications

The telecommunication comprises telephone, cellular phone transmission, internet connection, TV signals and radio broadcast.

All telecommunication will be ensured by a link to the existing radio link (TELE-Greenland) via a repeater station. The repeater station must be established on a mountain top at the same standard as a normal TELE Greenland – site. It includes a diesel generator with intermediate operation with solar battery and traditional batteries. The capacity is 8 x 2 Mbps un-doubled.

A cable from the telecom mast leads to all buildings following the power cable alignment. The cable also carries alarm signals to a central room in the office building.

Stationary telephones are installed in the office building, RH, KDL, garage, workshops and power station. In the accommodation buildings only one stationary telephone is installed. All other telephone connection is by cellular phone to a transmitter on the telecom mast.

TV connections are installed in the RH, and TV rooms in the accommodation buildings. Hotspots are established in all buildings for Wi-Fi internet connection. A number of local and international radio stations will be broadcasted from a transmitter on the telecom link mast, facilitating radio listening all over the area including the construction site.

6.17.2.12 Vehicles

The fleet of vehicles for operation of each of the camp includes:

- double cabin 4WD pick-ups, for managers, maintenance, cleaning, power house, security, etc.;
- trucks with crane for transportation and snow clearing;
- forklift truck for handling containers;
- tractor for waste handling and snow clearing;
- snow clearing equipment for trucks;
- fire engine.

6.17.3 Summary of facilities

The facilities that will be included in camps 1 to 4 are presented in Table 6.37, with their respective area.

Table 6.37 Camp facilities at site 6g

	Camp 1		Camp 2		Camp 3		Camp 4	
	Nb.	m ²	Nb.	m ²	Nb.	m ²	Nb.	m ²
<i>Site preparation (including roads)</i>		250 m x 250 m = 62,500 m ²		250 m x 250 m = 62,500 m ²		240 m x 240 m = 57,600 m ²		100 m x 300 m = 30,000 m ²
<i>Infrastructures</i>								
• Power station (4.1, 2.2, 1.5 and 1.8 MW)	1	700	1	600	1	500	1	400
• Water supply plan	1		1		1		1	
• Sewer plan	1		1		1		1	
• Incinerator	1		1		1		1	
• Telecommunications	1		1		1		1	
<i>Sleeping buildings</i>								
• Workers : 40 modules/building (2 beds/room)	2	2 825	1	1 410	1	1 410	1	1 410
• Staff : 40 modules/building (1 bed/room) (Common rooms included-18 modules/building)	2	2 825	1	1 410	1	1 410	1	1 410
<i>Service buildings</i>								
• Offices	1	292	1	146	1	97	1	122
• Dining	1	341	1	195	1	122	1	146
• Kitchen (including day storage)	1	171	1	97	1	61	1	73
• Laundry	1	73	1	49	1	24	1	24
• Recreation hall	1	428	1	227	1	155	1	185
• Infirmary	1	49	1	24	1	12	1	24
<i>Workshops</i>								
• Mechanical	1	200	1	150	1	100	1	100
• Electrical	1	200	1	150	1	100	1	100
• Carpenter	1	200	1	150	1	100	1	100
Garage & Fire fighting (incl. fire fighting equip.)	1	400	1	300	1	150	1	200
<i>Storage buildings (2 months + 1 mth reserve)</i>								
• Frozen	1	10 344	1	2 357	1	1 615	1	1 920
• Cold (refrigerated)	1	10 344	1	2 357	1	1 615	1	1 920
• Dry (non perishable)	1	22 412	1	5 107	1	3 499	1	4 161

7 Project schedule

7.1 Introduction

The prefeasibility project schedule yields a 5 year construction program. The project schedule confirms the feasibility of the project as per Alcoa's parameters for site 6g. The project schedule is presented in Appendix 1 of the current report in a summarized high level view. This present section will describe the construction methodology, assumptions, schedule highlights and preliminary critical path.

7.2 Schedule Elements

The attached schedule has been produced in MS Project and is displayed in a Gantt bar chart format with supporting legend. The defined summary activities viewed include activity description, duration and proposed start and finish dates. These activities and summaries represent the high level critical components, including schedule dependencies and constraints.

The project schedule is represented by groups by types of Work.

- Tunnels
- Powerhouse
- Headrace
- Intake
- Spillways
- Dams
- Canals

7.2.1 Schedule Assumptions

7.2.1.1 Daylight/Climatic Conditions

- The summer period has extended daylight, permitting 2 daytime working shifts;
- The limitation of Winter daylight and climatic conditions reduce the amount of above ground construction work;
- Elements such as wind, snow and rain are considerations in the present schedule;
- Helicopter lifting and travel is limited to daytime operation;
- Due to fjord freezing and sea ice , shipments by sea will be received at site between June and October
- Some underground works such as tunnel excavation, concreting and turbine installation are not weather nor daylight dependent allowing for continual activity.

7.2.1.2 Workforce Schedules

- Workcrew 400 hours on and 2 weeks off rotation
- Staff: 1 Month on and 2 weeks off rotation
- 26 working days per month
- 6 day work crew
- 10 hours work shift
- All underground work is calculated on 2 shift per day and is not weather dependent

7.2.1.3 Road Construction Methodology

Many sections of the access road require rock excavation on steep hill side rock terrain. In order to shorten the construction duration of the road, is proposed to start with a 4 meter wide penetration track permitting a faster progression and access for the equipment required to widen the road to final dimensions at a later date.

Some sections can be reached only by air. The 4 meter penetration track is then realized by air lifting construction equipment, materials and personnel by helicopter

7.2.2 Seasonal Work Activities

The main construction activities to be conducted during each season are outlined below.

Summer

- Roads
- Excavation (rock and overburden)
- Backfill
- Tunneling
- Concrete

Seasonal Transition Periods (Spring and Fall seasons)

- Tunneling
- Concrete (with outside shelter)

Winter

- Tunneling
- Rock Excavation and blasting (1 daylight shift)
- Concrete (with outside shelter)

The productivities of the major activities are presented in the table below. The estimated durations are a total amount per type of activity. Excavation shifts follow along with anticipated sea shipment volumes.

Table 7.1 Major Activities Productivities

Name	Month	Day	Days per month
TBM's (Tunnel Boring Machine) excavation (QTY 2) excluding installation and removing)	650	25	26
Drill and Blast excavation (30 to 100 m ² – access tunnels)	120	+/-5	26
Drill and Blast excavation (17.4 m ² – cable tunnels)	56	+/-2	26

- Earth Moving / Rock excavation: 20 hours@2 Shifts per day if required (Seasonal daylight restrictions);
- Typical shipment by sea: 10,000 metric tons of materials and 20 000 m³ of equipment or a combination of both.

The total excavation volumes along with the main quantities to be used during construction for the five years of the project are presented in the following table.

Table 7.2 Main Quantities

Name	M ³	Tons	Liters
Tunnels excavated by TBM	225 000		
Tunnels excavated by Drill and Blast	346 000		
Open surface excavation Rock	102 100		
Open surface excavation Overburden	232 000		
Dam Rockfill Volume	533 000		
Asphalt Rockfill Volume	4 700		
Concrete	14 900		
Cement		5600	
Rebar		980	
Bitumen		555	
Structural Steel		160	
Steel Liner		460	
Fuel Requirements			56 200 000

7.2.3 Workers and Staff

The total project man hours requirement is the following :

- Workers = 2,400,000 Man Hours
- Staff = 1,159,200 Man Hours

7.3 Program

7.3.1 Critical Items

The isolated fjord is where the first roll out of the project will begin. The site is accessible after June thaw to establish a pre camp, unloading of ship(s) for supplies and equipment.

After establishment of preliminary pre camp and “beaching of equipment” The major activity is the road building and excavation inland towards construction sites and camp.

Based on the schedule provided and analysis the primary critical path are identified as follows.

- Establishing the initial pre-camp and port site
- Road construction to establish (4) work base camps

7.3.2 Primary Logistics

The project site is in a remote location which is dependent on well planned and executed logistic support. Both sea and air support are critical to establish and then continually sustain the project site and operations.

7.3.3 Sea Shipments

Including the initial site beach landing and camp set up, 11 sea shipments are planned over the 5 years project duration. There are 4 sea shipments of 20,000 m³ and 7 material shipments of 10,000 metric tons. Temporary floating docks and transport barges will be utilized to move materials to the project camp site from the ships. This will provide construction equipment and materials for fabrication of aggregates, concrete, rebar, asphalt, fuel and explosives.

Construction equipment, temporary installations, transportation vehicles and camp modules will also be by sea shipments.

7.3.4 Air Support

Personnel working on the project will arrive and depart at Kangerlussuaq, the Greenland International airport. Shuttling of work crews to the main project work sites will be via helicopter. This is the existing condition for the entire project including the 2 hydro projects, the transmission lines and the smelter. Weekly incoming Air cargo shipments to the local airport are required in order to deliver such items as perishable food and other required items. The helicopter will then transport these items and personnel to project sites.

Helicopters are needed for the transmission line construction including heavy type craft. An overall logistic operation for the whole project could be foreseen regarding the air support needs. A good coordination between all the construction sites would represent definite cost saving. This is taken into consideration in the pricing of the utilization of helicopters for access road sections not accessible otherwise. In this case, all transportation of workers, construction equipment and materials is done by helicopter.

7.3.5 Inland Water Route

The road construction to Camp 4 is over difficult terrain therefore the use of an Inland water passage and Ice Bridge has been incorporated in order to optimize schedule and cost.

Tug boats and tow barges will be used to move equipment and materials

Personnel will be transported to work site via outboard motor boats.

Conventional transport vehicles will be used during the season of the ice bridge.

7.3.6 Major activities start time

The following table indicates the start time of the major activities. The dates are shown in months after receipt of order.

Table 7.3 Estimated start date for construction activities

Name	Months ARO (After receipt of order)
<i>Year 1</i>	
Start Pre Camp, Floating docking facilities	6
Start Tunnels	11
Start Powerhouse	11

Name	Months ARO (After receipt of order)
Complete Order Procurement TBM (Tunnel Boring Machine)	13
<i>Year 2</i>	
Start Establish Camp 2	17
Start TBM Erection	18
Start Intake	20
Start Spillway 1	20
Start DAM 1	24
Start DAM 2	21
Start Head Race (TBM) Boring	21
<i>Year 3</i>	
Establish Start Camp 3	29
Start Canal 1	32
Start Canal 2	35
Start Tunnel 1	31
Start DAM 3	32
Start Establish Camp 4	31
Start Canal 4	33
Start Canal 3	32
Start Dam 5	32
Start Dam 4 and Spillway 2	33
<i>Year 4</i>	
Start Intake Structure	40
Start Dam 1 Diversion Tunnel plugs and Grouting	44
Start Tunnel 1 Regulating Structure	37
Start Dam 3, 5 and 4 Rock Fill	42
<i>Year 5</i>	
Complete Wet and Dry Commissioning	55

The following table indicates the summarized activities by summer season. The summarized activities dates are shown in Months after receipt of order and Months after landing.

Table 7.4 Summary of Key Road and Camp construction Activities

Season	Name	Months ARO (After receipt of order)	Months after landing
First summer (5 months duration)	Landing establish pre camp	6	
	Start road construction to Camp 1		1
	Start Camp 1 to Intake area Camp 2		1
	Bridge		2
Second summer (5 months duration)	Installation of main camp modules	1	
	Complete Road Camp 1 to Intake area Camp 2	16	
	Start Camp 2 to Lake Imarsuaq Camp 3	19	
	Installation of camp modules	17	

Season	Name	Months ARO (After receipt of order)	Months after landing
Third (5 months duration)	Complete Road Camp 2 to Lake Imarsuaq Camp 3	28	
	Bridge	21	
	Establish water transport from Camp 3 to Camp 4	31	
	Installation of camp 3 modules	29	
	Installation of camp 4 modules	30	

8 Project cost estimation

8.1 Introduction

This present section describes the organization, assumptions and results of the project financial cost estimate of 6g. The detailed cost estimate is included in Appendix 1 while the high level summary is included on the following page. Highlights are the following:

Total direct cost:	\$191.2M
Total indirect cost:	\$498.5M
Total project cost:	\$689.7M

The cost estimate has been prepared using the Alcoa Project WBS with high level activities being the following:

Direct costs

3100	Harbor site preparation
3200	Port facility
3300	Primary road construction
3400	Civil works related to powerhouse, tailrace, tunnels and surge tunnel
3500	Civil works related to power tunnel
3600	Dams and spillways
3700	Electrical works
3800	Mechanical and electrical works
3900	Architectural works

Indirect costs

6100	Temporary construction facilities
6200	Construction services
6300	Construction equipment, tools and supplies
6400	Material transportation
6500	Construction camps
6600	Insurance, taxes, permits and fees
6700	Miscellaneous freight
7000	EPCM home office
8000	EPCM field office
9000	Contingency

For each of these high level items, the following details are provided:

- Man hours;
- Man power cost;
- Consumable material cost;
- Permanent material cost;
- Equipment operation cost;
- Fuel cost; and
- Total cost

The estimate in Appendix 1 includes a breakdown of the high level activities and a further breakdown per second level WBS element.

The estimates provided are all in US dollars. The exchange rates used for other currency conversions are the following:

- Canadian to US dollar = 0.9
- Euro to the US dollar = 0.65675

8.2 Cost estimate methodology

The cost estimate uses the contractor methodology which takes into consideration construction methods, with previously witnessed productivities (man hour requirements), adjusted to the particular conditions of this project, being the remoteness, permafrost and temperature, summer and winter daylight conditions and the wind, snow and rain statistics of the area and of course, transportation logistics particularities (sea, air and boat).

8.3 Estimate quantities and unit rates

8.3.1 Direct costs

The estimate quantities for the direct construction elements were provided by the designer teams, based on preliminary drawings, but which have included design optimizations. Permanent equipment prices were based on budgetary quotes received from suppliers and material costs based on present world prices. It should be noted that transportation costs related to these last two items are considered indirect costs and discussed below.

All construction equipment operation and maintenance costs are also considered direct costs. The largest quantity commodity is fuel with a unit price at \$0.72/liter, which is the present (August 2009) price in Greenland.

Other major commodity unit prices are the following:

- Cement: \$73/mt (from North America)
- Reinforcing steel: \$689/mt (from North America)
- Bitumen: \$625/mt (from North America)

With approximately 1.6 million direct man hours, man power average base labour rate plays a big role in the overall cost estimate; it is considered to be \$24/hour (including premiums, overtime and overhead). This rate was calculated using the following assumptions:

- Base labour hourly rate: \$14.70 (in Greenland)
- Shift work hourly premium: \$ 1.20
- Overtime premium (%): 50%
- Overhead (%): 26.36% (based on Greenland laws)
- Project work week: 60 hours

8.3.2 Indirect costs

The following section describes details concerning project indirects. Although man hours are required throughout most of the indirect budget elements, it is worth noting that the total indirect man hour count is approximately 0.5 million, with the same all inclusive hourly rate of \$24/hour, as described above.

Total indirect costs amount to approximately **\$500M**, with one third of this total amount required for construction, maintenance, catering and operations of the camps.

Below is a brief description of what has been foreseen in each of the principal indirect cost codes and pertinent cost information.

8.3.2.1 Temporary construction facilities (6100)

The estimate preparation for temporary construction facilities involved determining work site requirements. They are as follows:

- Buildings at all four construction sites (including an office, garage, trade shop, warehouse, dry house, washroom and foreman office), sized to the peak requirement, including site preparation, installation and dismantling;
- Concrete batch plant for each site (installation and dismantling)
- Crushing plant (transportable from site to site)
- Asphalt batch plant (at 2 sites)
- Explosive depots (at the main camp, sites 2, 3 and 4 and road sites)

The above facilities represent **\$5.6M** in project cost.

The above facilities require the support of roads and walkways as well as all utilities totaling **\$11.1M**.

8.3.2.2 Construction services (6200)

Construction services include all construction site operational requirements (excluding any camp requirements – which are included in 6500). This includes the following:

- Building maintenance;
- Operational vehicles such as fuel trucks, mechanic's and welding trucks, light vehicles (pick ups, SUVs and ambulance), for all four sites;
- Shop operations;
- Road maintenance;
- Communication (radio and cellular);
- Operation of the water route;
- Final site cleanup;
- Material handling and warehousing (including equipment and man hours, especially fuel depots);
- NDE and QA/QC testing;
- Surveying (excluding man hours);
- Site security;
- Man power transportation (point of origin to appropriate camp); and
- General expenses.

The above site operational costs represent **\$48.6M**, of which warehousing and material handling accounts for **\$10.3M** and manpower transportation **\$25.8M**.

Small tools and supplies have also been included under this budget cost code, not having been included with construction equipment. It should be noted that small tool cost has been evaluated at \$0.30/man hour and supplies at \$0.10/man hour.

8.3.2.3 Construction equipment, tools and supplies (6300)

The list of construction equipment is included in Appendix 1, describing the type and quantity required for the various construction sites. The equipment cost attributed includes only the depreciated value.

Construction equipment represents a budget of **\$58.8M**. The tunnel boring machines account for **\$11.1M** of this total, the asphalt batch plant accounts for **\$3M** and the crusher, **\$3.6M**.

All equipment transportation costs (mobilization and demobilization) have been included in "Miscellaneous freight (6700)".

8.3.2.4 Material transportation (6400)

Material transportation includes freight and insurance costs of all construction bulk materials. This cost element accounts for **\$16.6M** of the total project budget.

8.3.2.5 Construction camps (6500)

Almost one third of the indirect cost are attributed to the construction and operation of the construction camps. Site 6g includes 4 camp sites, accommodating a total of 950 workers and staff (including Transmission Line construction requirements). Camp capacities were established using peak requirements at each of the work sites.

Construction and removal of the camps amount to **\$117.4M**, while operation amounts to **\$35M**, representing approximately \$80 per man day, half of this amount to cover food, catering and camp maintenance and the other half for utility requirements (principally fuel for generators).

This project cost item is worth a total of **\$152.4M**.

8.3.2.6 Insurance, taxes, permits and fees (6600)

Project insurance has been evaluated at **\$24.9M**, including responsibility insurance, calculated at 2.02% of \$700M and risk, calculated at 0.62% of the project value. The project Execution bond was calculated at \$0.0069/\$ value.

Equipment insurance was calculated at 0.5% of the total equipment value, evaluated at **\$120M**.

It should be noted that it was assumed that the project would be exempt of taxes, duties and port fees.

8.3.2.7 Miscellaneous freight (6700)

Miscellaneous freight includes all mobilization and demobilization costs for project equipment, camp modules and camps utility equipment. In addition, it includes all inbound costs for permanent equipment such as turbine/generators, electro-mechanical and mechanical components.

Equipment freight, including insurance, was evaluated at **\$19.3M** and camp module and utility equipment freight evaluated at **\$26.9M**, for a total of **\$46.2M**.

8.3.2.8 EPCM – home and field offices (7000 and 8000)

EPCM home office costs have been evaluated by considering the following:

- Contractor home office services are evaluated at 2% of the direct construction costs of \$191M, yielding **\$3.8M**;
- Contractor home office services for indirects have not been considered;
- General EPCM project management has been evaluated at **\$2M**;
- Total EPCM home office estimate is **\$5.8M**.

It should be noted that the following other EPCM costs are not considered:

- Engineering FEL2 and FEL3 activities;
- Procurement activities for purchase and contracts;
- Detailed engineering activities during the construction phase; and
- Contractor 10% profit.

EPCM site office costs have been evaluated by estimating the contractor and general management man month requirements. The contractor requirement include site supervisory staff (superintendents) and all higher grades.

Contractor requirements were estimated at approximately 3 000 man months and general management requirements were estimated at 1 600 man months. A common monthly rate of \$10 000 was used, yielding a total EPCM site office cost of **\$45.8M**.

8.3.2.9 Contingency (9000)

Project contingency has been established by analyzing each project component, as specified in the WBS, and assigned a specific contingency which reflects the confidence level. It should be noted that an average of 15% has been established for all directs and indirects, while 10% was figured for EPCM items.

The total project contingency has been set at **\$85.1M**, subdivided in the following way:

- Direct cost contingency: **\$33.1M** (17% of total direct value)
- Indirect cost contingency: **\$44.2M** (12% of total indirect value)
- EPCM contingency: **\$5.0M** (10%of EPCM value)

Transmission line contingencies are included in the overall cost.

8.4 Total project cost including Transmission Line Project

WBS	Description	Site 7e	Site 6g	Total Hydro sites	Men-hours (Both sites)
2100	Harbor site preparation	474 981 \$	474 981 \$	949 962 \$	4 460
2200	Port Facility	4 050 016 \$	5 233 722 \$	9 283 738 \$	2 594
2300	Primary roads construction	45 875 129 \$	32 358 790 \$	78 233 919 \$	431 987
2400	Civil works related to Powerhouse, Tailrace tunnel and Surge tunnel	42 329 062 \$	23 233 879 \$	65 562 941 \$	867 656
2500	Civil works related to Power	130 717 844 \$	25 988 319 \$	156 706 163 \$	1 635 677

WBS	Description	Site 7e	Site 6g	Total Hydro sites	Men-hours (Both sites)
	tunnel				
2600	Dams and Spillway	32 288 698 \$	27 603 038 \$	59 891 736 \$	804 730
2700	Electrical Works	35 132 187 \$	26 691 494 \$	61 823 681 \$	344 493
2800	Mechanical + Electrical Works	120 575 844 \$	44 085 105 \$	164 660 949 \$	791 400
2900	Architectural works	5 497 800 \$	5 497 800 \$	10 995 600 \$	0
	<i>Directs costs - Sub-Total</i>	<i>416 941 561 \$</i>	<i>191 167 128 \$</i>	<i>608 108 689 \$</i>	<i>4 882 996</i>
6100	Temporary Construction Facilities	8 595 590 \$	16 725 950 \$	25 321 540 \$	81 403
6200	Construction Services	65 047 197 \$	48 642 636 \$	113 689 833 \$	645 277
6300	Construction Equipment, Tools & Supplies	74 738 361 \$	58 817 800 \$	133 556 161 \$	0
6400	Material Transportation	25 105 518 \$	16 568 461 \$	41 673 979 \$	0
6500	Construction Camp	107 729 334 \$	148 628 635 \$	256 357 969 \$	148 666
6600	Insurance, Taxes, Permits, Fees	25 871 461 \$	24 908 494 \$	50 779 955 \$	0
6700	Miscellaneous	34 785 789 \$	46 247 259 \$	81 033 048 \$	0
7000	EPCM Home Office	12 338 831 \$	5 823 343 \$	18 162 174 \$	0
8000	EPCM Field Office	54 170 000 \$	45 840 000 \$	100 010 000 \$	0
9000	Contingency	120 933 738 \$	79 884 241 \$	200 817 979 \$	0
	<i>Indirects costs - Sub-Total</i>	<i>529 315 819 \$</i>	<i>492 086 819 \$</i>	<i>1 021 402 638 \$</i>	<i>875 346</i>
	<i>Miscellaneous non accounted hours</i>				<i>750 000</i>
	<i>Total Hydro Costs</i>	<i>946 257 380 \$</i>	<i>683 253 947 \$</i>	<i>1 629 511 327 \$</i>	<i>6 508 342</i>
	Transmission line (by Efla)	93 900 000 \$	121 000 000 \$	214 900 000 \$	
	Substation (by Efla)	21 600 000 \$	18 400 000 \$	40 000 000 \$	
	T-line contingencies	11 500 000 \$	13 900 000 \$	25 400 000 \$	
	<i>Total Costs</i>	<i>1 073 257 380 \$</i>	<i>836 553 947 \$</i>	<i>1 909 811 327 \$</i>	
	With N-1 transmission line (by Efla)	64 000 000 \$	76 100 000 \$	140 000 000 \$	
	With N-1 substation (by Efla)	2 700 000 \$	3 300 000 \$	6 000 000 \$	
	N-1 T-line contingencies	6 700 000 \$	7 900 000 \$	14 600 000 \$	
	<i>Total Costs (with N-1 transmission line)</i>	<i>1 146 657 380 \$</i>	<i>923 853 947 \$</i>	<i>2 070 411 327 \$</i>	

9 Procurement

The tendering and construction strategy that will be put into place has to meet the project requirements for the construction of the large scale infrastructures that are planned in Greenland. Other constraints include the short time requirement, the remoteness of the proposed construction sites and the cold conditions encountered at the construction sites.

The proposed formula is to follow an accelerated regime in which construction is divided into several lots. Construction of the preliminary facilities such as the harbor and the roads is started as soon as the design and tendering has been done for this part of the project. It will allow the construction of the further works to start earlier as the various accesses will have already been put in place. However, construction of the various lots shouldn't be started prior to the end of the final design of that particular lot. Such a procedure, even if it can reduced the overall construction time, is not recommended since it implies high risk regarding cost control and quality of the design.

9.1 Tendering process

Time appears to be of the essence; also as a long duration development period increases the accrued interest during construction as well as overall general expenses and other financial cost. Accordingly, a practical development approach is essential. The safest alternative considered has the following steps:

1. Pre-bidding the harbor and road construction,
2. Accelerating essential field exploration; aimed at larger risk components,
3. Selecting an engineer for the tender design, without bidding process, who masters valued engineering based on construction driven approach,
4. Giving the selected engineer a maximum of 8 months to develop a proper tender design and overall specification package,
5. Adopting a transparent risk sharing procedure in the tender documents, with bonuses and not only penalties.
6. Encouraging the contractor (bidders) to offer alternative solutions, also during construction, with pre-defined shearing of cost-savings,
7. Selecting highly qualified experts to review design (a person who understands and masters valued construction driven design) as well as supporting construction (construction management expert(s)).

For the design phase, work should be carried out by a designated small project design team, located at one common project office, preferably away from the head office. Past experience has shown that long design period, with project staff located at multiple offices, results in "end-rush work", poorer design, delays and higher overall development cost. The smaller and the higher caliber the design team is, the lower will the overall project development cost be.

9.2 Division of construction contracts

The following sub-divisions of the construction lots appears beneficial, both to accelerate the project completion (local smaller contractors are likely to be able to mobilize earlier than large joint-venture), as well as to lower cost. It is likely that local smaller contractors

are more likely to be able to mobilize earlier than large joint-ventures. It is assumed that this approach will be supported by a small high quality project-design team following the above addressed traditional tendering approach.

1. Pre-bid construction of the harbor and of the preliminary camp.
2. Pre-bid construction of all temporary roads and eventually the “first phase” construction road to the upper project area. The downside of this through might be that the “Dam-contractor” and the Power-Intake contractor might claim if the road to those locations is not finished “on time”. Pre-bidding this work, with clear intent of possible combination of all lots, would be useful.
3. Eventually, pre-bid the excavation of the access tunnel to the power cavern, also used for setting up and excavating the TBM driven tunnels.
4. Bid separately upper-project facilities other than the power intake; including construction of the dams, the canals, the diversion tunnels and other upper project auxiliary structures.
5. Bid separately the construction of the:
 - Headrace tunnel and shaft excavation, stabilization and lining, including cleaning and testing. In that bid package, make it clear that the bidders stand free to excavate the waterways with a tunnel boring machine (TBM) or by drill an blast, and they can increase or decrease the tunnel slope and introduce shafts at will, including ventilation shafts, as long as the head loss in the tunnel will not increase beyond prescribed value (and as long as this will not increase tendered cost).
 - The power and transformer caverns, tailrace tunnel and all waterways and power generating and appurtenant structures, including the power intake and tunnel closure plugs, switchyard and operation building, etc.
 - Include in this package the bifurcation and the penstock steel liner (specialized sub for this task). The reason for not separating this task from the civil works is to reduce claims.
6. Bid electro-mechanical equipment as the 6th lot. The various suppliers will likely joint-venture.
7. Bid construction of the transmission lines separately, as the 7th lot.

While some of those lots would be tendered at a different time, allowing the Engineer and the Owner to start up certain critical parts of the project faster than if all would be bid at the same time, the setup should allow any Joint Venture to bid on one, more or all of the lots. The later would encourage large international consortiums to collaborate with smaller “local” contractors, who could mobilize faster for the pre-construction works and who are familiar working under arctic conditions.

10 Risk analysis

The five main risks that were identified for the hydro are:

- Greater than anticipated infrastructure and logistics difficulties could increase costs and delays project start up ;
- Civil works construction difficulties could increase costs and delays (access road, tunnelling, dam construction);
- Unfavorable weather conditions (change in duration of either winters or summers - movement of materials is easier during winter conditions -Fjord ice, fog, movement over snow or ice whereas construction is easier during summer conditions.);
- Difficulties could be encountered along the 300 km transmission lines to be constructed in rough terrain, with long fjord and glacier crossings. Some of them are state-of-the-art
- Environmental issues increase project cost, potentially impact start and completion dates/schedules and reduce available power output (NGO delays, Water releases downstream of dams, Ecosystems or archeological features in flooded areas or T-line corridor, project footprint).

11 Project optimization

Optimization of the project layout was carried out to obtain the largest firm yield possible at the lowest per MW cost, while meeting the project requirements regarding the smelter energy needs. Additional optimization will need to be done in the next project phase to increase the reliability and the revenues of the project.

11.1 Powerhouse location

Following the 2009 investigations and more precisely the results obtained from the hydraulic jacking tests performed in a borehole drilled at the powerhouse location (penstock levels), it was concluded that the minimum stress levels in the rock formations are rather low (4.8-5 MPa at elevation 41 m), compared to the Norwegian recommendations for the design of underground hydroelectric works. It was assumed that these lower stresses are result of stress relief near the steep, free-standing fjord. Higher stresses are likely to be found at greater depth and further back from the fjords. Therefore, in accordance with the topography, it was decided to move the powerhouse complex approximately 500 m away from the fjord. At this location the rock cover is 555 m. The new location reduces the length of the headrace tunnel, while the tailrace tunnel is longer, thus balancing the costs. However, the length of the access tunnels is longer increasing the construction costs.

Figure 11.1 shows the new location of the powerhouse complex.

11.2 Potential savings

For the next project phase, additional activities and investigations could allow to reduce the overall project cost and optimize the power production at site 6g. The main items that are targeted for potential savings are outlined below.

11.2.1 List of items

Headrace tunnel diameter

Optimization of the headrace tunnel should be carried in more details for different actualization rates. Also, since the power plant is not connected to a public network, a smaller tunnel would allow cost reductions without reducing the revenues. The firm power available at the site would however be reduced due to an increase in head losses. However, the firm power could be balanced by raising the dams to increase the useful volume of the reservoir. A more detail analysis on the subject should be carried out.

So far, we have considered a conservative overall potential economy of 4.5 M\$ with a reduction in the headrace tunnel diameter.

Penstocks and manifold optimization

The penstocks, manifold and the lined section of the headrace tunnel leading to the turbines can be optimized with further economical analysis. Both the diameter of the excavated cross-section and the concrete and steel thickness should be reviewed in the next project phase to reduce the costs.

Dam axis and cross-section

Following a better knowledge of the overburden foundation conditions, the requirements for the complete excavation of the overburden underneath Dam 3 maybe re-assessed and another type of dam can be considered.

Despite Dam 3 isolation and the high mobilization cost of an asphalt plant compared to its relatively small asphaltic concrete core volume that would be required, it may still be interesting to keep an asphaltic concrete core rockfill dam (ACRD) type for this dam too.

A concrete faced rockfill dam (CFRD) is a type of dam that should be studied in more details in the future. This type of dam is less weather sensitive than ACRD and it can be constructed faster. Despite, CFRD is somewhat more labour intensive and implies the exclusion of the upstream cofferdam from the dam body, it may be a more interesting dam type especially if there in case of construction schedule concerns.

If ACRD is maintained, optimization of the typical cross section maybe conducted. For instance, the reduction of the width of filter and transition zones maybe lead to material cost savings. However, the related more restrictive material placement conditions should be carefully studied.

More detailed economical analysis should be later conducted on the different dam types to determine the optimal solution regarding the in-situ conditions and the scheduling constraints.

Rock support

The rock support assumptions can be reduced compared to the estimate presented in the report, thus reducing the cost of each proposed tunnel (headrace, tailrace, access and diversion tunnels).

Road construction

The construction methodology and the initial cost estimate for the road construction were prepared without a site visit. A site visit made by an experience road contractor would allow optimizing the proposed methodology, thus refining the cost and likely reducing the overall cost since the level of contingency could be reduced. Actual contingencies on the road construction estimate account for 25% of the cost. The airlift is also a major item in road construction and a site visit would potentially allow reducing this cost.

Diversion tunnel

In the next study phase, the excavation volume of the diversion tunnels with regard to the cofferdam heights can be optimized to reduce costs.

Concrete plugs

The concrete plugs are designed with a thickness of 5 m. This thickness could be reduced following a more detail structural analysis to approximately 2 or 3 m.

Cable tunnel

A new concept in the ventilation of the cable tunnels would allow eliminating the concrete blocks that are planned to split the tunnel cross-section in two sections. The cross-section of the cable tunnel could then be kept at a minimum size.

Construction camps

In the initial cost estimate, the full purchase prices of all camp buildings and facilities are considered. However, those items will have a remaining value following the end of the project. A depreciation of 60% of the initial cost was applied to the initial purchase price to determine the remaining values that represent savings that can be applied to the overall cost of the project.

Temporary construction facilities

As for the construction camps buildings, no remaining value was applied to the temporary construction facilities. Some items like pipes, generators and other can be reuse by the contractors. Therefore, a remaining value of 40% of the initial purchase price is also applied on those items, representing a cost saving for the project.

11.2.2 Uncertainties in purchasing costs

The main items of the projects are the fuel cost, equipment cost and the man power. The estimation of all of those items relies on unit costs. A change in unit cost for one of those items could largely influence both positively or negatively the overall project cost. Therefore, it is important to validate those unit costs in the next study phase. The potential savings currently considered from those items are detailed below.

In the initial cost estimate, a cost of 0.72 US\$/L was used for the fuel, which correspond to the price of crude oil in Greenland. However, it would be possible to purchase the fuel from another country and transport it to Greenland, at a lower cost per liter. Fluctuations in the price of the crude oil barrel over time can lead to important cost variations as the cost of fuel is one of the major items of the project. At the moment, it is proposed to reduce the price per liter to 0.66 US\$ which corresponds to the international August 2009 price.

Also, the purchasing costs of Caterpillar equipments were initially given by Denmark suppliers, which correspond to the costs considered in the cost estimate. A cost reduction is anticipated if equipments were to be purchased in the United States and transported to Greenland.

Finally, a cost reduction in the transportation of the labor to and from Greenland is also possible with chartered flights. Indeed, the initial cost estimate considered the actual market price for a single flight ticket to or from Greenland with the available airlines servicing Greenland.

The cost reductions were weighted on both site 7e and 6g to consider the amount of fuel, equipment and labor used for each site.

11.2.3 Working conditions

Additional savings are possible for the project, depending on the working conditions that are assumed, and the contingencies that are applied to the project. Alcoa suggested various criteria to consider in the cost estimate that are different from the parameters used in the base cost estimate, which roughly represent the actual practices in Canada. It is possible that the working conditions could be below the western countries standards if workers from other countries are employed for the project.

The criteria considered in the base cost estimate concerning the workers conditions compared with the new criteria proposed by Alcoa are the followings:

Tableau 11.1 Working conditions

	Initial cost estimate criteria	Revised criteria proposed by Alcoa
Hourly rate	24\$/hr	10\$/hr
Workers shift	40 days of work	120 days
Staff shift	40 days of work	60 days of work

Applying the new hourly rate to the cost estimate yields important cost savings on all project items. As for the longer work shifts, it reduced the cost of man power transportation to and from Greenland, as well as the number of overall trips.

The potential savings that can be obtained from the above considerations are:

New hourly rate of 10\$/hr: Alcoa suggested the use of a 10\$/h rate for Chinese labor. This change represents approximately 25 M\$ at site 6g, considering a productivity reduction of 25%

Reduced man-power transportation due to longer working shifts: approximately 15 M\$ cost reduction at site 6g

11.2.4 Summary table of potential savings

The following table summarizes the items for which optimization or elimination could reduce the cost of the projected site 6g, except for the potential savings related with a reduction of the hourly labor rate from \$24/hr to \$10/hr which are presented in Table 11.3. The cost reductions (or increase as for the powerhouse location) anticipated were estimated for every component previously discussed. Similar efforts were conducted by the transmission line design team to obtain a potential saving of 5 M\$ for the site 6g transmission line. The potentials savings are presented in Table 11.2.

Table 11.2 Summary of potential savings at site 6g

Item	Potential savings (M\$)
Powerhouse location	8.5 increase
Headrace tunnel diameter	4.5
Penstocks and manifold	1.2
Rock support requirements	1.5
Road construction methodology	2.5
Diversion tunnel	3.5
Concrete plug design	0.9
Cable tunnel	1.6
Construction camp	34.8
Temporary construction facilities	1.1
Fuel cost	3.4
Construction equipment cost	6
Labor transportation cost	16.4
Transmission line	5.0
<i>Total</i>	<i>74</i>

The cost savings outlined above were distributed in the project cost summary table to match the original subdivisions of the project. The fuel and construction equipment cost reductions do apply to all items of the projects as they are used throughout. As for the potential savings in the labor transportation cost, they include a 1.5 M\$ saving using chartered flights and 14.9 M\$ with longer working shifts as explained in section 11.2.3.

Potential savings were determined in part with a deeper analysis of some uncertainties that exist in the project. Therefore, the overall contingencies of the project are reduced to 10 % of the total amount of direct and indirect costs.

Table 11.3 in the next page shows the overall cost summary of the site 6g project with the potential savings. Potential savings were determined in part with a deeper analysis of some uncertainties that exist in the project. Therefore, the overall contingencies of the project are reduced to 10% of the total amount of direct and indirect costs.

Table 11.3 Cost impact of workers hourly rate

Pos.	Item	\$24/hour (initial cost estimate)	\$10/hour (potential alternative)
<i>1. Civil works</i>			
1.1	Dams	25.8	24.3
1.2	Tunnels	58.9	55.3
1.3	Canals and intake	11.4	10.8
1.4	Underground power station	39.6	37.9
<i>2. Mechanical and electrical equipment</i>		92.0	87.2
<i>3. Infrastructure</i>			
3.1	Harbors and roads	55.3	52.3
3.2	Construction Camps	114.6	106.8
3.3	Construction material transportation	16.2	15.4
<i>Direct costs total</i>		413.9	390.2
<i>4. Indirect costs</i>			
4.1	Construction services and temporary facilities	38.5	36.5
4.2	Travel cost	9.4	9.4
4.3	Insurance	24.9	24.9
4.4	EPCM	51.7	51.7
<i>Indirect costs total</i>		124.4	122.4
<i>5. Transmission line</i>		134.4	134.4
<i>Sub-total</i>		672.7	647.0
<i>Total (with contingency)</i>		740	712
Hydro Plant Output (MW)		185	185
<i>M\$/MW</i>		4.00	3.85
<i>N-1 Transmission line (added cost)</i>		79.3	79.3
<i>Total (with contingency)</i>		827	799
<i>M\$/MW</i>		4.47	4.32

11.3 Additional opportunities

Site 6g is currently design to be operated with two reservoirs in series, the Big and Lower Lake. The inflows will come from the natural watershed of those two lakes along with sub-basin (E) located to the north of the Big Lake, that will be diverted into it.

Two other tunnels could be implemented at site 6g to increase the inflows to the project reservoirs and the firm power available.

First, Tunnel 2 (discussed briefly in section 4) is an equilibrium tunnel that would allow to increase the storage volume of the Big Lake by using the useful volume of the adjacent Middle Lake between elevation 669 and 680 m. Middle Lake is adjacent to the Big Lake but the low point between the two lakes is at elevation 680 m. The estimated construction cost for this tunnel is approximately 5 M\$, including the required access road from Tunnel 1. According to the simulations that were ran, this tunnel would allow gaining approximately 3 MW of additional firm power. If it was decided to construct this tunnel,

careful attention should be given to the risk of freezing due to potential stagnant water inside the tunnel for a long period of time. As for the required dimensions, constructability issues would govern the sizing of the tunnel since it acts as an equilibrium tunnel only.

Secondly, Tunnel 3 (discussed briefly in section 4) could be constructed a few kilometers to the east of the Lower Lake. This tunnel would allow diverting an additional catchment (sub-basin A) into the project system to increase the inflows, and by the same way the firm power. Although the inflows are relatively low from this sub-basin (average of 1.4 m³/s), it would still allow to increase the firm power by an additional 4 MW. The estimated cost to implement this tunnel, including a 12 km access road to the site is of 12 M\$. The main constraint related to the design of this tunnel is the fact that it would be a tunnel that would only flow for a period of approximately 4 months per year. The tunnel should then be designed to avoid the presence of stagnant water and ice blocks inside the tunnel.

There is also a possible future opportunity regarding the potential increase in run-off, mainly from the glaciers due the forecasted global warming. 2040 projected discharge series were computed by Vatnaskil but were not used to determine the firm power available at site 6g, since those series consider a steady increase in temperature over the next 40 years. However, there exist a potential that such predictions could occur in the future as global warming is forecasted to continue, along with fast glacier melting. It is then possible that the power production at site 6g could be increased with a future rise in inflows. This additional opportunity should be studied in more depth, either to increase the installed capacity at the site or leave space for a future increase in hydro equipments. Indeed, the projected 2040 series were used in the flood determination to reduce the risk regarding dam and structure safety. The opportunity cost of increasing the power capacity at site 6g would then only include the costs related to the conveyance structures and production devices.

Appendix 1

Cost estimate

Greenland - 6g Cost Estimate

Project Summary

WBS	Description	Man Power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption	TOTAL PRICE	Men-hours
3100	Harbor site preparation	53,520 \$	364,785 \$	- \$	34,022 \$	22,654 \$	474,981 \$	2,230
3200	Port Facility	31,128 \$	992,500 \$	4,181,360 \$	23,562 \$	5,172 \$	5,233,722 \$	1,297
3300	Primary roads construction	4,315,032 \$	5,222,000 \$	- \$	5,704,611 \$	12,393 \$	32,358,790 \$	195,598
3400	Civil works related to Powerhouse, Tailrace tunnel and Surge tunnel	7,359,144 \$	5,206,613 \$	6,170,052 \$	2,576,190 \$	1,921,880 \$	23,233,879 \$	307,693
3500	Civil works related to Power tunnel	7,715,074 \$	7,270,316 \$	2,334,784 \$	3,170,091 \$	5,498,054 \$	25,988,319 \$	321,469
3600	Dams and Spillway	9,120,154 \$	6,686,875 \$	3,201,243 \$	4,531,696 \$	4,063,070 \$	27,603,038 \$	394,692
3700	Electrical Works	3,418,498 \$	841,873 \$	20,435,654 \$	1,644,355 \$	351,114 \$	26,691,494 \$	142,490
3800	Mechanical + Electrical Works	7,894,250 \$	799,549 \$	34,632,371 \$	758,935 \$	- \$	44,085,105 \$	225,550
3900	Architectural works	- \$	- \$	5,497,800 \$	- \$	- \$	5,497,800 \$	0
<i>Directs costs - Sub-Total</i>		<i>39,906,800 \$</i>	<i>27,384,511 \$</i>	<i>76,453,264 \$</i>	<i>18,443,462 \$</i>	<i>11,874,337 \$</i>	<i>191,167,128 \$</i>	<i>1,591,019</i>
6100	Temporary Construction Facilities	1,290,876 \$	6,148,065 \$	- \$	1,773,342 \$	7,515,347 \$	16,725,950 \$	53,706
6200	Construction Services	7,628,510 \$	6,196,427 \$	- \$	3,051,636 \$	3,998,382 \$	48,642,636 \$	317,855
6300	Construction Equipment, Tools & Supplies	- \$	- \$	- \$	- \$	- \$	58,817,800 \$	0
6400	Material Transportation	- \$	- \$	16,500,000 \$	- \$	- \$	16,568,461 \$	0
6500	Construction Camp	1,975,440 \$	34,221,083 \$	92,763,189 \$	2,576,283 \$	17,092,640 \$	148,628,635 \$	84,819
6600	Insurance, Taxes, Permits, Fees	- \$	- \$	- \$	- \$	- \$	24,908,494 \$	0
6700	Miscellaneous freight	- \$	- \$	- \$	- \$	- \$	46,247,259 \$	0
7000	EPCM Home Office	- \$	- \$	- \$	- \$	- \$	5,823,343 \$	0
8000	EPCM Field Office	- \$	- \$	- \$	- \$	- \$	45,840,000 \$	0
9000	Contingency	- \$	- \$	- \$	- \$	- \$	79,884,241 \$	0
<i>Indirects costs - Sub-Total</i>		<i>10,894,826 \$</i>	<i>46,565,575 \$</i>	<i>109,263,189 \$</i>	<i>7,401,261 \$</i>	<i>28,606,369 \$</i>	<i>492,086,819 \$</i>	<i>456,380</i>
<i>Miscellaneous non-accounted hours</i>								<i>250,000</i>
Total Costs		50,801,626 \$	73,950,086 \$	185,716,453 \$	25,844,723 \$	40,480,706 \$	683,253,947 \$	2,297,399

WBS	Description	Quantity	Un.	Man Power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption	TOTAL PRICE	Unit price	Men-hours	
3100	Harbor site preparation											
3,110	Site development	48,000	m²	20,160 \$	100,000 \$	- \$	16,286 \$	10,614 \$	147,060 \$	3.06	840	
3,120	Fences and Gates			13,200 \$	64,785 \$	- \$	1,450 \$	1,426 \$	80,861 \$		550	
3,130	Exterior Lighting			Included in Helicopter Pad								0
3,140	Helicopter pad			20,160 \$	200,000 \$	- \$	16,286 \$	10,614 \$	247,060 \$		840	
3200	Port Facility											
3210	Wharf			- \$	- \$	3,931,360 \$	- \$	- \$	3,931,360 \$		0	
3220	General Material Receiving and Handling			- \$	- \$	250,000 \$	- \$	- \$	250,000 \$		0	
3230	Storage warehouse			10,968 \$	68,500 \$	- \$	15,000 \$	- \$	94,468 \$		457	
3240	Workshop and Miscellaneous			Included in Architectural Works								0
3250	Office for custom authorities			Included in Architectural Works								0
3260	Fuel depot			20,160 \$	924,000 \$	- \$	8,562 \$	5,172 \$	957,894 \$		840	
3300	Primary roads construction											
3310	Cross section F1	4,967	m	540,294 \$	- \$	- \$	- \$	- \$	2,727,126 \$	549	24,559	
3320	Cross section F2	7,750	m	898,036 \$	- \$	- \$	- \$	- \$	4,540,594 \$	586	40,820	
3330	Cross section F3	17,690	m	1,032,847 \$	- \$	- \$	- \$	- \$	5,143,308 \$	291	46,948	
3340	Cross section C1	6,870	m	383,223 \$	- \$	- \$	- \$	- \$	1,940,321 \$	282	17,419	
3350	Cross section C2	5,884	m	385,631 \$	- \$	- \$	- \$	- \$	1,985,959 \$	338	17,529	
3360	Cross section C3	3,420	m	932,441 \$	- \$	- \$	- \$	- \$	4,939,918 \$	1,444	42,384	
3370	Bridges	2	un	- \$	5,222,000 \$	- \$	- \$	- \$	5,222,000 \$		0	
3380	Air Lift			142,560 \$	- \$	- \$	5,704,611 \$	12,393 \$	5,859,564 \$		5,940	
3400	Civil works related to Powerhouse, Tailrace tunnel and Surge tunnel											
3410	Excavation											
3411	Powerhouse and Access	113,400	m²	1,421,796 \$	1,918,245 \$	515,387 \$	487,537 \$	362,764 \$	4,705,729 \$	41	59,242	
3412	Transformer Chamber and Access	21,750	m²	236,179 \$	314,039 \$	87,608 \$	82,760 \$	64,412 \$	784,998 \$	36	9,841	
3413	Powerhouse tailrace including Access and Outlet	65,657	m²	1,474,770 \$	1,476,471 \$	447,337 \$	448,741 \$	553,891 \$	4,401,210 \$	67	61,486	
3414	Cable and Escape Tunnel	18,425	m²	1,525,863 \$	480,011 \$	311,523 \$	572,639 \$	300,161 \$	3,190,197 \$	173	63,578	
3420	Concrete Works											
3421	Transformer Chamber Concrete	1,660	m³	371,912 \$	174,081 \$	361,425 \$	152,985 \$	92,075 \$	1,152,478 \$	694	15,501	
3422	Powerhouse - Phase I	1,755	m³	420,379 \$	184,046 \$	382,126 \$	163,283 \$	98,490 \$	1,248,324 \$	711	17,520	
3423	Powerhouse - Phase II	1,240	m³	314,828 \$	130,042 \$	270,034 \$	114,218 \$	68,735 \$	897,857 \$	724	13,121	
3424	Penstocs and Manifold	650	m³	118,861 \$	53,836 \$	102,995 \$	51,700 \$	32,948 \$	360,340 \$	554	4,954	
3425	Intake Tunnel	1,800	m³	474,402 \$	163,567 \$	391,940 \$	167,503 \$	101,034 \$	1,298,446 \$	721	19,771	

WBS	Description	Quantity	Un.	Man Power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption	TOTAL PRICE	Unit price	Men-hours
3426	Cable and Escape Tunnel			278,907 \$	23,399 \$	528,139 \$	45,953 \$	32,302 \$	908,700 \$		12,620
3430	Powerhouse crane installation	1	ls	21,600 \$	900 \$	- \$	6,094 \$	3,920 \$	32,514 \$		900
3440	Powerhouse overhead roofing	45	mt	8,100 \$	225 \$	180,000 \$	3,033 \$	1,881 \$	193,239 \$	4,294	338
3450	Structural Steel	160		38,400 \$	800 \$	604,000 \$	10,717 \$	6,653 \$	660,570 \$	4,129	1,600
3460	Steel lining - Penstocks and Manifold	460	mt	154,560 \$	5,520 \$	1,380,000 \$	35,321 \$	30,404 \$	1,605,805 \$	3,491	6,440
3470	Concrete Plugs - Tunnels	2,790	m³	498,587 \$	281,431 \$	607,538 \$	233,706 \$	172,210 \$	1,793,472 \$	643	20,782
3500	Civil works related to Power tunnel										
3510	Power tunnel (including Rock Support)	224,138	m³	6,272,315 \$	5,940,620 \$	1,828,927 \$	2,759,258 \$	5,209,303 \$	22,010,423 \$	98	261,349
3520	Power tunnel Access	68,265	m³	1,015,455 \$	1,084,886 \$	316,303 \$	248,717 \$	196,146 \$	2,861,507 \$	42	42,311
3530	Intake excavation	15,000	m³	213,120 \$	165,324 \$	- \$	80,501 \$	52,238 \$	511,183 \$	34	8,880
3540	Intake structure	760	m³	214,184 \$	79,486 \$	189,554 \$	81,615 \$	40,367 \$	605,206 \$	796	8,930
3600	Dams and Spillway										
3,610	Diversion Tunnels (including concrete plug)										
3611	Dam 1 - Diversion Tunnel	7,125	m³	131,168 \$	152,619 \$	25,997 \$	46,612 \$	32,473 \$	388,869 \$	55	5,465
3614	Dam 4 - Diversion Tunnel	5,788	m³	134,738 \$	174,030 \$	24,783 \$	45,578 \$	30,833 \$	409,962 \$	71	5,614
3620	Cofferdams										
3621	Dam 1 - Cofferdams			221,952 \$	61,591 \$	18,975 \$	101,976 \$	78,021 \$	482,515 \$		9,284
3624	Dam 4 - Cofferdams			200,670 \$	31,815 \$	81,938 \$	73,698 \$	63,604 \$	451,725 \$		8,328
3625	Dam 5 - Cofferdams			196,560 \$	38,904 \$	61,851 \$	84,252 \$	72,770 \$	454,337 \$		8,136
3630	Foundation										
3631	Dam 1 - Foundation			101,040 \$	232,886 \$	- \$	26,226 \$	24,123 \$	384,275 \$		4,210
3632	Dam 2 - Foundation			180,480 \$	243,034 \$	- \$	73,562 \$	61,150 \$	558,226 \$		7,520
3633	Dam 3 - Foundation			344,899 \$	247,572 \$	62,953 \$	235,382 \$	183,794 \$	1,074,600 \$		14,372
3634	Dam 4 - Foundation			55,440 \$	215,567 \$	- \$	13,950 \$	12,820 \$	297,777 \$		2,310
3635	Dam 5 - Foundation			96,240 \$	232,586 \$	- \$	25,230 \$	22,964 \$	377,020 \$		4,010
3640	Impervious core										
3641	Dam 1 - Impervious core	14,300	m³	417,582 \$	199,835 \$	336,997 \$	244,061 \$	172,509 \$	1,370,984 \$	96	21,639
3642	Dam 2 - Impervious core	13,700	m³	460,365 \$	259,699 \$	460,833 \$	260,334 \$	146,232 \$	1,587,463 \$	116	23,102
3644	Dam 4 - Impervious core	7,300	m³	420,489 \$	136,199 \$	204,431 \$	188,348 \$	141,866 \$	1,091,333 \$	149	19,612
3645	Dam 5 - Impervious core	16,500	m³	561,181 \$	233,563 \$	356,741 \$	296,328 \$	206,688 \$	1,654,501 \$	100	27,858
3650	Rockfill										
3651	Dam 1 - Rockfill	82,950		618,665 \$	490,062 \$	- \$	401,931 \$	326,667 \$	1,837,325 \$	22	25,923
3652	Dam 2 - Rockfill	64,700		501,244 \$	415,731 \$	- \$	349,118 \$	289,240 \$	1,555,333 \$	24	21,033
3653	Dam 3 - Rockfill	160,000		926,340 \$	546,595 \$	376,818 \$	568,409 \$	507,989 \$	2,926,151 \$	18	38,635

WBS	Description	Quantity	Un.	Man Power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption	TOTAL PRICE	Unit price	Men-hours
3654	Dam 4 - Rockfill	32,900		102,441 \$	29,906 \$	- \$	88,110 \$	81,095 \$	301,552 \$	9	4,169
3655	Dam 5 - Rockfill	105,200		282,006 \$	69,249 \$	- \$	233,321 \$	208,158 \$	792,734 \$	8	11,533
3660	Spillways										
3661	Spillway 1			231,937 \$	80,649 \$	187,096 \$	81,447 \$	38,680 \$	619,809 \$		9,669
3662	Spillway 2			356,895 \$	120,344 \$	258,771 \$	126,096 \$	50,576 \$	912,682 \$		14,862
3670	Transfer Tunnels										
3671	Tunnel T1 Excavation			1,874,496 \$	1,789,893 \$	406,553 \$	573,566 \$	1,027,010 \$	5,671,518 \$		78,105
3672	Tunnel T1 Intake structure	900	m³	210,386 \$	121,631 \$	336,506 \$	78,105 \$	46,683 \$	793,311 \$		8,772
3680	Canals										
3681	Canals 1 and 2	20,000	m³	148,780 \$	236,914 \$	- \$	99,549 \$	80,602 \$	565,845 \$	28	6,218
3683	Canal 3	92,000	m³	308,880 \$	294,203 \$	- \$	201,229 \$	145,230 \$	949,542 \$	10	12,843
3684	Canal 4	4,500	m³	35,280 \$	31,798 \$	- \$	15,278 \$	11,293 \$	93,649 \$	21	1,470
3700	Electrical Works										
3710	Supply and Installation of Transformers and Power cables			163,200 \$	11,400 \$	4,000,000 \$	153,000 \$	- \$	4,327,600 \$		6,800
3720	Supply and Installation of High voltage distribution plant			1,210,560 \$	57,800 \$	5,445,000 \$	434,600 \$	- \$	7,147,960 \$		50,440
3730	Permanent camp Utilities Substation										
3731	Water treatment Area Substation										
3732	Administration Building Area Substation										
3733	Sewage Treatment Area Substation										
3734	Fire & Process Water Area Pumping Station Substation										
3735	Maintenance Shop and Warehouse Area Substation										
3736	Port Facility Substation										
3740	Emergency Generator			2,400 \$	1,000 \$	170,000 \$	1,500 \$	- \$	174,900 \$		100
3750	Plant Communications			156,000 \$	25,000 \$	780,000 \$	100,000 \$	- \$	1,061,000 \$		6,500
3760	Power plant Command Circuitry			1,200,000 \$	300,000 \$	4,100,000 \$	700,000 \$	- \$	6,300,000 \$		50,000
3770	Switch yard Site	50,000	m³	101,670 \$	218,160 \$	- \$	87,778 \$	244,763 \$	652,371 \$	13	4,270
3780	Supply Line to Power Tunnel Intake	19	km	292,616 \$	114,303 \$	2,832,101 \$	86,257 \$	55,358 \$	3,380,635 \$	177.928	12,200
3790	Supply Line to Tunnel 1 Intake	20	km	292,052 \$	114,210 \$	3,108,553 \$	81,220 \$	50,993 \$	3,647,028 \$	182.351	12,180
3800	Mechanical + Electrical Works										
3810	Supply and Installation of Turbine/Generators assemblies			5,670,000 \$	619,969 \$	24,047,921 \$	309,985 \$	- \$	30,647,875 \$		162,000
3820	Supply and installation of Power tunnel intake Gates and Valves			570,500 \$	- \$	838,000 \$	- \$	- \$	1,408,500 \$		16,300
3830	Supply and installation of Tunnel 1 Regulating gates			539,000 \$	- \$	825,000 \$	- \$	- \$	1,364,000 \$		15,400

WBS	Description	Quantity	Un.	Man Power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption	TOTAL PRICE	Unit price	Men-hours
3840	Supply and installation of Draft tube Gates			140,000 \$	- \$	230,000 \$	- \$	- \$	370,000 \$		4,000
3850	Supply the overhead crane	1	ls	- \$	- \$	1,575,000 \$	- \$	- \$	1,575,000 \$		0
3860	Underground Utilities										
3861	Fire water System			87,500 \$	10,780 \$	404,250 \$	26,950 \$	- \$	529,480 \$		2,500
3862	Potable Water System			112,000 \$	15,920 \$	597,000 \$	39,800 \$	- \$	764,720 \$		3,200
3863	Sewage and Sanitary System			241,500 \$	47,920 \$	1,916,800 \$	119,800 \$	- \$	2,326,020 \$		6,900
3864	Compressed Air System			61,250 \$	12,160 \$	486,400 \$	30,400 \$	- \$	590,210 \$		1,750
3865	Process Water System			70,000 \$	13,600 \$	544,000 \$	34,000 \$	- \$	661,600 \$		2,000
3866	CVAC			402,500 \$	79,200 \$	3,168,000 \$	198,000 \$	- \$	3,847,700 \$		11,500
3900	Architectural works										
3,910	Service Building			- \$	- \$	5,497,800 \$	- \$	- \$	5,497,800 \$		
Directs costs - Sub-Total				39,906,800 \$	27,384,511 \$	76,453,264 \$	18,443,462 \$	11,874,337 \$	191,167,128 \$		1,591,019

Item : Indirect Summary

WBS	Description	Quantity	Un.	Man Power	Construction materials	Permanent Materials	Equipment Operation	Fuel Consumption	TOTAL PRICE	Unit price	Men-hours	
6100 Temporary Construction Facilities												
6110	Work Areas, including Buildings											
6113	Work Areas, including Buildings - Hydro Site 6g			936,972 \$	4,374,120 \$	- \$	207,052 \$	119,280 \$	5,635,744 \$		38,960	
6120	Roads , Walkways, Parking Lots											
6123	Roads , Walkways, Parking Lots - Hydro Site 6g			38,880 \$	10,000 \$	- \$	31,408 \$	20,471 \$	100,759 \$		1,620	
6130	Utilities											
6133	Utilities - Hydro Site 6g			315,024 \$	1,763,945 \$	- \$	1,534,882 \$	7,375,596 \$	10,989,447 \$		13,126	
6140	Weather Protection											
6143	Weather Protection - Hydro Site 6g				Special weather protection is included in appropriate items							
6200 Construction Services												
6210	General Site Operation											
6213	General Site Operation - Hydro Site 6g			2,196,638 \$	2,998,288 \$	- \$	715,153 \$	2,690,348 \$	8,600,427 \$		91,527	
6220	Final Clean Up											
6223	Final Clean Up - Hydro Site 6g			280,800 \$	10,000 \$	- \$	57,321 \$	34,903 \$	383,024 \$		11,700	
6230	Material Handling & Warehousing											
6233	Material Handling & Warehousing - Hydro Site 6g			5,151,072 \$	1,592,800 \$	- \$	2,279,162 \$	1,273,131 \$	10,296,165 \$		214,628	
6240	NDE & QA/QC Testing Services											
6243	NDE & QA/QC Testing Services - Hydro Site 6g			- \$	- \$	- \$	- \$	- \$	2,000,000 \$		0	
6250	Surveying											
6253	Surveying - Hydro Site 6g			- \$	156,000 \$	- \$	- \$	- \$	156,000 \$		0	
6260	Site Security											
6263	Site Security - Hydro Site 6g			- \$	586,639 \$	- \$	- \$	- \$	586,639 \$		0	
6270	Man Power Transportation											
6273	Man Power Transportation - Hydro Site 6g			- \$	- \$	- \$	- \$	- \$	25,767,681 \$		0	
6280	General Expenses											
6283	General Expenses - Hydro Site 6g			- \$	852,700 \$	- \$	- \$	- \$	852,700 \$		0	
6300 Construction Equipment, Tools & Supplies												
6330	Construction Equipment, Tools & Supplies - Hydro Site 6g								58,817,800 \$			
6400 Material Transportation												
6430	Material Transportation - Hydro Site 6g			- \$	- \$	16,500,000 \$	- \$	- \$	16,568,461 \$		0	
6500 Construction Camp												
6510	Site Preparation											
6513	Site Preparation - Hydro Site 6g	120,100	m²	119,280 \$	10,000 \$	- \$	92,656 \$	- \$	221,936 \$		7,479	
6520	Infrastructure											
6523	Infrastructure - Hydro Site 6g			1,710,720 \$	7,774,930 \$	11,142,230 \$	869,498 \$	490,379 \$	21,987,757 \$		71,280	
6530	Camps											

Item : Indirect Summary

WBS	Description	Quantity	Un.	Man Power	Construction materials	Permanent Materials	Equipment Operation	Fuel Consumption	TOTAL PRICE	Unit price	Men-hours
6533	Camps - Hydro Site 6g			- \$	13,547,055 \$	81,614,737 \$	- \$	- \$	95,161,792 \$		0
6540	Catering										
6543	Catering - Hydro Site 6g			- \$	12,044,518 \$	- \$	- \$	- \$	12,044,518 \$		0
6550	Operation										
6553	Operation - Hydro Site 6g			145,440 \$	844,580 \$	6,222 \$	1,614,129 \$	16,602,261 \$	19,212,632 \$		6,060
6600	Insurance, Taxes, Permits, Fees										
6630	Insurance, Taxes, Permits, Fees - Hydro Site 6g								24,908,494 \$		
6700	Miscellaneous										
6730	Freight - Hydro Site 6g								46,247,259 \$		
7000	EPCM Home Office										
7100	EPCM Home Office - FEL 1 & 2										
7130	EPCM Home Office - FEL 1 & 2 - Hydro Site 6g								5,823,343 \$		
8000	EPCM Field Office										
8100	EPCM Field Office - FEL 1 & 2										
8130	EPCM Field Office - FEL 1 & 2 - Hydro Site 6g								45,840,000 \$		
9000	Contingency										
9003	Hydro Site 6g - Contingency								79,884,241 \$		
	Indirects costs - Sub-Total							28,606,369 \$	492,086,819 \$		456,380
	Directs costs - Sub-Total							11,874,337 \$	191,167,128 \$		1,591,019
	Miscellaneous non accounted hours										250,000
	Total Costs							40,480,706 \$	683,253,947 \$		2,297,399

Item : 3100

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
3130	Exterior Lighting																	
	Included in Helicopter Pad									0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
3130	Exterior Lighting									0	0	0	0	0	0	0		0
3140	Helicopter pad			0	0													
	Helicopter pad									0	0	0	0	0	0	0		
	Powerhouse area 30,000 (300 x 100) 42,000 m²									0	0	0	0	0	0	0		
	Roads 12,000 (3000 x 4) 3,000 m² / sh									0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
	- M-P 6			840	h	24.00				20,160	0	0	0	0	0	0	20,160	840
										0	0	0	0	0	0	0		
	- Cat D7R II LGP Track-Type Tractor 38.25 28.00 90% 1 126 h									0	0	0	4,820	2,540	7,360			
	- Cat 725 Articulated Dumper 25 T 24.00 20.00 90% 3 378 h									0	0	0	9,072	5,443	14,515			
	- Cat 329DL Hydraulic Excavator 19.00 29.00 90% 1 126 h									0	0	0	2,394	2,631	5,025			
										0	0	0	0	0	0			
	- Misc. (Dust control, fuel depot, accessories, etc..) 1 ls					200,000				0	200,000	0	0	0	200,000			
										0	0	0	0	0	0			
										0	0	0	0	0	0			
										0	0	0	0	0	0			
3140	Helicopter pad									20,160	200,000	0	16,286	10,614	247,060		840	

Item : 3200

WBS	DESCRIPTION	UNIT PRICES									TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
										24.00 \$					0.72 \$			
3210	Wharf									0	0	3,931,360	0	0	3,931,360			0
3220	General Material Receiving and Handling			1	ls													
	- Boom truck 17 tons			1	ls					0	0	200,000	0	0	200,000			0
	- Miscellaneous			1	ls					0	0	50,000	0	0	50,000			0
3220	General Material Receiving and Handling									0	0	250,000	0	0	250,000			0
3230	Storage warehouse																	
	- Supply	10	x	50	500 m ²		137.00			0	68,500	0	0	0	68,500			
	- Install				457 h	24.00				10,968	0	0	0	0	10,968			457
	- Equipment				500 m ²			30.00		0	0	0	15,000	0	15,000			
3230	Storage warehouse									10,968	68,500	0	15,000	0	94,468			457
3240	Workshop and Miscellaneous			0	0													
	Included in Architectural Works									0	0	0	0	0	0			0
3240	Workshop and Miscellaneous									0	0	0	0	0	0			0
3250	Office for custom authorities			0	0													
	Included in Architectural Works									0	0	0	0	0	0			0

Item : 3300

WBS	DESCRIPTION	UNIT PRICES									TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS								
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption											
											24.00 \$					0.75 \$										
- M-P	3.049 h/m			7,482 h		22.00					164,609	0	0	0	0	0	164,609								7,482	
- Other	291.10 \$ /m			2,454 m							0	0	0	0	0	0	714,359									
Intake - Lake Imarsuaq	1,115	4									0	0	0	0	0	0	0									
Access road to Canal 1	390	4									0	0	0	0	0	0	0									
Access road to Dam 5	878	4									0	0	0	0	0	0	0									
Access road to Canal 4	1,047	4									0	0	0	0	0	0	0									
	3,430										0	0	0	0	0	0	0									
- M-P	2.929 h/m			10,046 h		22.00					221,022	0	0	0	0	0	221,022								10,046	
- Other	258.30 \$ /m			3,430 m							0	0	0	0	0	0	885,969									
											0	0	0	0	0	0	0									
											0	0	0	0	0	0	0									
3350	Cross section C2										385,631	0	0	0	0	0	1,985,959								17,529	

3360 Cross section C3				3,420 m																						
											0	0	0	0	0	0	0									
Harbour - Intake	1,910	5	<u>W(m)</u>								0	0	0	0	0	0	0									
- M-P	12.475 h/m			23,827 h		22.00					524,200	0	0	0	0	0	524,200								23,827	
- Other	1,220.30 \$ /m			1,910 m							0	0	0	0	0	0	2,330,773									
Intake - Lake Imarsuaq	1,510	4									0	0	0	0	0	0	0									
Access road to Canal 1		4									0	0	0	0	0	0	0									
Access road to Dam 5	0	4									0	0	0	0	0	0	0									
Access road to Canal 4	0	4									0	0	0	0	0	0	0									
	1,510										0	0	0	0	0	0	0									
- M-P	12.289 h/m			18,556 h		22.00					408,241	0	0	0	0	0	408,241								18,556	
- Other	1,110.40 \$ /m			1,510 m							0	0	0	0	0	0	1,676,704									
											0	0	0	0	0	0	0									
											0	0	0	0	0	0	0									
3360	Cross section C3										932,441	0	0	0	0	0	4,939,918								42,384	

3370 Bridges				2 un																						
Harbour - Intake	6+340 to 6+370	32,000 \$ /m	30 m	32,000							0	0	0	0	0	0	0									
Intake - Lake Imarsuaq	4+940 to 5+010	23,000 \$ /m	70 m	23,000							0	1,610,000	0	0	0	0	1,610,000									
Vented Causeway		80,000 \$ /un	24 un	80,000							0	1,920,000	0	0	0	0	1,920,000									
Bed level Causeway		6,000 \$ /un	122 un	6,000							0	732,000	0	0	0	0	732,000									
											0	0	0	0	0	0	0									

Item : 3300

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.75 \$			
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
3370	Bridges									0	5,222,000	0	0	0	0	5,222,000		0

3380 Air Lift

**Helicopter capacity
(for 1 hour trip)**

Type	(kg)
Sikorsky S-61N	3,750
AS 350 B3	1,200
Bell 214	2,700
Boeing 234	12,700

Cost per hour

	Mob	liter / hour	Fuel	Rental	(\$ / hour)
		1.67 \$			
S-61N		600	1,002.00 \$	9,667 \$	
AS 350 B3	31,350 \$	200	334.00 \$	2,180 \$	
Bell 214	39,200 \$	600	1,002.00 \$	5,416 \$	
Boeing 234	800,000 \$	1,515	2,530.05 \$	8,130 \$	

Air Transportation

Boeing 234

	Weight	Air lift #	Qty	Lifts
Cat 311C U	11,980	1	2	2
		2	2	2
Cat 950H Wheel Loader	18,500	1	1	3
		2	1	3
Compressor XAHS 237 (500 cfm)	3,000	1	2	2
		2	3	3
Furukawa HCR9-ES	9,000	1	3	3
		2	3	5

Miscellaneous

- Equipment air lift

					26	26 h		8,130										
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- Mobilisation

Shared with catering and transportation			40%			1 ls		320,000										
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Bell 214

	Air lift	Days	Trips / day
	1	2	
	83	60	
	332	240	
			143
			572

- Men

572 trips	1 h / trip			572 h		5,416												
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- Miscellaneous

1 trip / day	1 h / trip		143 days	143 h		5,416												
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Item : (3411)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.75 \$		

3410 Excavation

3411 Powerhouse and Access				113,400 m³													
Access to Powerhouse				81,400 m³						0	0	0	0	0	0	0	0
Powerhouse				32,000 m³						0	0	0	0	0	0	0	0
				113,400 m³						0	0	0	0	0	0	0	0
Access to Powerhouse										0	0	0	0	0	0	0	0
Horse shoe	10 x 10		92.5 m³	880 m	81,400 m³					0	0	0	0	0	0	0	0
		<u>Dia.</u>	<u>Area (m²)</u>							0	0	0	0	0	0	0	0
Arc	11.59	15	17.50							0	0	0	0	0	0	0	0
Height	10.00									0	0	0	0	0	0	0	0
Wall	7.50		75.00							0	0	0	0	0	0	0	0
Width	10.00									0	0	0	0	0	0	0	0
			92.5							0	0	0	0	0	0	0	0
Excavation										0	0	0	0	0	0	0	0
Progression	4.66 m									0	0	0	0	0	0	0	0
Number of rounds	189									0	0	0	0	0	0	0	0
Number of shifts	265	Prod. Factor	1.4							0	0	0	0	0	0	0	0
<u>Number of holes</u>			<u>(m)</u>	<u>(Feet)</u>						0	0	0	0	0	0	0	0
Production	74	55 mm dia.	70,350	230,747						0	0	0	0	0	0	0	0
Contour	41	55 mm dia.	38,977	127,846						0	0	0	0	0	0	0	0
			115							0	0	0	0	0	0	0	0
Cut	3	109 mm dia.	2,852	9,355						0	0	0	0	0	0	0	0
			118							0	0	0	0	0	0	0	0
Drilling depth	5.03 m		112,179	367,947						0	0	0	0	0	0	0	0
Durations				(hours)	189 rounds						0	0	0	0	0	0	0
Drilling	150 m / h		3.96	748 h						0	0	0	0	0	0	0	0
Blasting	1.15 min / hole		2.26	427 h						0	0	0	0	0	0	0	0
Scaling & W. mesh			2.00	378 h						0	0	0	0	0	0	0	0
Mucking	205 m³ / h		2.10	397 h						0	0	0	0	0	0	0	0
Drilling labour											0	0	0	0	0	0	0
	Total M-H	Bolting	W. Mesh	Remaining							0	0	0	0	0	0	0
	8	21,200	2,968	2,544	15,688						0	0	0	0	0	0	0
		14%	12%								0	0	0	0	0	0	0
Drilling	4.00	189	756 h							0	0	0	0	0	0	0	0
		9 h / sh	84 sh							0	0	0	0	0	0	0	0
	8 men / sh	10 h / sh	6,720 m-h							0	0	0	0	0	0	0	0
Loading & Blasting	2.26	189	427 h							0	0	0	0	0	0	0	0
		9 h / sh	47 sh							0	0	0	0	0	0	0	0
	8 men / sh	10 h / sh	3,800 m-h							0	0	0	0	0	0	0	0
Remaining for services			5,168							0	0	0	0	0	0	0	0
Drilling			840 h							0	0	0	0	0	0	0	0
- M-P			8	6,720 h	24.00					161,280	0	0	0	0	161,280	6,720	
		189 rounds								0	0	0	0	0	0	0	

Item : (3411)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.75 \$		
	- Cat 329DL Hydraulic Excavator	19.00	29.00	50%	1					19.00	29.00	0	0	0	0	0	0	2,038
	- Cat 988H Wheel Loader	39.20	48.00	90%	1					39.20	48.00	0	0	0	3,489	3,204	0	6,693
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1					38.25	28.00	0	0	0	3,404	1,869	0	5,273
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	3					24.00	20.00	0	0	0	6,432	4,020	0	10,452
	Disposal of excavated materials																	
	Average hauling distance :		1.00 km															
	Loading	8																
	Going	2	30 km / h															
	Unloading	3																
	Return	2	30 km / h															
		15 min.																
	Efficacité :	85%	18 min. / trip															
			0.29 h / trip															
			9 h / sh															
			31 trips / sh															
	Cat 725 Articulated Dumper 25 T		12 m³															
			372 m³ / truck-sh															
	Number of trucks :		3															
	BENCHES																	
	Bench 1	H	W	(m²)	L													
		8.8	16.5	145.2	67.52													
	Bench 2	10.8	16.5	178.2	67.52													
		2	13.54	27.08	67.52													
				205.28														
					23,664 m³													
	Progression	10 m																
	Rounds	7 rounds / bench																
	Line drilling	0.6 m c/c																
	Damper holes	16 un / round																
	Helper holes	16 un / round																
	Production holes	39 un / round																
	DRILLING																	
	2,5" dia.	Lentgh	Holes	Depth	Drilling													
	B1	Line drilling	168	280	9.3	2,604												
	B2	Line drilling	168	280	11.3	3,164												
	B1	Damper		112	9.3	1,042												
	B2	Damper		112	11.3	1,266												
	3" dia.																	
	B1	Helper		112	9.3	1,042												
	B2	Helper		112	11.3	1,266												
	B1	Production		273	9.3	2,539												
	B2	Production		273	11.3	3,085												
				1,554														
	Preshearing area	3,461 m²																
	Durations				250 m / machine / sh													
			(m)	(shift)														
	Line drilling		5,768	23														
	Damper		2,308	9														
	Production & Helpers		7,932	32														
			16,008	64														
					3 mach													
						21 sh												

Item : (3411)

WBS	DESCRIPTION		Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
					M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
		10 h / sh	210 h							24.00 \$				0.75 \$			
- M-P			6	1,260 h	24.00						30,240	0	0	0	30,240	1,260	
- Hydraulic Drilling Machine	19.40	15.00	90%	3	567 h			19.40	15.00		0	0	0	11,000	6,379	17,379	
	Feet	ft / un									0	0	0	0	0	0	
- Bits 2,5"Ø	26,496	1,500			18 un	160.00					0	2,880	0	0	0	2,880	
- Bits 3"Ø	26,023	1,500			17 un	200.00					0	3,400	0	0	0	3,400	
- Rod 18'	52,519	5,000			11 un	500.00					0	5,500	0	0	0	5,500	
- Coupling	52,519	3,000			18 un	50.00					0	900	0	0	0	900	
- Shank	52,519	10,000			5 un	300.00					0	1,500	0	0	0	1,500	
- Misc. Materials	52,519				52,519 ft	0.05					0	2,626	0	0	0	2,626	
LOADING & BLASTING			23,664	m³							0	0	0	0	0	0	
- Cordex		0.52 m / m³			12,305 m	0.60					0	7,383	0	0	0	7,383	
- Xactex		0.1 kg m³			2,366 kg	7.50					0	17,748	0	0	0	17,748	
- Detonators		0.04 un / m³			947 un	7.00					0	6,626	0	0	0	6,626	
- Dynamite		0.96 kg / m³			22,718 kg	6.00					0	136,307	0	0	0	136,307	
Production of	1,554	5 min / hole		130 h							0	0	0	0	0	0	
		Eff. Factor		0.75							0	0	0	0	0	0	
				173 h							0	0	0	0	0	0	
- M-P			4	692 h	24.00						16,608	0	0	0	16,608	692	
- Explosives Truck	5.00	15.00	90%	1	156 h			5.00	15.00		0	0	0	780	1,755	2,535	
MUCKING											0	0	0	0	0	0	
		23,664 m³ (bank)									0	0	0	0	0	0	
		35,497 m³ (loose)									0	0	0	0	0	0	
		2,535 m³ / round									0	0	0	0	0	0	
Production of		1,250 m³ / sh		28 sh							0	0	0	0	0	0	
				280 h							0	0	0	0	0	0	
- M-P			##	2,800 h	24.00						67,200	0	0	0	67,200	2,800	
- Cat 988H Wheel Loader	39.20	48.00	90%	1	252 h			39.20	48.00		0	0	0	9,878	9,072	18,950	
- Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	252 h			19.00	29.00		0	0	0	4,788	5,481	10,269	
- Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	2	504 h			24.00	20.00		0	0	0	12,096	7,560	19,656	
- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	252 h			38.25	28.00		0	0	0	9,639	5,292	14,931	
Disposal of excavated materials											0	0	0	0	0	0	
		Average hauling distance :		0.50 km							0	0	0	0	0	0	
											0	0	0	0	0	0	
Loading		3									0	0	0	0	0	0	
Going		1		30 km / h							0	0	0	0	0	0	
Unloading		3									0	0	0	0	0	0	
Return		1		30 km / h							0	0	0	0	0	0	
		8		min.							0	0	0	0	0	0	
Efficacité :		85%		9 min. / trip							0	0	0	0	0	0	
				0.16 h / trip							0	0	0	0	0	0	
				9 h / sh							0	0	0	0	0	0	
				58 trips / sh							0	0	0	0	0	0	
				12 m³							0	0	0	0	0	0	
Cat 725 Articulated Dumper 25 T				696 m³ / truck-sh							0	0	0	0	0	0	

Item : (3411)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption					
	Number of trucks :			2									24.00 \$				0.75 \$			
	Rock Support	L	H	Area																
	Area	168	19.6	3,294 m²																
	Supply																			
	- Rock bolts 6 m	5 m² / un	659 un	Losses	3%	679 un		110.00					0	74,690	0	0	0	0	74,690	
	- Wire mesh	2,789 m²		Lapping	15%	3,208 m²		4.60					0	14,757	0	0	0	0	14,757	
	- Spikes 0,7 m	1.56 m² / un	1,788 un		3%	1,842 un		4.50					0	8,289	0	0	0	0	8,289	
	- Wire					2,789 m²		0.04					0	112	0	0	0	0	112	
	Rock bolts Installation																			
	Production of			100 m / sh		40 sh														
				17 un / sh																
	6 m	3,954 m		10 h / sh		400 h							0	0	0	0	0	0	0	
	- M-P				6	2,400 h		24.00					57,600	0	0	0	0	0	57,600	2,400
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	360 h			37.00	20.00		0	0	0	13,320	5,400	18,720		
	- Fork lift 15 T	13.00	9.00		90%	1	360 h			13.00	9.00		0	0	0	4,680	2,430	7,110		
	- Boom truck 17 tons	13.65	18.00		90%	1	360 h			13.65	18.00		0	0	0	4,914	4,860	9,774		
	- Drilling rig (on fork lift)				90%	1	360 h			11.00			0	0	0	3,960	0	3,960		
	Rock bolts Injection																			
	Production of			40 un / sh		17 sh														
				10 h / sh		170 h														
	- M-P				5	850		24.00					20,400	0	0	0	0	0	20,400	850
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	153 h			37.00	20.00		0	0	0	5,661	2,295	7,956		
	- Moyno pump	2.00			75%	1	128 h			2.00			0	0	0	256	0	256		
	- Cement (bags)	3,954 m			100%	646 bags				10.00			0	0	6,460	0	0	6,460		
		12,969 ft		0.02269801 sf									0	0	0	0	0	0		
		2 in. Dia hole		294 cu ft									0	0	0	0	0	0		
		0.91 bag / cu ft		323 bags									0	0	0	0	0	0		
	- Intraplast "N"	0.4 kg / bag		129 kg	1%	130 kg				3.00			0	0	390	0	0	390		
	- Miscellaneous					659 un		0.30					0	198	0	0	0	198		
	Wire mesh Installation																			
	Production of			240 m² / sh		13 sh														
				10 h / sh		130 h														
	- M-P				5	650 h		24.00					15,600	0	0	0	0	0	15,600	650
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	117 h			37.00	20.00		0	0	0	4,329	1,755	6,084		
	- Jack leg	2.00			30%	39 h				2.00			0	0	0	78	0	78		
	- Misc. Drilling materials			1,788 un		0.7 m		1.00					0	1,252	0	0	0	1,252		
	Wire mesh removing	(under level 15)																		
		L	H	Area	m²															
		168	7	1,176																
	Production of			600 m² / sh		2 sh														
				10 h / sh		20 h														

Item : (3411)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption					
	Supply																			
			<u>Lenght (m)</u>																	
	- Rock bolts 2.5 m	321 un	828	Losses	3%	331 un				60.00				0	0	19,860	0	19,860		
	- Rock bolts 3 m	60 un	186	Losses	3%	62 un				70.00				0	0	4,340	0	4,340		
	- Rock bolts 4 m	38 un	156	Losses	3%	39 un				80.00				0	0	3,120	0	3,120		
	- Rock bolts 5 m	17 un	90	Losses	3%	18 un				105.00				0	0	1,890	0	1,890		
		436	1,260											0	0	0	0	0		
	- Injection tubes	150 m roll			3%	9 rolls				110.00				0	0	990	0	990		
	- Oakum	130 bolts / box			3%	4 box				280.00				0	0	1,120	0	1,120		
	- Grease	154 bolts / box			3%	3 box				336.00				0	0	1,008	0	1,008		
														0	0	0	0	0		
	- Wire mesh	3,928 m ²			15%	4,517 m ²				4.60				0	0	20,778	0	20,778		
	- Reinf. Mesh	102 m ²			15%	117 m ²				5.60				0	0	655	0	655		
		4,030 m ²												0	0	0	0	0		
	- Spikes 1,1 m	1.25 m c/c	3,224 un		3%	3,321 un				4.50				0	0	14,945	0	14,945		
	- Wire		0.04 \$ / m ²			4,030 m ²				0.04				0	0	161	0	161		
		m ²	m ³											0	0	0	0	0		
	Shotcrete 50 mm	841	0.05	42										0	0	0	0	0		
	Shotcrete 100 mm	102	0.1	10										0	0	0	0	0		
				52										0	0	0	0	0		
	- Cement (40 kg Bags)	0.03 m ³ / bag		Losses	7.5%	1,873 bags				10.00				0	0	18,730	0	18,730		
		33.33 bags / m ³	1,742 bags											0	0	0	0	0		
	- Sand	1.40 mt / m ³	0.11 h / mt			73 mt	2.61	8.08	0.00	2.60	11.98			191	591	0	190	658	1,630	8
														0	0	0	0	0	0	
	- Monoset (3% of cement)	69,699 kg			3%	2,091 kg				3.40				0	0	7,109	0	7,109		
														0	0	0	0	0	0	
	- Steel arch (W 100)	19.0 kg / m	87 m			1,644 kg				4.00				0	0	6,574	0	6,574		
	- Steel arch (W 150)	22.0 kg / m	35 m			761 kg				5.00				0	0	3,806	0	3,806		
														0	0	0	0	0	0	
	Rock bolts Installation					88 sh								0	0	0	0	0	0	
		1,260 m	14 m / sh																	
		436 un	5 un / sh																	
			0.5 h / un. including positioning																	
			3 h / sh			264 h														
	1) Drilling with Jumbo													0	0	0	0	0	0	
														0	0	0	0	0	0	
	- Jumbo				90%	1	238 h				102.50			0	0	0	24,395	0	24,395	
														0	0	0	0	0	0	
	2) Install with 50t crane with basket													0	0	0	0	0	0	
														0	0	0	0	0	0	
	- M-P					3	792 h	24.00						19,008	0	0	0	0	19,008	792
														0	0	0	0	0	0	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	238 h			37.00	20.00			0	0	0	8,806	3,570	12,376	
														0	0	0	0	0	0	
	- Impact tool					1 un				300.00				0	300	0	0	0	300	
	- Test rig					1 un				1,200.00				0	1,200	0	0	0	1,200	
	- Torque rench					1 un				280.00				0	280	0	0	0	280	
														0	0	0	0	0	0	
	3) Injection	40 bolts / sh				11 sh								0	0	0	0	0	0	
			10 h / q			110 h								0	0	0	0	0	0	
														0	0	0	0	0	0	
	- M-P					4	440 h	24.00						10,560	0	0	0	0	10,560	440
														0	0	0	0	0	0	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	99 h			37.00	20.00			0	0	0	3,663	1,485	5,148	
	- Moyno pump	2.00			75%	1	83 h			2.00				0	0	0	166	0	166	
														0	0	0	0	0	0	

Item : (3411)

WBS	DESCRIPTION		Qty		Un.		UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
							M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$						0.75 \$		
-	Cement (bags)	1,260 m 4,131 ft	0.02269801 sf	100%	206	bags			10.00				0	0	2,060	0	0	2,060	
		2 in. Dia hole	94 cu ft										0	0	0	0	0	0	
		0.91 cu ft / bag	103 bags										0	0	0	0	0	0	
-	Intraplast "N"	0.4 kg / bag	41 kg	1%	42	kg			3.00				0	0	126	0	0	126	
-	Miscellaneous				436	un			0.30				0	131	0	0	0	131	
	Wire mesh installation												0	0	0	0	0	0	
	Installation by Jumbo team												0	0	0	0	0	0	
	Production of	200 m² / sh	4,030 m²		20	sh							0	0	0	0	0	0	
			10 h / sh		201	h							0	0	0	0	0	0	
	Plus												0	0	0	0	0	0	
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	181	h			37.00	20.00		0	0	0	6,697	2,715	9,412	
-	Jack leg	2.00		30%		60	h			2.00			0	0	0	120	0	120	
-	Miscellaneous materials	Spike drilling	3,546 m			3,546	m			1.00			0	3,546	0	0	0	3,546	
	Shotcreting					52	m³						0	0	0	0	0	0	
	Production of	0.7 h / m³	37 h										0	0	0	0	0	0	
			7.5 h / sh Eff.			5	sh						0	0	0	0	0	0	
			10 h / sh			50	h						0	0	0	0	0	0	
-	M-P				9	450	h	24.00					10,800	0	0	0	0	10,800	
													0	0	0	0	0	0	
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	45	h			37.00	20.00		0	0	0	1,665	675	2,340	
-	Shotcrete pump	17.00		60%	1	30	h			17.00			0	0	0	510	0	510	
-	Hoses			25%	1	13	h			35.00			0	455	0	0	0	455	
-	Nozzle	66 m³ / un				1	un			275.00			0	275	0	0	0	275	
	Arches installation					7	un						0	0	0	0	0	0	
	Production of	121 m	17 m / un			4	sh						0	0	0	0	0	0	
			10 h / sh			40	h						0	0	0	0	0	0	
-	M-P				5	200	h	24.00					4,800	0	0	0	0	4,800	
													0	0	0	0	0	0	
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	36	h			37.00	20.00		0	0	0	1,332	720	2,052	
-	Miscellaneous materials					7	un			200.00			0	1,400	0	0	0	1,400	
	Outside services are included in TBM Power tunnel												0	0	0	0	0	0	
	Services	Using outside installations for TBM				1,174	m						0	0	0	0	0	0	
		Access tunnel	880										0	0	0	0	0	0	
		Intake tunnel and penstock	294										0	0	0	0	0	0	
			1,174 m										0	0	0	0	0	0	
	Ventilation & Heating												0	0	0	0	0	0	
-	M-P	3.0 h / m			3,521	h	24.00						84,500	0	0	0	0	84,500	
													0	0	0	0	0	0	
-	Miscellaneous materials				1,174	m			10.00				0	11,736	0	0	0	11,736	
													0	0	0	0	0	0	
	Dewatering												0	0	0	0	0	0	
													0	0	0	0	0	0	

Item : (3412)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
				30 rounds							24.00 \$					0.75 \$		
-	Jumbo E 3C	14.00		4.5 h	135 h					14.00		0	0	0	0	0	0	1,890
-	Cat GEP 550 - 400KW	6.50	102.40		135 h					6.50	102.40	0	0	0	878	10,368	11,246	
	Feet	ft / un										0	0	0	0	0	0	0
-	Bits 2"Ø	51,475	1,600		32 un		85.00					0	2,720	0	0	0	0	2,720
-	Bits 4"Ø	1,485	1,500		1 un		500.00					0	500	0	0	0	0	500
-	Rod 18'	52,960	7,500		7 un		485.00					0	3,395	0	0	0	0	3,395
-	Coupling	52,960	3,700		14 un		50.00					0	700	0	0	0	0	700
-	Shank	52,960	12,500		4 un		300.00					0	1,200	0	0	0	0	1,200
-	Misc. Materials	52,960			52,960 ft		0.04					0	2,118	0	0	0	0	2,118
Loading & Blasting					68 h							0	0	0	0	0	0	0
-	M-P			8	547 h	24.00						13,125	0	0	0	0	0	13,125
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1					37.00	20.00	0	0	0	2,294	930	3,224	
-	Fork lift 10 T	11.00	7.00	90%	1					11.00	7.00	0	0	0	682	326	1,008	
-	Explosives Truck	5.00	15.00	90%	1					5.00	15.00	0	0	0	310	698	1,008	
5.03 m holes												0	0	0	0	0	0	0
		30 Rounds										0	0	0	0	0	0	0
		<u>Number</u>	<u>Total</u>	<u>Length (m)</u>								0	0	0	0	0	0	0
	Contour holes	38	1,140	5,734								0	0	0	0	0	0	0
	Production holes	66	1,980	9,959								0	0	0	0	0	0	0
		104	3,120									0	0	0	0	0	0	0
-	Prima cord	5.5 m		6,270 5%	6,584 m		1.00					0	6,584	0	0	0	0	6,584
-	Cap 6m				3,526 un		3.50					0	12,341	0	0	0	0	12,341
-	Dynamite RXL 438	11,550 m³	Powder fact	1.6	18,480 kg		5.60					0	103,488	0	0	0	0	103,488
-	XACTEX	1,140 holes		3,135 5%	3,292 kg		7.50					0	24,690	0	0	0	0	24,690
		2.75 kg / hole										0	0	0	0	0	0	0
Mucking												0	0	0	0	0	0	0
		11,550 m³										0	0	0	0	0	0	0
	1.5 Loose >>>>	17,325 m³										0	0	0	0	0	0	0
		578 m³ / round										0	0	0	0	0	0	0
	Production	140 m³ / h		4.13 h								0	0	0	0	0	0	0
		30 rounds		124 h x 10/9 >>								0	0	0	0	0	0	0
					138 h							0	0	0	0	0	0	0
-	M-P			7	963 h	24.00						23,100	0	0	0	0	0	23,100
-	Cat 329DL Hydraulic Excavator	19.00	29.00	50%	1					19.00	29.00	0	0	0	1,311	1,501	2,812	
-	Cat 988H Wheel Loader	39.20	48.00	90%	1					39.20	48.00	0	0	0	4,861	4,464	9,325	
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1					38.25	28.00	0	0	0	4,743	2,604	7,347	
-	Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	2					24.00	20.00	0	0	0	5,952	3,720	9,672	
Disposal of excavated materials												0	0	0	0	0	0	0
	Average hauling distance :		0.50 km									0	0	0	0	0	0	0
	Loading	8										0	0	0	0	0	0	0
	Going	1	30 km / h									0	0	0	0	0	0	0
	Unloading	3										0	0	0	0	0	0	0
	Return	1	30 km / h									0	0	0	0	0	0	0
		13 min.										0	0	0	0	0	0	0
	Efficacité :	85%	15 min. / trip									0	0	0	0	0	0	0
			0.25 h / trip									0	0	0	0	0	0	0
			9 h / sh									0	0	0	0	0	0	0

Item : (3412)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.75 \$		
	Cat 725 Articulated Dumper 25 T	36 trips / day									0	0	0	0	0	0	0	
		12 m³									0	0	0	0	0	0	0	
		432 m³ / truck-sh									0	0	0	0	0	0	0	
	Number of trucks :	2									0	0	0	0	0	0	0	
	Rolling Path										0	0	0	0	0	0	0	
		Length 140									0	0	0	0	0	0	0	
		Width 8.00									0	0	0	0	0	0	0	
		Thickness 0.30									0	0	0	0	0	0	0	
		Volume 336									0	0	0	0	0	0	0	
	Production	1,200 m³ / sh									0	0	0	0	0	0	0	
						1 sh					0	0	0	0	0	0	0	
						10 h					0	0	0	0	0	0	0	
	- M-P			8	80 h	24.00					1,920	0	0	0	0	0	0	1,920
	- Cat 988H Wheel Loader	39.20	48.00	90%	1						0	0	0	0	353	324	677	
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1						38.25	28.00	0	0	344	189	533	
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	1						24.00	20.00	0	0	216	135	351	
	Rock Support																	
	Horse shoe	10 x 9	82.5 m³	140														
			Area (m²)															
	Arc	11.59	10	17.50														
	Height	9.00																
	Wall	6.50																
	Width	10.00																
			82.5															
	Required																	
		Length	Dia. (m)	Arch (m)														
	Class 1	105.0	12.5	11.59	75%													
	Class 2	21.0	12.5	11.59	15%													
	Class 3	9.8	12.5	11.59	7.0%													
	Class 4	3.5	12.5	11.59	2.5%													
	Class 5	0.7	12.5	11.59	0.5%													
		140			100%													
	Class 1																	
	Rock bolts 2,5 m	1 un / m		105 un														
	Shotcrete 50 mm	18.59 m² / m		293 m²	15%													
	Wire mesh	18.59 m² / m		1,659 m²	85%													
	Class 2																	
	Rock bolts 2,5 m	2.3 un / m		48 un														
	Shotcrete 50 mm	18.59 m² / m		59 m²	15%													
	Wire mesh	18.59 m² / m		332 m²	85%													
	Class 3																	
	Rock bolts 3 m	2.9 un / m		28 un														
	Shotcrete 50 mm	18.59 m² / m		91 m²	50%													
	Wire mesh	18.59 m² / m		91 m²	50%													
	Class 4																	
	Rock bolts 4 m	5.2 un / m		18 un														
	Shotcrete 50 mm	7.0 m² / m		7 m²	30%													
	Wire mesh	7.0 m² / m		17 m²	70%													
	Shotcrete 100 mm	11.6 m² / m		41 m²	100%													
	Reinf. Mesh	11.6 m² / m		41 m²	100%													
	Steel arch (W 100)	1.5 m c/c		2 un														

Item : (3412)

WBS	DESCRIPTION	%	n	Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS								
						M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption											
											24.00 \$					0.75 \$										
	Class 5			24.6 m / arch	49 m																					
	Rock bolts 5 m			11.6 un / m	8 un																					
	Shotcrete 50 mm			7.0 m ² / m	1 m ²	30%																				
	Wire mesh			7.0 m ² / m	3 m ²	70%																				
	Shotcrete 100 mm			11.6 m ² / m	8 m ²	100%																				
	Reinf. Mesh			11.6 m ² / m	8 m ²	100%																				
	Steel arch (W 150)			0.75 m c/c	1 un																					
				24.6 m / arch	25 m																					
	Supply				Lenght (m)																					
	- Rock bolts 2,5 m			153 un	395	Losses 3%					158 un	60.00														
	- Rock bolts 3 m			28 un	87	Losses 3%					29 un	70.00														
	- Rock bolts 4 m			18 un	76	Losses 3%					19 un	80.00														
	- Rock bolts 5 m			8 un	40	Losses 3%					8 un	105.00														
				207	598																					
	- Injection tubes			150 m roll		3%					4 rolls	110.00														
	- Oakum			130 bolts / box		3%					2 box	280.00														
	- Grease			154 bolts / box		3%					1 box	336.00														
	- Wire mesh			2,103 m ²		15%					2,418 m ²	4.60														
	- Reinf. Mesh			49 m ²		15%					56 m ²	5.60														
				2,151 m ²																						
	- Spikes 1,1 m			1.25 m c/c	1,721 un	3%					1,773 un	4.50														
	- Wire				0.04 \$ / m ²						2,151 m ²	0.04														
				m ²	m ²																					
	Shotcrete 50 mm			451 0.05	23																					
	Shotcrete 100 mm			49 0.1	5																					
					27																					
	- Cement (40 kg Bags)			0.03 m ³ / bag		Losses 7.5%					983 bags	10.00														
				33.33 bags / m ³	914 bags																					
	- Sand			1.40 mt / m ³	0.11 h / mt						38 mt	2.61	8.08	0.00	2.60	11.98	100	310	0	100	345	855		4		
	- Monoset (3% of cement)				36,575 kg	3%					1,097 kg	3.40														
	- Steel arch (W 100)			19.0 kg / m	49 m						934 kg	4.00														
	- Steel arch (W 150)			22.0 kg / m	25 m						541 kg	5.00														
	Rock bolts Installation										42 sh															
				598 m	14 m / sh																					
				207 un	5 un / sh																					
					0.5 h / un. including positioning																					
					2.5 h / sh						105 h															
	1) Drilling with Jumbo																									
	- Jumbo					90%	1				95 h		102.50													
	- Cat GEP 550 - 400KW			6.50	102.40						95 h		6.50	102.40												
	2) Install with 50t crane with basket																									
	- M-P						3				315 h	24.00														
	- Crane - Rough terrain 50 t (L-Belt)			37.00	20.00	90%	1				95 h		37.00	20.00												
	- Impact tool										1 un	300.00														
	- Test rig										1 un	1,200.00														

Item : (3412)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.75 \$		
	- Torque rench			1	un		280.00					0	280	0	0	0	280	
3)	Injection	40 bolts / sh		6	sh							0	0	0	0	0	0	
			10 h / sh	60	h							0	0	0	0	0	0	
	- M-P			4	240 h	24.00						5,760	0	0	0	0	5,760	240
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	54 h				37.00	20.00	0	0	0	1,998	810	2,808	
	- Moyno pump	2.00		75%	1	45 h					2.00	0	0	0	90	0	90	
	- Cement (bags)	598 m		100%	98 bags			10.00				0	0	980	0	0	980	
		1,961 ft	0.02269801 sf									0	0	0	0	0	0	
		2 in. Dia hole	45 cu ft									0	0	0	0	0	0	
		0.91 bag / cu ft	49 bags									0	0	0	0	0	0	
	- Intraplast "N"	0.4 kg / bag	20 kg	1%	20 kg			3.00				0	0	60	0	0	60	
	- Miscellaneous				207 un		0.30					0	62	0	0	0	62	
	Wire mesh installation											0	0	0	0	0	0	
	Production of	200 m ² / sh	2,151 m ²		11 sh							0	0	0	0	0	0	
			10 h / sh		108 h							0	0	0	0	0	0	
	- M-P			6	645 h	24.00						15,490	0	0	0	0	15,490	645
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	97 h				37.00	20.00	0	0	0	3,589	1,455	5,044	
	- Jack leg	2.00		30%	1	32 h					2.00	0	0	0	64	0	64	
	- Miscellaneous materials	Spike drilling	1,893 m		1,893 m		1.00					0	1,893	0	0	0	1,893	
												0	0	0	0	0	0	
												0	0	0	0	0	0	
	Shotcreting				27 m ³							0	0	0	0	0	0	
	Production of	0.7 h / m ³	19 h									0	0	0	0	0	0	
			7.5 h / sh Eff.		3 sh							0	0	0	0	0	0	
			10 h / sh		30 h							0	0	0	0	0	0	
	- M-P			5	150 h	24.00						3,600	0	0	0	0	3,600	150
												0	0	0	0	0	0	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	27 h				37.00	20.00	0	0	0	999	405	1,404	
	- Shotcrete pump	17.00		60%	1	18 h				17.00		0	0	0	306	0	306	
	- Hoses			25%	1	8 h		35.00				0	280	0	0	0	280	
	- Nozzle	66 m ³ / un			0 un		275.00					0	0	0	0	0	0	
												0	0	0	0	0	0	
	Arches installation	74 m	25 m / un		3 un							0	0	0	0	0	0	
	Production of	2 un / sh			2 sh							0	0	0	0	0	0	
			10 h / sh		20 h							0	0	0	0	0	0	
	- M-P			5	100 h	24.00						2,400	0	0	0	0	2,400	100
												0	0	0	0	0	0	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	18 h				37.00	20.00	0	0	0	666	270	936	
	- Miscellaneous materials				3 un		200.00					0	600	0	0	0	600	
	Transformer cavern											0	0	0	0	0	0	
		14 x 13,2										0	0	0	0	0	0	
	Arc	15.50	60.60									0	0	0	0	0	0	
	Height	13.20										0	0	0	0	0	0	

Item : (3412)

WBS	DESCRIPTION	%	n	Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
						M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.75 \$		
	5.03 m holes										0	0	0	0	0	0	0	
	12 Rounds										0	0	0	0	0	0	0	
	<u>Number</u> <u>Total</u> <u>Length (m)</u>										0	0	0	0	0	0	0	
	Contour holes	50	600	3,018							0	0	0	0	0	0	0	
	Production holes	144	1,728	8,692							0	0	0	0	0	0	0	
		194	2,328								0	0	0	0	0	0	0	
	- Prima cord	5.5 m		3,300	5%	3,465 m		1.00			0	3,465	0	0	0	0	3,465	
	- Cap 6m			2,328	13%	2,631 un		3.50			0	9,209	0	0	0	0	9,209	
	- Dynamite RXL 438	10,000 m³	Powder fact	1.6		16,000 kg		5.60			0	89,601	0	0	0	0	89,601	
	- XACTEX	600 holes		1,650	5%	1,733 kg		7.50			0	12,998	0	0	0	0	12,998	
		2.75 kg / hole									0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
	Rock Support										0	0	0	0	0	0	0	
			<u>Length</u>	55.68 m							0	0	0	0	0	0	0	
			<u>Area (m²)</u>								0	0	0	0	0	0	0	
	14 x 13,2										0	0	0	0	0	0	0	
	Arc	15.50		60.60							0	0	0	0	0	0	0	
	Height	13.20									0	0	0	0	0	0	0	
	Wall	8.50		119.00							0	0	0	0	0	0	0	
	Width	14.00									0	0	0	0	0	0	0	
				179.6	55.68	10,000 m³					0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
	<u>Required</u>										0	0	0	0	0	0	0	
		<u>Length</u>	<u>Dia.(m)</u>	<u>Arch (m)</u>							0	0	0	0	0	0	0	
	Class 1	41.8	12.5	11.59	75%						0	0	0	0	0	0	0	
	Class 2	8.4	12.5	11.59	15%						0	0	0	0	0	0	0	
	Class 3	3.9	12.5	11.59	7.0%						0	0	0	0	0	0	0	
	Class 4	1.4	12.5	11.59	2.5%						0	0	0	0	0	0	0	
	Class 5	0.3	12.5	11.59	0.5%						0	0	0	0	0	0	0	
		56			100%						0	0	0	0	0	0	0	
	Class 1										0	0	0	0	0	0	0	
	Rock bolts 2,5 m	1 un / m		42 un							0	0	0	0	0	0	0	
	Shotcrete 50 mm	22.59 m² / m		142 m²	15%						0	0	0	0	0	0	0	
	Wire mesh	22.59 m² / m		802 m²	85%						0	0	0	0	0	0	0	
	Class 2										0	0	0	0	0	0	0	
	Rock bolts 2,5 m	2.3 un / m		19 un							0	0	0	0	0	0	0	
	Shotcrete 50 mm	22.59 m² / m		28 m²	15%						0	0	0	0	0	0	0	
	Wire mesh	22.59 m² / m		160 m²	85%						0	0	0	0	0	0	0	
	Class 3										0	0	0	0	0	0	0	
	Rock bolts 3 m	2.9 un / m		11 un							0	0	0	0	0	0	0	
	Shotcrete 50 mm	22.59 m² / m		44 m²	50%						0	0	0	0	0	0	0	
	Wire mesh	22.59 m² / m		44 m²	50%						0	0	0	0	0	0	0	
	Class 4										0	0	0	0	0	0	0	
	Rock bolts 4 m	5.2 un / m		7 un							0	0	0	0	0	0	0	
	Shotcrete 50 mm	11.0 m² / m		5 m²	30%						0	0	0	0	0	0	0	
	Wire mesh	11.0 m² / m		11 m²	70%						0	0	0	0	0	0	0	
	Shotcrete 100 mm	11.6 m² / m		16 m²	100%						0	0	0	0	0	0	0	
	Reinf. Mesh	11.6 m² / m		16 m²	100%						0	0	0	0	0	0	0	
	Steel arch (W 100)	1.5 m c/c		1 un							0	0	0	0	0	0	0	
		32.5 m / arch		33 m							0	0	0	0	0	0	0	
	Class 5										0	0	0	0	0	0	0	
	Rock bolts 5 m	11.6 un / m		3 un							0	0	0	0	0	0	0	
	Shotcrete 50 mm	11.0 m² / m		1 m²	30%						0	0	0	0	0	0	0	
	Wire mesh	11.0 m² / m		2 m²	70%						0	0	0	0	0	0	0	
	Shotcrete 100 mm	11.6 m² / m		3 m²	100%						0	0	0	0	0	0	0	

Item : (3412)

WBS	DESCRIPTION	%	n	Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
						M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.75 \$		
	Reinf. Mesh	11.6	m ² / m	3	m ²	100%					0	0	0	0	0	0	0	
	Steel arch (W 150)	0.75	m c/c	0	un						0	0	0	0	0	0	0	
		32.5	m / arch	0	m						0	0	0	0	0	0	0	
	Supply		Lenght (m)								0	0	0	0	0	0	0	
	- Rock bolts 2,5 m	61	un	158	Losses	3%			60.00		0	0	3,780	0	0	0	3,780	
	- Rock bolts 3 m	11	un	33	Losses	3%			70.00		0	0	770	0	0	0	770	
	- Rock bolts 4 m	7	un	28	Losses	3%			80.00		0	0	560	0	0	0	560	
	- Rock bolts 5 m	3	un	15	Losses	3%			105.00		0	0	315	0	0	0	315	
		82		234							0	0	0	0	0	0	0	
	- Injection tubes	150	m roll			3%			110.00		0	0	220	0	0	0	220	
	- Oakum	130	bolts / box			3%			280.00		0	0	280	0	0	0	280	
	- Grease	154	bolts / box			3%			336.00		0	0	336	0	0	0	336	
											0	0	0	0	0	0	0	
	- Wire mesh	1,019	m ²			15%			4.60		0	0	5,391	0	0	0	5,391	
	- Reinf. Mesh	19	m ²			15%			5.60		0	0	123	0	0	0	123	
		1,038	m ²								0	0	0	0	0	0	0	
	- Spikes 1,1 m	1.25	m c/c	831	un	3%			4.50		0	0	3,852	0	0	0	3,852	
	- Wire			0.04	\$ / m ²				0.04		0	0	42	0	0	0	42	
											0	0	0	0	0	0	0	
	Shotcrete 50 mm	219	0.05	11							0	0	0	0	0	0	0	
	Shotcrete 100 mm	19	0.1	2							0	0	0	0	0	0	0	
				13							0	0	0	0	0	0	0	
	- Cement (40 kg Bags)	0.03	m ³ / bag		Losses	7.5%			10.00		0	0	4,620	0	0	0	4,620	
		33.33	bags / m ³	430	bags						0	0	0	0	0	0	0	
	- Sand	1.40	mt / m ³	0.11	h / mt				2.61	8.08	0.00	2.60	11.98	47	146	0	47	
											0	0	0	0	0	0	0	
	- Monoset (3% of cement)			17,204	kg	3%			3.40		0	0	1,754	0	0	0	1,754	
											0	0	0	0	0	0	0	
	- Steel arch (W 100)	19.0	kg / m	33	m				4.00		0	0	2,470	0	0	0	2,470	
	- Steel arch (W 150)	22.0	kg / m	0	m				5.00		0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
	Rock bolts Installation										0	0	0	0	0	0	0	
		234	m	14	m / sh						0	0	0	0	0	0	0	
		82	un	5	un / sh						0	0	0	0	0	0	0	
				0.5	h / un. including positioning						0	0	0	0	0	0	0	
				3.0	h / sh						0	0	0	0	0	0	0	
				51	h						0	0	0	0	0	0	0	
	1) Drilling with Jumbo										0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
	- Jumbo					90%	1			102.50			0	0	0	0	4,715	
	- Cat GEP 550 - 400KW	6.50		102.40						6.50	102.40		0	0	0	299	3,533	
	2) Install with 50t crane with basket												0	0	0	0	0	
											0	0	0	0	0	0	0	
	- M-P						3		24.00				0	0	0	0	0	
											3,672		0	0	0	0	3,672	
											0		0	0	0	0	0	
	- Crane - Rough terrain 50 t (L-Belt)	37.00		20.00		90%	1				37.00	20.00	0	0	1,702	690	2,392	
													0	0	0	0	0	
	- Impact tool								300.00		0		300	0	0	0	300	
	- Test rig								1,200.00		0		1,200	0	0	0	1,200	
	- Torque rench								280.00		0		280	0	0	0	280	
											0		0	0	0	0	0	
	3) Injection	40	bolts / sh	82	un						0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
	- M-P						4		24.00				2,880	0	0	0	2,880	
											0		0	0	0	0	0	

Item : (3412)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.75 \$		
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1					37.00	20.00	0	0	0	999	405	1,404	
	- Moyno pump	2.00		75%	1					2.00		0	0	0	46	0	46	
	- Cement (bags)	234 m		100%								0	0	0	0	0	0	
		766 ft	0.02269801 sf		38			10.00				0	0	380	0	0	380	
		2 in. Dia hole	17 cu ft									0	0	0	0	0	0	
		0.91 cu ft / bag	19 bags									0	0	0	0	0	0	
	- Intraplast "N"	0.4 kg / bag	8 kg	1%				3.00				0	0	24	0	0	24	
	- Miscellaneous							0.30				0	25	0	0	0	25	
	Wire mesh installation											0	0	0	0	0	0	
	Installation by Jumbo team											0	0	0	0	0	0	
	Production of	200 m² / sh	1,038 m²		5							0	0	0	0	0	0	
			10 h / sh		52							0	0	0	0	0	0	
												0	0	0	0	0	0	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1					37.00	20.00	0	0	0	1,739	705	2,444	
	- Jack leg	2.00		30%						2.00		0	0	0	32	0	32	
	- Miscellaneous materials	Spike drilling	914.1 m					1.00				0	914	0	0	0	914	
												0	0	0	0	0	0	
												0	0	0	0	0	0	
	Shotcreting											0	0	0	0	0	0	
	Production of	0.7 h / m³	9 h		13							0	0	0	0	0	0	
			7.5 h / sh Eff.		2							0	0	0	0	0	0	
			10 h / sh		20							0	0	0	0	0	0	
												0	0	0	0	0	0	
	- M-P				9			24.00				4,320	0	0	0	0	4,320	180
												0	0	0	0	0	0	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1					37.00	20.00	0	0	0	666	270	936	
	- Shotcrete pump	17.00		60%	1					17.00		0	0	0	204	0	204	
	- Hoses			25%	1			35.00				0	175	0	0	0	175	
	- Nozzle	66 m³ / un						275.00				0	0	0	0	0	0	
												0	0	0	0	0	0	
												0	0	0	0	0	0	
	Arches installation	33 m	33 m / un		1							0	0	0	0	0	0	
	Production of	2 un / sh			1							0	0	0	0	0	0	
			10 h / sh		10							0	0	0	0	0	0	
												0	0	0	0	0	0	
	- M-P				5			24.00				1,200	0	0	0	0	1,200	50
												0	0	0	0	0	0	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1					37.00	20.00	0	0	0	333	135	468	
	- Miscellaneous materials							200.00				0	200	0	0	0	200	
												0	0	0	0	0	0	
												0	0	0	0	0	0	
	Mucking	10,000 m³										0	0	0	0	0	0	
	1.5 Loose »»»»	15,000 m³										0	0	0	0	0	0	
		1,250 m³ / round										0	0	0	0	0	0	
	Production	140 m³ / h	8.93 h									0	0	0	0	0	0	
		12 rounds	107 h x 10/9 »»		119							0	0	0	0	0	0	
												0	0	0	0	0	0	
	- M-P				7			24.00				20,000	0	0	0	0	20,000	833
												0	0	0	0	0	0	
	- Cat 329DL Hydraulic Excavator	19.00	29.00	50%	1					19.00	29.00	0	0	0	1,140	1,305	2,445	
	- Cat 988H Wheel Loader	39.20	48.00	90%	1					39.20	48.00	0	0	0	4,194	3,852	8,046	
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1					38.25	28.00	0	0	0	4,093	2,247	6,340	
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	3					24.00	20.00	0	0	0	7,704	4,815	12,519	
												0	0	0	0	0	0	
	Disposal of excavated materials											0	0	0	0	0	0	

Item : (3412)

WBS	DESCRIPTION				UNIT PRICES						TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS			
					M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption							
%	n	Qty	Un.																		
										24.00 \$					0.75 \$						
-	Rock bolts 3 m	3.0 un / m	63 un	Losses	3%	65 un			70.00					0	0	4,550	0	0	4,550		
			189 m drilling											0	0	0	0	0	0		
-	Wire mesh	12.60 m ² / m	265 m ²		15%	304 m ²			4.60					0	0	1,398	0	0	1,398		
-	Spikes ,7 m	1.25 m c/c	212 un		3%	218 un			4.50					0	0	981	0	0	981		
-	Wire		0.04 \$ / m ²			265 m ²			0.04					0	0	11	0	0	11		
	Rock bolts Installation	(Operating on platform hooked on a crane)												0	0	0	0	0	0		
1)	Drilling	10 m / h	19 h											0	0	0	0	0	0		
			7.5 h / sh Eff.			3 sh								0	0	0	0	0	0		
			10 h / sh			30 h								0	0	0	0	0	0		
-	M-P					90 h		24.00						2,160	0	0	0	0	2,160		90
-	Jack leg	2.00			90%	1	27 h		2.00	0.00				0	0	0	54	0	54		
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	27 h		37.00	20.00				0	0	999	405	1,404			
-	Compressor XAHS 237 (500 cfm)	15.00	29.00		90%	1	27 h		15.00	29.00				0	0	405	587	992			
2)	Install with platform down the shaft													0	0	0	0	0	0		
		63 un	0.5 h / un											0	0	0	0	0	0		
			31.5 h											0	0	0	0	0	0		
			7.5 h / sh. Eff.			5 sh								0	0	0	0	0	0		
			10 h/sh			50 h								0	0	0	0	0	0		
-	M-P					200 h		24.00						4,800	0	0	0	0	4,800		200
-	Jack leg	2.00			90%	1	45 h		2.00	0.00				0	0	0	90	0	90		
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	45 h		37.00	20.00				0	0	1,665	675	2,340			
-	Impact tool					1 un			300.00					0	300	0	0	0	300		
-	Test rig					1 un			1,200.00					0	1,200	0	0	0	1,200		
-	Torque wrench					1 un			280.00					0	280	0	0	0	280		
	Wire mesh installation													0	0	0	0	0	0		
	Production of		175 m ² / sh			2 sh								0	0	0	0	0	0		
			10 h / sh			20 h								0	0	0	0	0	0		
-	M-P					100 h		24.00						2,400	0	0	0	0	2,400		100
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	18 h		37.00					0	0	0	666	0	666		
-	Jack leg	2.00			30%	1	6 h		2.00					0	0	0	12	0	12		
-	Compressor XAHS 237 (500 cfm)	15.00	29.00		90%	1	18 h		15.00					0	0	0	270	0	270		
-	Miscellaneous materials		Spike drilling 148.4 m			148 m			1.00					0	148	0	0	0	148		
	Wire mesh removing													0	0	0	0	0	0		
	Production of		200 m ² / sh			1 sh								0	0	0	0	0	0		
			10 h / sh			10 h								0	0	0	0	0	0		
-	M-P					50 h		24.00						1,200	0	0	0	0	1,200		50
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	9 h		37.00	20.00				0	0	0	333	135	468		
-	Boom truck 17 tons	13.65	18.00		90%	1	9 h		13.65	18.00				0	0	0	123	122	245		
	Services	Using outside installations for TBM					196 m							0	0	0	0	0	0		

Item : (3412)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption					
											24.00 \$					0.75 \$				
	Transformer Chamber Access tunnel			140																
	Transformer Chamber			56																
				196 m																
	Ventilation & Heating																			
	- M-P	3.0 h / m		587 h		24.00						14,089	0	0	0	0	0	14,089		587
	- Miscellaneous materials			196 m			10.00					0	1,957	0	0	0	0	1,957		
	Dewatering																			
	- M-P	2.0 h / m		391 h		24.00						9,393	0	0	0	0	0	9,393		391
	- Miscellaneous materials			196 m			10.00					0	1,957	0	0	0	0	1,957		
	Industrial Water Supply																			
	- M-P	3.5 h / m		685 h		24.00						16,437	0	0	0	0	0	16,437		685
	- Miscellaneous materials			196 m			10.00					0	1,957	0	0	0	0	1,957		
	Compressed Air																			
	- M-P	3.5 h / m		685 h		24.00						16,437	0	0	0	0	0	16,437		685
	- Miscellaneous materials			196 m			24.00					0	4,696	0	0	0	0	4,696		
	Electrical services																			
	- M-P	3.5 h / m		685 h		24.00						16,437	0	0	0	0	0	16,437		685
	- Miscellaneous materials			196 m			24.00					0	4,696	0	0	0	0	4,696		
												0	0	0	0	0	0	0		
												0	0	0	0	0	0	0		
3412	Transformer Chamber and Access			21,750								236,179	314,039	87,608	82,760	64,412	784,998	36.09	9,841	

Item : (3413)

WBS	DESCRIPTION				UNIT PRICES						TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
					M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
	%	n	Qty	Un.														
										24.00 \$					0.75 \$			
	Arches installation	582 m	21 m / un	28 un						0	0	0	0	0	0	0		
	Production of	2 un / sh		14 sh						0	0	0	0	0	0	0		
			10 h / sh	140 h						0	0	0	0	0	0	0		
	- M-P			700 h	24.00					16,800	0	0	0	0	0	16,800		
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	126 h				37.00	20.00	0	0	0	4,662	2,520	7,182	0		
	- Miscellaneous materials			28 un		200.00				0	5,600	0	0	0	5,600	0		
	Dewatering	Duration	6 months	1,202 m						0	0	0	0	0	0	0		
	Purchase of equipment and materials									0	0	0	0	0	0	0		
	- Pumps			1 ls		20,000				0	20,000	0	0	0	20,000	0		
	- Miscellaneous			1,202 m		15.00				0	18,023	0	0	0	18,023	0		
	- M-P		2.0 h / m	2,403 h	24.00					57,672	0	0	0	0	57,672	2,403		
	Outside Installation			30 h						0	0	0	0	0	0	0		
	- M-P			210 h	24.00					5,040	0	0	0	0	5,040	210		
	- Equipment			30 h				200.00		0	0	0	6,000	0	6,000	0		
	Pumping	26 weeks	6 d / w	156 days						0	0	0	0	0	0	0		
			20 h / day	3,120 h						0	0	0	0	0	0	0		
	- M-P			3,120 h	24.00					74,880	0	0	0	0	74,880	3,120		
	- Miscellaneous			26 weeks		110.00				0	2,860	0	0	0	2,860	0		
	Industrial Water Supply									0	0	0	0	0	0	0		
	Purchase of equipment and materials	Duration	6 months							0	0	0	0	0	0	0		
	- Pumps			2 un		20,000				0	40,000	0	0	0	40,000	0		
	- Miscellaneous			1,202 m		21.00				0	25,232	0	0	0	25,232	0		
	- M-P		2.0 h / m	2,403 h	24.00					57,672	0	0	0	0	57,672	2,403		
	Compressed Air	Duration	6 months							0	0	0	0	0	0	0		
	- M-P		3.5 h / m	4,205 h	24.00					100,926	0	0	0	0	100,926	4,205		
	- Miscellaneous materials			1,202 m		24.00				0	28,836	0	0	0	28,836	0		
	Ventilation & Heating									0	0	0	0	0	0	0		
	- M-P		3.0 h / m	3,605 h	24.00					86,508	0	0	0	0	86,508	3,605		
	- Miscellaneous materials			1,202 m		10.00				0	12,015	0	0	0	12,015	0		
	Electrical services									0	0	0	0	0	0	0		
	- M-P		3.5 h / m	4,205 h	24.00					100,926	0	0	0	0	100,926	4,205		
	- Miscellaneous materials			1,202 m		22.00				0	26,433	0	0	0	26,433	0		

Item : (3413)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.75 \$		
	Outside services are included in TBM										0	0	0	0	0	0	0	
	Outlet			4,690 m³							0	0	0	0	0	0	0	
	Wet			1,800 m³							0	0	0	0	0	0	0	
	Construction roads	(m)	(m² / m)	(m³)							0	0	0	0	0	0	0	
	Widening permanent road	5,000	5	25,000							0	0	0	0	0	0	0	
	From powerhouse access to outlet	500	11	5,500							0	0	0	0	0	0	0	
		5,500		30,500							0	0	0	0	0	0	0	
	Backfill from excavated materials										0	0	0	0	0	0	0	
	Foundation			30,500 m³							0	0	0	0	0	0	0	
	Production of	1,200 m³ / sh									0	0	0	0	0	0	0	
			10 h / s								0	0	0	0	0	0	0	
				25 sh							0	0	0	0	0	0	0	
				250 h							0	0	0	0	0	0	0	
	- M-P			4	1,000 h	24.00					24,000	0	0	0	0	0	24,000	1,000
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	225 h			38.25	28.00	0	0	0	8,606	4,725	13,331		
	- Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	45%	1	113 h			14.85	20.00	0	0	0	1,678	1,695	3,373		
	- Cat 329DL Hydraulic Excavator	19.00	29.00	25%	1	63 h			19.00	29.00	0	0	0	1,197	1,370	2,567		
	- Miscellaneous (culverts, signalisation, etc...)				5,500 m		2.00				0	11,000	0	0	0	11,000		
	Pavement	0.3	x	10	3 m³ / m						0	0	0	0	0	0	0	
	Production of	1,000 m³ / sh									0	0	0	0	0	0	0	
					17 sh						0	0	0	0	0	0	0	
					10 h / s						0	0	0	0	0	0	0	
	- M-P			10	1,650 h	24.00					39,600	0	0	0	0	0	39,600	1,650
	- Cat D6T LGP Track-Type Tractor	28.40	26.10	90%	1	149 h			28.40	26.10	0	0	0	4,232	2,917	7,149		
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	45%	5	371 h			24.00	20.00	0	0	0	8,904	5,565	14,469		
	- Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	25%	1	41 h			14.85	20.00	0	0	0	609	615	1,224		
	- Cat 14M Motorgrader	16.65	25.75	90%	1	149 h			16.65	25.75	0	0	0	2,481	2,878	5,359		
	- Cat 980H Wheel Loader	29.00	23.45	90%	1	149 h			29.00	23.45	0	0	0	4,321	2,621	6,942		
	Hauling distance from crusher	5.00 km									0	0	0	0	0	0	0	
	Loading	4									0	0	0	0	0	0	0	
	Trip up	9	35 km / h								0	0	0	0	0	0	0	
	Unloading	4									0	0	0	0	0	0	0	
	Back trip	9	35 km / h								0	0	0	0	0	0	0	
		26 min.									0	0	0	0	0	0	0	
	Efficiency :	85%	31 min. / trip								0	0	0	0	0	0	0	
			0.51 h / trip								0	0	0	0	0	0	0	
			9 h / sh								0	0	0	0	0	0	0	
			18 trips / sh								0	0	0	0	0	0	0	
	Cat 725 Articulated Dumper 25 T	12.0 m³									0	0	0	0	0	0	0	
		216 m³/mach/sh									0	0	0	0	0	0	0	
	Number of trucks per shift	5									0	0	0	0	0	0	0	
	- Pavement material	1.8 mt / m³	0.11 h / mt		29,700 mt	2.61	8.08	0.00	2.60	11.98	77,517	239,976	0	77,220	266,855	661,568	3,267	
	Rock Excavation - Dry				4,690 m³						0	0	0	0	0	0	0	
	Drilling										0	0	0	0	0	0	0	

Item : (3413)

WBS	DESCRIPTION		Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
					M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
%	n																	
										24.00 \$					0.75 \$			
	Drilling									0	0	0	0	0	0	0		
	Drilling grid ,9 x 1,2	0.90	1.20	1.08 m²						0	0	0	0	0	0	0		
	Drilling length			10,926 m						0	0	0	0	0	0	0		
	Production of			200 m / machine / sh	55 sh					0	0	0	0	0	0	0		
				3 machines	18 sh					0	0	0	0	0	0	0		
				10 h / s	183 h					0	0	0	0	0	0	0		
	- M-P				6	1,100 h	24.00				0	0	0	0	0	0		
										26,400	0	0	0	0	0	26,400	1,100	
	- Hydraulic Drilling Machine	19.40	15.00	90%	3	495 h			19.40	15.00	0	0	9,603	5,569	15,172			
	- Drilling materials					10,926 m	0.70				7,648	0	0	0	7,648			
											0	0	0	0	0			
											0	0	0	0	0			
											0	0	0	0	0			
	Blasting																	
	Average depth of holes			20 m														
	Number of holes			546 un														
	- Dynamite	1.5 kg / m³	2,700 kg	Losses 5%		2,835 kg		5.60			15,876	0	0	0	15,876			
	- Caps			Losses 5%		574 un		4.50			2,583	0	0	0	2,583			
											0	0	0	0	0			
											0	0	0	0	0			
	Production	8 min / hole	73 h			8 sh					0	0	0	0	0			
				10 h / s		80 h					0	0	0	0	0			
											0	0	0	0	0			
	- M-P				4	320 h	24.00				7,680	0	0	0	7,680	320		
											0	0	0	0	0			
	- Explosives Truck	5.00	15.00	90%	1	72 h			5.00	15.00	0	0	360	810	1,170			
	- Misc. Blasting materials					1,800 m³	0.10				180	0	0	0	180			
											0	0	0	0	0			
											0	0	0	0	0			
	Mucking																	
	Rock plug		1,800								0	0	0	0	0			
	1.5 loose »»»»		2,700								0	0	0	0	0			
	Drilling platform		10,000								0	0	0	0	0			
			12,700	m³							0	0	0	0	0			
	With clamshell and casted on each side																	
	Production		425 m³ / sh			30 sh					0	0	0	0	0			
				10 h / s		300 h					0	0	0	0	0			
											0	0	0	0	0			
	- M-P				4	1,200 h	24.00				28,800	0	0	0	28,800	1,200		
											0	0	0	0	0			
	- Crane 150T - Crawler	50.75	25.00	90%	1	270 h			50.75	25.00	0	0	13,703	5,063	18,766			
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	45%	1	135 h			38.25	28.00	0	0	5,164	2,835	7,999			
											0	0	0	0	0			
	- Miscellaneous					300 h		5.00			0	1,500	0	0	1,500			
											0	0	0	0	0			
											0	0	0	0	0			
											0	0	0	0	0			
3413	Powerhouse tailrace including Access and Outlet					65,657					1,474,770	1,476,471	447,337	448,741	553,891	4,401,210	67.03	61,486

Item : (3414)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.75 \$		
	- Bits 3"Ø	11 772	1 500		8 un		200.00					0	1 600	0	0	0	1 600	
	- Rod 18'	137 341	5 000		27 un		500.00					0	13 500	0	0	0	13 500	
	- Shank	137 341	10 000		14 un		300.00					0	4 200	0	0	0	4 200	
	- Misc. Materials	137 341			137 341 ft		0.05					0	6 867	0	0	0	6 867	
												0	0	0	0	0	0	
	Mucking	18 425	m³									0	0	0	0	0	0	
	1.5 Loose »»»»	27 637	m³									0	0	0	0	0	0	
		78	m³ / round									0	0	0	0	0	0	
	Transformer chamber section		200 m									0	0	0	0	0	0	
	Scooptram		Average hauling distance :		0.15 km							0	0	0	0	0	0	
												0	0	0	0	0	0	
	Loading	0.5										0	0	0	0	0	0	
	Going	1.0			8.0 km / h							0	0	0	0	0	0	
	Unloading	3.0										0	0	0	0	0	0	
	Return	1.0			8.8 km / h							0	0	0	0	0	0	
		5.5	min.									0	0	0	0	0	0	
	Efficacité :	85%			6.5 min. / trip							0	0	0	0	0	0	
					0.11 h / trip							0	0	0	0	0	0	
					3.00 m³ / trip							0	0	0	0	0	0	
					2.80 h / round							0	0	0	0	0	0	
												0	0	0	0	0	0	
												0	0	0	0	0	0	
	Left for Rock Support and services =				6.70 h							0	0	0	0	0	0	
	Trucks											0	0	0	0	0	0	
			Average hauling distance :		1.00 km							0	0	0	0	0	0	
												0	0	0	0	0	0	
	Loading	25.88			3 m³ / trip							0	0	0	0	0	0	
	Going	2			30 km / h							0	0	0	0	0	0	
	Unloading	3										0	0	0	0	0	0	
	Return	2			30 km / h							0	0	0	0	0	0	
		33	min.									0	0	0	0	0	0	
	Efficacité :	85%			39 min. / trip							0	0	0	0	0	0	
					0.64 h / trip							0	0	0	0	0	0	
					9 h / sh							0	0	0	0	0	0	
					14 trips / sh							0	0	0	0	0	0	
	Cat 725 Articulated Dumper 25 T				12 m³							0	0	0	0	0	0	
					168 m³ / truck-sh							0	0	0	0	0	0	
					Number of trucks :	2 (1+1)						0	0	0	0	0	0	
												0	0	0	0	0	0	
	Production	78 m³ / round			3.00 h / round							0	0	0	0	0	0	
		355 rounds			1 065 h x 10/9 »»							0	0	0	0	0	0	
						1 183 h						0	0	0	0	0	0	
	- M-P			5	5 917 h	24.00						142 000	0	0	0	0	142 000	5 917
												0	0	0	0	0	0	
	- R1300 G - Scooptram	20.35	16.00	90%	1	1 065 h			20.35	16.00		0	0	0	21 673	12 780	34 453	
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	1 065 h			38.25	28.00		0	0	0	40 736	22 365	63 101	
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	2	2 130 h			24.00	20.00		0	0	0	51 120	31 950	83 070	
												0	0	0	0	0	0	
	Outside section		865 m									0	0	0	0	0	0	
	Scooptram		Average hauling distance :		0.50 km							0	0	0	0	0	0	
												0	0	0	0	0	0	
	Loading	0.5										0	0	0	0	0	0	
	Going	4.0			8.0 km / h							0	0	0	0	0	0	
	Unloading	3.0										0	0	0	0	0	0	
	Return	3.0			8.8 km / h							0	0	0	0	0	0	
		10.5	min.									0	0	0	0	0	0	

Item : (3414)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
	Efficiency :	85%	12.4 min. / trip															
			0.21 h / trip															
			3.00 m³ / trip															
			5.34 h / round															
	Left for Rock Support and services =		4.16 h															
	Trucks																	
	Average hauling distance :		0.50 km															
	Loading	49.41	3 m³ / trip															
	Going	1	30 km / h															
	Unloading	3																
	Return	1	30 km / h															
		54 min.																
	Efficacité :	85%	64 min. / trip															
			1.07 h / trip															
			9 h / sh															
			9 trips / sh															
	Cat 725 Articulated Dumper 25 T		12 m³															
			108 m³ / truck-sh															
	Number of trucks :		2 (1+1)															
	Production	78 m³ / round	6.00 h / round															
		355 rounds	2 130 h x 10/9 »»															
							2 367 h											
	- M-P			5		11 833 h	24.00					284 000					284 000	11 833
	- R1300 G - Scooptram	20.35	16.00	90%	1	2 130 h		20.35	16.00				43 346	25 560			68 906	
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	2 130 h		38.25	28.00				81 473	44 730			126 203	
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	2	4 260 h		24.00	20.00				102 240	63 900			166 140	
	Rock Support																	
	D Shape	4,5 x 4,2	17.30 m³			1 065 m												
			<u>Area (m²)</u>															
	Arc	5.30	3.80															
	Height	4.20																
	Wall	3.00	13.50															
	Width	4.50																
			17.30															
			<u>Tunnel</u>															
	<u>Reshured</u>	<u>Length</u>	<u>Dia. (m)</u>	<u>Arch. (m)</u>														
	Class 1	798.8	12.5	11.59	75%													
	Class 2	159.8	12.5	11.59	15%													
	Class 3	74.6	12.5	11.59	7.0%													
	Class 4	26.6	12.5	11.59	2.5%													
	Class 5	5.3	12.5	11.59	0.5%													
		1 065																
	Class 1			<u>Qty</u>														
	Rock bolts 2,5 m	1 un / m		799 un														
	Shotcrete 50 mm	11.59 m² / m		1 389 m²	15%													
	Wire mesh	11.59 m² / m		7 869 m²	85%													
	Class 2																	
	Rock bolts 2,5 m	2.3 un / m		366 un														

Item : (3414)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS				
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption							
	Shotcrete 50 mm	11.59	m² / m	278	m²	15%							24.00 \$	0	0	0	0	0	0	0	0	
	Wire mesh	11.59	m² / m	1 574	m²	85%								0	0	0	0	0	0	0	0	
	Class 3													0	0	0	0	0	0	0	0	
	Rock bolts 3 m	2.9	un / m	216	un									0	0	0	0	0	0	0	0	
	Shotcrete 50 mm	11.59	m² / m	432	m²	50%								0	0	0	0	0	0	0	0	
	Wire mesh	11.59	m² / m	432	m²	50%								0	0	0	0	0	0	0	0	
	Class 4													0	0	0	0	0	0	0	0	
	Rock bolts 4 m	5.2	un / m	137	un									0	0	0	0	0	0	0	0	
	Shotcrete 50 mm	0.0	m² / m	0	m²	30%								0	0	0	0	0	0	0	0	
	Wire mesh	0.0	m² / m	0	m²	70%								0	0	0	0	0	0	0	0	
	Shotcrete 100 mm	11.6	m² / m	309	m²	100%								0	0	0	0	0	0	0	0	
	Reinf. Mesh	11.6	m² / m	309	m²	100%								0	0	0	0	0	0	0	0	
	Steel arch (W 100)	1.5	m c/c	18	un									0	0	0	0	0	0	0	0	
		11.3	m / arch	203	m									0	0	0	0	0	0	0	0	
	Class 5													0	0	0	0	0	0	0	0	
	Rock bolts 5 m	11.6	un / m	62	un									0	0	0	0	0	0	0	0	
	Shotcrete 50 mm	0.0	m² / m	0	m²	30%								0	0	0	0	0	0	0	0	
	Wire mesh	0.0	m² / m	0	m²	70%								0	0	0	0	0	0	0	0	
	Shotcrete 100 mm	11.6	m² / m	62	m²	100%								0	0	0	0	0	0	0	0	
	Reinf. Mesh	11.6	m² / m	62	m²	100%								0	0	0	0	0	0	0	0	
	Steel arch (W 150)	0.75	m c/c	7	un									0	0	0	0	0	0	0	0	
		11.3	m / arch	79	m									0	0	0	0	0	0	0	0	
	Supply													0	0	0	0	0	0	0	0	
			<u>Length (m)</u>											0	0	0	0	0	0	0	0	
	- Rock bolts 2,5 m	1 165	un	3 000	Losses	3%	1 200	un		60.00				0	0	72 000	0	0	0	0	72 000	
	- Rock bolts 3 m	216	un	666	Losses	3%	222	un		70.00				0	0	15 540	0	0	0	0	15 540	
	- Rock bolts 4 m	137	un	564	Losses	3%	141	un		80.00				0	0	11 280	0	0	0	0	11 280	
	- Rock bolts 5 m	62	un	320	Losses	3%	64	un		105.00				0	0	6 720	0	0	0	0	6 720	
		1 580		4 550										0	0	0	0	0	0	0	0	
	- Injection tubes	150	m roll			3%	31	rolls		110.00				0	0	3 410	0	0	0	0	3 410	
	- Oakum	130	bolts / box			3%	13	box		280.00				0	0	3 640	0	0	0	0	3 640	
	- Grease	154	bolts / box			3%	11	box		336.00				0	0	3 696	0	0	0	0	3 696	
														0	0	0	0	0	0	0	0	
	- Wire mesh	9 875	m²			15%	11 356	m²		4.60				0	0	52 238	0	0	0	0	52 238	
	- Reinf. Mesh	370	m²			15%	426	m²		5.60				0	0	2 386	0	0	0	0	2 386	
		10 245	m²											0	0	0	0	0	0	0	0	
	- Spikes 1,1 m	1.25	m c/c	8 196	un	3%	8 442	un		4.50				0	0	37 989	0	0	0	0	37 989	
	- Wire			0.04	\$ / m²		10 245	m²		0.04				0	0	410	0	0	0	0	410	
			<u>m²</u>		<u>m²</u>									0	0	0	0	0	0	0	0	
	Shotcrete 50 mm	2 098	0.05	105										0	0	0	0	0	0	0	0	
	Shotcrete 100 mm	370	0.1	37										0	0	0	0	0	0	0	0	
				142										0	0	0	0	0	0	0	0	
	- Cement (40 kg Bags)	0.03	m³ / bag		Losses	7.5%	5 086	bags		10.00				0	0	50 860	0	0	0	0	50 860	
		33.33	bags / m³	4 732	bags									0	0	0	0	0	0	0	0	
	- Sand	1.40	mt / m³	0.11	h / mt		199	mt	2.61	8.08	0.00	2.60	11.98	519	1 606	0	517	1 786	4 428	0	22	
														0	0	0	0	0	0	0	0	
	- Monoset (3% of cement)	189 265	kg			3%	5 678	kg		3.40				0	0	19 305	0	0	0	0	19 305	
														0	0	0	0	0	0	0	0	
	- Steel arch (W 100)	19.0	kg / m	203	m		3 865	kg		4.00				0	0	15 458	0	0	0	0	15 458	
	- Steel arch (W 150)	22.0	kg / m	79	m		1 740	kg		5.00				0	0	8 701	0	0	0	0	8 701	
														0	0	0	0	0	0	0	0	
	Rock bolts Installation													0	0	0	0	0	0	0	0	
	1 580 un	0.5	h	790	h									0	0	0	0	0	0	0	0	
				7.5	h / sh Eff.		106	sh						0	0	0	0	0	0	0	0	
				10	h / sh									0	0	0	0	0	0	0	0	
	1) Drilling													0	0	0	0	0	0	0	0	

Item : (3414)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS			
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption						
	- Nozzle	66 m³ / un		2 un			275.00					24.00 \$	0	550	0	0	0	0	550		
	Arches installation	283 m	11 m / un	25 un									0	0	0	0	0	0	0		
	Production of	2 un / sh		13 sh									0	0	0	0	0	0	0		
			10 h / sh	130 h									0	0	0	0	0	0	0		
	- M-P			5	650 h	24.00							15 600	0	0	0	0	0	15 600		650
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	117 h			37.00	20.00			0	0	0	4 329	1 755	6 084			
	- Miscellaneous materials				25 un		200.00						0	5 000	0	0	0	5 000			
	Outside services are included in TBM Power tunnel																				
	Dewatering	Duration	497 sh (days)	26 d / mth	19 mth								0	0	0	0	0	0	0		
	- M-P	1.0 h / m			1 065 h	24.00							25 560	0	0	0	0	0	25 560		1 065
	- Miscelaneous				1 065 m		10.00						0	10 650	0	0	0	10 650			
	Industrial Water Supply																				
	- M-P	2.0 h / m			2 130 h	24.00							51 120	0	0	0	0	0	51 120		2 130
	- Miscellaneous materials				1 065 m		10.00						0	10 650	0	0	0	10 650			
	Compressed Air																				
	- M-P	2.0 h / m			2 130 h	24.00							51 120	0	0	0	0	0	51 120		2 130
	- Miscelaneous materials				1 065 m		24.00						0	0	0	0	0	0	0		
	Ventilation & Heating																				
	- M-P	1.5 h / m			1 598 h	24.00							38 340	0	0	0	0	0	38 340		1 598
	- Miscelaneous materials				1 065 m		10.00						0	0	0	0	0	0	0		
	Electrical services																				
	- M-P	2.0 h / m			2 130 h	24.00							51 120	0	0	0	0	0	51 120		2 130
	- Miscelaneous materials				1 065 m		24.00						0	25 560	0	0	0	0	25 560		
													0	0	0	0	0	0	0		
													0	0	0	0	0	0	0		
3414	Cable and Escape Tunnel				18 425 m³								1 525 863	480 011	311 523	572 639	300 161	3 190 197		63 578	

Item : (3421-3426)

WBS	DESCRIPTION	UNIT PRICES									TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
	Reinforcing Steel										24.00 \$				0.72 \$			
	- Supply and Fabrication	60 kg / m³	15.88	h / mt	108 mt		381.05	217.41	988.17	69.37	39.85	41,154	23,481	106,722	7,492	3,098	181,947	1,715
	Installation																	
	- M-P	16.00 h / mt			1,728 h		24.00					41,472	0	0	0	0	41,472	1,728
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	20% 1	346 h				37.00	20.00		0	0	0	12,802	4,982	17,784	
	- Boom truck 17 tons	13.65	18.00	50% 1	864 h				13.65	18.00		0	0	0	11,794	11,197	22,991	
	Concrete transportation from the Batching Plan				1,836 m³													
	Average production	50 m³ / sh			37 sh							0	0	0	0	0	0	0
				10 h / sh	370 h							0	0	0	0	0	0	0
	- M-P				740 h		24.00					17,760	0	0	0	0	17,760	740
	- Readymix 8 m³	13.60	14.00	90% 1	333 h				13.60	14.00		0	0	0	4,529	3,357	7,886	
	Average hauling distance :		5.00 km									0	0	0	0	0	0	0
	Loading	10										0	0	0	0	0	0	0
	Going	10		30 km / h								0	0	0	0	0	0	0
	Unloading	15										0	0	0	0	0	0	0
	Return	9		35 km / h								0	0	0	0	0	0	0
		44 min.										0	0	0	0	0	0	0
	Efficacité :	85%		52 min. / trip								0	0	0	0	0	0	0
				0.86 h / trip								0	0	0	0	0	0	0
				9 h / sh								0	0	0	0	0	0	0
				11 trips / sh								0	0	0	0	0	0	0
	Readymix 8 m³			8 m³								0	0	0	0	0	0	0
				88 m³ / truck-sh								0	0	0	0	0	0	0
	Number of trucks :			1								0	0	0	0	0	0	0
												0	0	0	0	0	0	0
3425	Intake Tunnel				1,800							474,402	163,567	391,940	167,503	101,034	1,298,446	19,771

3426	Cable and Escape Tunnel																	
	Concrete Blocks				4,500 m²							0	0	0	0	0	0	0
	Supply											0	0	0	0	0	0	0
	- Concrete blocks		Losses	5%	4,725 m²		110.00					0	0	519,750	0	0	519,750	
	Production of	30 m² / sh			158 sh							0	0	0	0	0	0	
				10 h/sh	1,575 h							0	0	0	0	0	0	
	- M-P				11,025 h		24.00					264,600	0	0	0	0	264,600	
	- Boom truck 17 tons	13.65	18.00	90% 1	1,418 h				13.65	18.00		0	0	0	19,356	18,377	37,733	
	- Miscellaneous				1,575 h		6.00		12.00			0	9,450	0	18,900	0	28,350	
												0	0	0	0	0	0	

Item : (3421-3426)

WBS	DESCRIPTION	UNIT PRICES									TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$			
	Concrete Footing	Length	1,186								0	0	0	0	0	0	0	0	0
		Width	0.30								0	0	0	0	0	0	0	0	0
		Height	0.150								0	0	0	0	0	0	0	0	0
		Volume	53 m³								0	0	0	0	0	0	0	0	0
	- Concreting	5.50 h / m³				294 h	24.00				7,045	0	0	0	0	0	0	7,045	294
	- Construction materials					53 m³		74.00			0	3,949	0	0	0	0	0	3,949	
	- Construction equipment					53 m³			48.00	38.00	0	0	0	2,562	1,460	0	4,022		
	- Concrete	2.61 h / mt	Losses	2%		54 m³	62.54	17.48	155.35	24.23	22.05	3,377	944	8,389	1,308	2,176	16,194	141	
											0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		
	Pavement	Length	1,186								0	0	0	0	0	0	0	0	0
		Width	3.50								0	0	0	0	0	0	0	0	0
		Thickness	0.150								0	0	0	0	0	0	0	0	0
		Volume	623 m³								0	0	0	0	0	0	0	0	0
	Production	900 m³ / sh				1 sh					0	0	0	0	0	0	0	0	0
						10 h					0	0	0	0	0	0	0	0	0
	- M-P					40 h	24.00				960	0	0	0	0	0	960	40	
	- Cat 988H Wheel Loader	39.20	48.00	90%	1	9 h			39.20	48.00	0	0	0	353	311	664			
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	9 h			38.25	28.00	0	0	0	344	181	525			
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	1	9 h			24.00	20.00	0	0	0	216	130	346			
	- Crushed Stone	1.8 mt / m³	0.11 h / mt			1,121 mt	2.61	8.08	0.00	2.60	11.98	2,925	9,056	0	2,914	9,667	24,562	1,121	
											0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		
3426	Cable and Escape Tunnel										278,907	23,399	528,139	45,953	32,302	908,700		12,620	

Item : (3430-3470)

WBS	DESCRIPTION	%	n	Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
						M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.72 \$		
3430	Powerhouse crane installation			1	ls													
	Including testing																	
				10	sh						0	0	0	0	0	0		
				100	h						0	0	0	0	0	0		
				10	h / sh						0	0	0	0	0	0		
	- M-P			9	900 h	24.00					21,600	0	0	0	0	0	21,600	900
	- Crane - Rough terrain 120 t (L-Bel	45.00	23.00	75%	1	75 h			45.00	23.00	0	0	0	3,375	1,242	4,617		
	- Boom truck 17 tons	13.65	18.00	90%	1	90 h			13.65	18.00	0	0	0	1,229	1,166	2,395		
	- Tractor truck & Load Carrier - 65 T	11.50	15.00	60%	1	60 h			11.50	15.00	0	0	0	690	648	1,338		
	- Miscellaneous					100 h	9.00		8.00	12.00	0	900	0	800	864	2,564		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
3430	Powerhouse crane installation										21,600	900	0	6,094	3,920	32,514	900	
3440	Powerhouse overhead roofing			45	mt													
	Overhead Roofing																	
	Supply and install																	
	- Material			45	mt		4,000				0	0	180,000	0	0	180,000		
	Production of	8	mt / sh															
				6	sh													
				10	h / sh													
	- M-P			6	338 h	24.00					8,100	0	0	0	0	8,100	338	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	51 h			37.00	20.00	0	0	0	1,887	734	2,621		
	- Boom truck 17 tons	13.65	18.00	90%	1	51 h			13.65	18.00	0	0	0	696	661	1,357		
	- Miscellaneous					56 h	4.00		8.00	12.00	0	225	0	450	486	1,161		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
3440	Powerhouse overhead roofing			45							8,100	225	180,000	3,033	1,881	193,239	338	

Item : (3430-3470)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
3450	Structural Steel			160 mt														
	Steel Structure for service area		75							0	0	0	0	0	0	0	0	0
	Steel Structure for powerhouse area		85							0	0	0	0	0	0	0	0	0
			160 mt							0	0	0	0	0	0	0	0	0
	Supply and install			160 mt						0	0	0	0	0	0	0	0	0
	- Material			160 mt			3,775			0	0	604,000	0	0	0	604,000	0	0
	Production of	8.0 mt / sh		20 sh						0	0	0	0	0	0	0	0	0
			10 h / sh	200 h						0	0	0	0	0	0	0	0	0
	- M-P		8	1,600		24.00				38,400	0	0	0	0	0	38,400	0	1,600
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90% 1	180 h			37.00	20.00	0	0	0	6,660	2,592	9,252	0	0	0
	- Boom truck 17 tons	13.65	18.00	90% 1	180 h			13.65	18.00	0	0	0	2,457	2,333	4,790	0	0	0
	- Miscellaneous			200 h		4.00		8.00	12.00	0	800	0	1,600	1,728	4,128	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
3450	Structural Steel			160						38,400	800	604,000	10,717	6,653	660,570			1,600
3460	Steel lining - Penstocks and Manifold			460 mt														
	Supply and install			460 mt						0	0	0	0	0	0	0	0	0
	- Material			460 mt			3,000			0	0	1,380,000	0	0	1,380,000	0	0	0
	Production of	10.0 mt / sh		46 sh						0	0	0	0	0	0	0	0	0
			10 h / sh	460 h						0	0	0	0	0	0	0	0	0
	- M-P		##	6,440		24.00				154,560	0	0	0	0	154,560	0	0	6,440
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90% 1	414 h			37.00	20.00	0	0	0	15,318	5,962	21,280	0	0	0
	- Boom truck 17 tons	13.65	18.00	90% 1	414 h			13.65	18.00	0	0	0	5,651	5,365	11,016	0	0	0
	- Welding Machine - 400 A	2.00	6.00	90% 4	1,656 h			2.00	6.00	0	0	0	3,312	7,154	10,466	0	0	0
	- Miscellaneous			460 h		12.00		24.00	36.00	0	5,520	0	11,040	11,923	28,483	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
3460	Steel lining - Penstocks and Manifold			460						154,560	5,520	1,380,000	35,321	30,404	1,605,805			6,440

Item : 3510

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		

3500 Civil works related to Power tunnel

3510 Power tunnel (including Rock Support)				224,138 m³														
TBM - Operation																		
	<u>Dia.</u>	<u>Area</u>	<u>Lenth</u>	<u>Volume</u>														
	5.1	20.43	10,972	224,138														
-	Cutter cost			224,138 m³	14.30													
	Assembling	7 wks	60 h / wk	420 h														
-	Foreman			100% 1	420 h	24.00				10,080	0	0	0	0	0	10,080		420
-	Mechanic			100% 4	1,680 h	24.00				40,320	0	0	0	0	0	40,320		1,680
-	Electrician			100% 2	840 h	24.00				20,160	0	0	0	0	0	20,160		840
-	Iron worker			100% 4	1,680 h	24.00				40,320	0	0	0	0	0	40,320		1,680
-	Miner			100% 1	420 h	24.00				10,080	0	0	0	0	0	10,080		420
-	Truck Driver			100% 2	840 h	24.00				20,160	0	0	0	0	0	20,160		840
-	Crane op.			100% 1	420 h	24.00				10,080	0	0	0	0	0	10,080		420
-	Crane op.helper			100% 1	420 h	24.00				10,080	0	0	0	0	0	10,080		420
				16						0	0	0	0	0	0	0		
-	Crane 150T - Crawler	50.75	25.00	90% 1	378 h			50.75	25.00	0	0	0	19,184	6,804	25,988			
-	Welding Machine - 400 A	2.00	6.00	90% 1	378 h			2.00	6.00	0	0	0	756	1,633	2,389			
-	Tractor truck & Load Carrier - 65 T	11.50	15.00	45% 1	189 h			11.50	15.00	0	0	0	2,174	2,041	4,215			
	Boom truck 17 tons	13.65	18.00	90% 1	378 h			13.65	18.00	0	0	0	5,160	4,899	10,059			
	Labour									0	0	0	0	0	0	0		
	Average penetration	2.2 m / h								0	0	0	0	0	0	0		
	Duration									0	0	0	0	0	0	0		
	Distance	10,972 m			4,987 h					0	0	0	0	0	0	0		
	2 m strokes	5,486 strokes								0	0	0	0	0	0	0		
		5 min / stroke			458 h					0	0	0	0	0	0	0		
	1 Conveyer belt splicing /	300 m								0	0	0	0	0	0	0		
		15 h / splice			549 h					0	0	0	0	0	0	0		
					5,994					0	0	0	0	0	0	0		
	Efficiency	85%			7,052 h					0	0	0	0	0	0	0		
		9 h eff. / shift			784 sh					0	0	0	0	0	0	0		
		12 sh / week			65 weeks					0	0	0	0	0	0	0		
					15.0 months					0	0	0	0	0	0	0		
					14 m / sh					0	0	0	0	0	0	0		
	1) Day shift				392 sh					0	0	0	0	0	0	0		
					10 h / sh					0	0	0	0	0	0	0		
					3,920 h					0	0	0	0	0	0	0		
-	Foreman			100% 1	3,920 h	24.00				94,080	0	0	0	0	0	94,080		3,920
-	TBM Op.			100% 1	3,920 h	24.00				94,080	0	0	0	0	0	94,080		3,920
-	Rock Support			100% 6	23,520 h	24.00				564,480	0	0	0	0	0	564,480		23,520
-	Iron worker			100% 1	3,920 h	24.00				94,080	0	0	0	0	0	94,080		3,920
-	Electrician			100% 1	3,920 h	24.00				94,080	0	0	0	0	0	94,080		3,920
-	Services			100% 6	23,520 h	24.00				564,480	0	0	0	0	0	564,480		23,520
				16						0	0	0	0	0	0	0		

Item : 3510

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
2)	Night shift			392 sh						0	0	0	0	0	0		
				10 h / sh						0	0	0	0	0	0		
				3,920 h						0	0	0	0	0	0		
	- Foreman		100%	1	3,920 h	24.00				94,080	0	0	0	0	0	94,080	3,920
	- TBM Op.		100%	1	3,920 h	24.00				94,080	0	0	0	0	0	94,080	3,920
	- Rock Support		100%	6	23,520 h	24.00				564,480	0	0	0	0	0	564,480	23,520
	- Iron worker		100%	1	3,920 h	24.00				94,080	0	0	0	0	0	94,080	3,920
	- Electrician		100%	1	3,920 h	24.00				94,080	0	0	0	0	0	94,080	3,920
	- Services		100%	6	23,520 h	24.00				564,480	0	0	0	0	0	564,480	23,520
				16						0	0	0	0	0	0		
	Equipment									0	0	0	0	0	0		
	- TBM				4,987 h				80.00	0	0	0	398,982	0	0	398,982	
	- Conveyers				4,987 h				50.00	0	0	0	249,364	0	0	249,364	
	- Boom truck 17 tons	13.65		18.00	90%	1	3,528 h		13.65	18.00	0	0	0	48,157	45,723	93,880	
	- Miscellaneous				1 ls			1,000,000			0	1,000,000	0	0	0	1,000,000	
	Disposal of excavated materials										0	0	0	0	0	0	
		224,138 m³ solid									0	0	0	0	0	0	
	1.6 factor	358,621 loose									0	0	0	0	0	0	
					457 m³ / sh						0	0	0	0	0	0	
					51 m³ / h						0	0	0	0	0	0	
					0.85 m³ / min						0	0	0	0	0	0	
	Distance moyenne de transport :				3.00 km						0	0	0	0	0	0	
	chargement	15									0	0	0	0	0	0	
	aller	5			35 km / h						0	0	0	0	0	0	
	déchargement	3									0	0	0	0	0	0	
	retour	4			45 km / h						0	0	0	0	0	0	
		27			min.						0	0	0	0	0	0	
	Efficacité :	85%			32 min. / trip						0	0	0	0	0	0	
					0.53 h / trip						0	0	0	0	0	0	
					9 h / sh						0	0	0	0	0	0	
					17 trips / day						0	0	0	0	0	0	
	Cat 725 Articulated Dumper 25 T				12 m³						0	0	0	0	0	0	
					204 m³ / truck-sh						0	0	0	0	0	0	
	Number of trucks :				3						0	0	0	0	0	0	
					784 sh						0	0	0	0	0	0	
					10 h / sh						0	0	0	0	0	0	
	- M-P				5	39,200 h	24.00				940,800	0	0	0	0	940,800	39,200
	- Cat 725 Articulated Dumper 25 T	24.00		20.00	90%	3	21,168 h			24.00	20.00	0	508,032	304,819	812,851		
	- Cat D6T LGP Track-Type Tractor	28.40		26.10	90%	1	7,056 h			28.40	26.10	0	200,390	132,596	332,986		
											0	0	0	0	0		
											0	0	0	0	0		
											0	0	0	0	0		
											0	0	0	0	0		
											0	0	0	0	0		
	Dismantling TBM & Conveyers										0	0	0	0	0	0	
											0	0	0	0	0	0	
	- M-P				16	3,840 h	24.00				92,160	0	0	0	0	92,160	3,840
											0	0	0	0	0		
	- Crane 150T - Crawler	50.75		25.00	90%	1	216 h			50.75	25.00	0	10,962	3,888	14,850		
	- Tractor truck & Load Carrier - 65 T	11.50		15.00	90%	2	432 h			11.50	15.00	0	4,968	4,666	9,634		

Item : 3510

WBS	DESCRIPTION	UNIT PRICES										TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS								
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption												
											24.00 \$				0.72 \$												
	- Steel arch (W 150)	22.0 kg / m		24,138 kg							4.00				0	0	0	0	0	0	0	0	0				
	- Steel arch (W 100)	19 kg / m		52,117 kg							4.00				0	0	208,468	0	0	0	0	0	0	0	0	0	0
	Rock bolts Installation														0	0	0	0	0	0	0	0	0	0	0	0	0
				784 sh											0	0	0	0	0	0	0	0	0	0	0	0	0
				10 h / sh											0	0	0	0	0	0	0	0	0	0	0	0	0
	1) Drilling with TBM Crew														0	0	0	0	0	0	0	0	0	0	0	0	0
	2) Install with 50t crane with basket														0	0	0	0	0	0	0	0	0	0	0	0	0
	11,847 un			15 un / sh											0	0	0	0	0	0	0	0	0	0	0	0	0
	- M-P				3						24.00				564,480	0	0	0	0	0	0	0	0	0	0	0	23,520
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		1										0	0	0	261,072	0	0	0	0	0	0	0	0	261,072
	- Impact tool														0	0	0	0	0	0	0	0	0	0	0	0	0
	- Test rig														0	300	0	0	0	0	0	0	0	0	0	0	300
	- Torque rench														0	1,200	0	0	0	0	0	0	0	0	0	0	1,200
															0	280	0	0	0	0	0	0	0	0	0	0	280
	3) Injection	40 bolts / sh													0	0	0	0	0	0	0	0	0	0	0	0	0
				10 h / sh											0	0	0	0	0	0	0	0	0	0	0	0	0
															0	0	0	0	0	0	0	0	0	0	0	0	0
	- M-P				4						24.00				285,120	0	0	0	0	0	0	0	0	0	0	0	285,120
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		1										0	0	0	98,901	0	0	0	0	0	0	0	0	98,901
	- Moyno pump	2.00			1										0	0	0	4,455	0	0	0	0	0	0	0	0	4,455
															0	0	0	0	0	0	0	0	0	0	0	0	0
	- Cement (bags)	31,469 m													0	0	51,500	0	0	0	0	0	0	0	0	0	51,500
		103,217 ft	0.022698 sf												0	0	0	0	0	0	0	0	0	0	0	0	0
		2 in. Dia hole	2,343 cu ft												0	0	0	0	0	0	0	0	0	0	0	0	0
		0.91 bag / cu ft	2,575 bags												0	0	0	0	0	0	0	0	0	0	0	0	0
	- Intraplast "N"	0.4 kg / bag	1,030 kg		1%										0	0	3,120	0	0	0	0	0	0	0	0	0	3,120
	- Miscellaneous														0	3,554	0	0	0	0	0	0	0	0	0	0	3,554
	Wire mesh installation														0	0	0	0	0	0	0	0	0	0	0	0	0
	Installation by TBM crew														0	0	0	0	0	0	0	0	0	0	0	0	0
	Production of	200 m² / sh	5,538 m²												0	0	0	0	0	0	0	0	0	0	0	0	0
			10 h / sh												0	0	0	0	0	0	0	0	0	0	0	0	0
	Plus														0	0	0	0	0	0	0	0	0	0	0	0	0
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		1										0	0	0	9,213	0	0	0	0	0	0	0	0	9,213
	- Jack leg	2.00			1										0	0	0	166	0	0	0	0	0	0	0	0	166
	- Miscellaneous materials	Spike drilling	4873 m												0	4,873	0	0	0	0	0	0	0	0	0	0	4,873
															0	0	0	0	0	0	0	0	0	0	0	0	0
															0	0	0	0	0	0	0	0	0	0	0	0	0
	Shotcreting														0	0	0	0	0	0	0	0	0	0	0	0	0
	By the TBM crew														0	0	0	0	0	0	0	0	0	0	0	0	0
	Production of	0.7 h / m³	831 h												0	0	0	0	0	0	0	0	0	0	0	0	0
			7.5 h / sh Eff.												0	0	0	0	0	0	0	0	0	0	0	0	0
			10 h / sh Eff.												0	0	0	0	0	0	0	0	0	0	0	0	0
	Plus														0	0	0	0	0	0	0	0	0	0	0	0	0
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		1										0	0	0	36,963	0	0	0	0	0	0	0	0	36,963
	- Shotcrete pump	17.00			1										0	0	0	11,322	0	0	0	0	0	0	0	0	11,322
	- Hoses				1										0	9,730	0	0	0	0	0	0	0	0	0	0	9,730
	- Nozzle	66 m³ / un													0	4,950	0	0	0	0	0	0	0	0	0	0	4,950
															0	0	0	0	0	0	0	0	0	0	0	0	0
	Arches installation														0	0	0	0	0	0	0	0	0	0	0	0	0
															0	0	0	0	0	0	0	0	0	0	0	0	0

Item : 3520

WBS	DESCRIPTION	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power				Consumable materials	Permanent Materials
	10 h / s			20 h						24.00 \$				0.72 \$		
	- M-P		8	160 h		24.00				3,840	0	0	0	0	3,840	160
	- Cat 988H Wheel Loader	39.20	48.00	90%	1	18 h			39.20	48.00	0	0	706	622	1,328	
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	18 h			38.25	28.00	0	0	689	363	1,052	
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	1	18 h			24.00	20.00	0	0	432	259	691	
	Rock Support															
	D Shape	10 x 10	92.5 m³	738 m				68,265 m³								
			<u>Area (m²)</u>													
	Arc	11.59	17.50													
	Height	10.00														
	Wall	7.50														
	Width	10.00	75.00													
			92.5													
		<u>Tunnel</u>														
	<u>Required</u>	<u>Length</u>	<u>Dia.(m)</u>	<u>Arch (m)</u>												
	Class 1	553.5	11.59	75%												
	Class 2	110.7	11.59	15%												
	Class 3	51.7	11.59	7.0%												
	Class 4	18.5	11.59	2.5%												
	Class 5	3.7	11.59	0.5%												
		738		100%												
	Class 1		<u>Qty</u>													
	Rock bolts 2,5 m	1 un / m	554 un													
	Shotcrete 50 mm	20.59 m² / m	1,709 m²	15%												
	Wire mesh	20.59 m² / m	9,687 m²	85%												
	Class 2															
	Rock bolts 2,5 m	2.3 un / m	253 un													
	Shotcrete 50 mm	20.59 m² / m	342 m²	15%												
	Wire mesh	20.59 m² / m	1,937 m²	85%												
	Class 3															
	Rock bolts 3 m	2.9 un / m	150 un													
	Shotcrete 50 mm	20.59 m² / m	532 m²	50%												
	Wire mesh	20.59 m² / m	532 m²	50%												
	Class 4															
	Rock bolts 4 m	5.2 un / m	95 un													
	Shotcrete 50 mm	9.0 m² / m	50 m²	30%												
	Wire mesh	9.0 m² / m	116 m²	70%												
	Shotcrete 100 mm	11.6 m² / m	214 m²	100%												
	Reinf. Mesh	11.6 m² / m	214 m²	100%												
	Steel arch (W 100)	1.5 m c/c	12 un													
		26.6 m / arch	319 m													
	Class 5															
	Rock bolts 5 m	11.6 un / m	43 un													
	Shotcrete 50 mm	9.0 m² / m	10 m²	30%												
	Wire mesh	9.0 m² / m	23 m²	70%												
	Shotcrete 100 mm	11.6 m² / m	43 m²	100%												
	Reinf. Mesh	11.6 m² / m	43 m²	100%												
	Steel arch (W 150)	0.75 m c/c	5 un													

Item : 3520

WBS	DESCRIPTION	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS				
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power				Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption
	26.6 m / arch			133 m							24.00 \$				0.72 \$			
	Supply			Lenght (m)														
	- Rock bolts 2,5 m	807 un	2,018	Losses 3%	831 un		60.00					49,860	0	0	0	0	49,860	
	- Rock bolts 3 m	150 un	450	Losses 3%	155 un		70.00					10,850	0	0	0	0	10,850	
	- Rock bolts 4 m	95 un	380	Losses 3%	98 un		80.00					7,840	0	0	0	0	7,840	
	- Rock bolts 5 m	43 un	215	Losses 3%	44 un		105.00					4,620	0	0	0	0	4,620	
		1,095	3,063									0	0	0	0	0	0	
	- Injection tubes	150 m roll		3%	21 rolls		110.00					2,310	0	0	0	0	2,310	
	- Oakum	130 bolts / box		3%	9 box		280.00					2,520	0	0	0	0	2,520	
	- Grease	154 bolts / box		3%	7 box		336.00					2,352	0	0	0	0	2,352	
												0	0	0	0	0	0	
	- Wire mesh	12,296 m ²		15%	14,140 m ²		4.60					65,044	0	0	0	0	65,044	
	- Reinf. Mesh	257 m ²		15%	295 m ²		5.60					1,652	0	0	0	0	1,652	
		12,552 m ²										0	0	0	0	0	0	
	- Spikes 1,1 m	1.25 m c/c	10,042 un	3%	10,343 un		4.50					46,544	0	0	0	0	46,544	
	- Wire		0.04 \$ / m ²		12,552 m ²		0.04					502	0	0	0	0	502	
		m ²	m ³									0	0	0	0	0	0	
	Shotcrete 50 mm	2,643 0.05	132									0	0	0	0	0	0	
	Shotcrete 100 mm	257 0.1	26									0	0	0	0	0	0	
			158									0	0	0	0	0	0	
	- Cement (40 kg Bags)	0.03 m ³ / bag	5,260 bags	Losses 7.5%	5,655 bags		10.00					56,550	0	0	0	0	56,550	
		33.33 bags / m ³										0	0	0	0	0	0	
	- Sand	1.40 mt / m ³	0.11 h / mt		221 mt	2.61	8.08	0.00	2.60	11.98	577	1,785	0	574	1,906	4,842	24	
												0	0	0	0	0	0	
	- Monoset (3% of cement)	210,414 kg		3%	6,312 kg		3.40					21,461	0	0	0	0	21,461	
												0	0	0	0	0	0	
	- Steel arch (W 100)	19.0 kg / m	319 m		6,063 kg		4.00					24,250	0	0	0	0	24,250	
	- Steel arch (W 150)	22.0 kg / m	133 m		2,925 kg		5.00					14,625	0	0	0	0	14,625	
	Rock bolts Installation				223 sh							0	0	0	0	0	0	
		3,063 m	14 m / sh									0	0	0	0	0	0	
		1,095 un	5 un / sh									0	0	0	0	0	0	
			0.5 h / un. including positioning									0	0	0	0	0	0	
			3 h / sh		669 h							0	0	0	0	0	0	
	1) Drilling with Jumbo											0	0	0	0	0	0	
												0	0	0	0	0	0	
	- Jumbo			90%	602 h				102.50			0	0	0	0	0	61,705	
	- Cat GEP 550 - 400KW	6.50	102.40		602 h			6.50	102.40			0	0	3,913	44,384	48,297	0	
	2) Install with 50t crane with basket	1,095 un										0	0	0	0	0	0	
			5 un / sh									0	0	0	0	0	0	
			0.5 h / un incl. Positionning									0	0	0	0	0	0	
			2.5 h / sh		548 h							0	0	0	0	0	0	
	- M-P			3	1,643 h	24.00						39,420	0	0	0	0	39,420	1,643
												0	0	0	0	0	0	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	493 h			37.00	20.00			0	0	0	18,241	7,099	25,340	
												0	0	0	0	0	0	
	- Impact tool				1 un		300.00					0	300	0	0	0	300	
	- Test rig				1 un		1,200.00					0	1,200	0	0	0	1,200	
	- Torque wrench				1 un		280.00					0	280	0	0	0	280	
												0	0	0	0	0	0	
	3) Injection	40 bolts / sh			28 sh							0	0	0	0	0	0	
			10 h / sh		280 h							0	0	0	0	0	0	

Item : 3520

WBS	DESCRIPTION		%	n	Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
							M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$				
- M-P				4	1,120 h		24.00					0	0	0	0	0	0	0	0	1,120
- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	252 h				37.00	20.00		0	0	0	9,324	3,629			12,953	
- Moyno pump	2.00		75%	1	210 h				2.00			0	0	0	420	0			420	
- Cement (bags)	3,063 m		100%		502 bags			10.00				0	0	5,020	0	0			5,020	
	10,045 ft	0.022698 sf										0	0	0	0	0			0	
	2 in. Dia hole	228 cu ft										0	0	0	0	0			0	
	0.91 cu ft / bag	251 bags										0	0	0	0	0			0	
- Intraplast "N"	0.4 kg / bag	100 kg	1%		101 kg			3.00				0	0	303	0	0			303	
- Miscellaneous					1,095 un		0.30					0	329	0	0	0			329	
												0	0	0	0	0			0	
Wire mesh installation												0	0	0	0	0			0	
Installation by Jumbo team												0	0	0	0	0			0	
Production of	200 m² / sh	12,552 m²			63 sh							0	0	0	0	0			0	
		10 h / sh			628 h							0	0	0	0	0			0	
Plus												0	0	0	0	0			0	
- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	565 h				37.00	20.00		0	0	0	20,905	8,136			29,041	
- Jack leg	2.00		30%		188 h				2.00			0	0	0	376	0			376	
- Miscellaneous materials	Spike drilling	11,046 m			11,046 m		1.00					0	11,046	0	0	0			11,046	
												0	0	0	0	0			0	
												0	0	0	0	0			0	
Shotcreting					158 m³							0	0	0	0	0			0	
Production of	0.7 h / m³	110 h										0	0	0	0	0			0	
		7.5 h / sh Eff.			15 sh							0	0	0	0	0			0	
		10 h / sh			150 h							0	0	0	0	0			0	
- M-P				9	1,350 h		24.00					32,400	0	0	0	0			32,400	1,350
												0	0	0	0	0			0	
- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	135 h				37.00	20.00		0	0	0	4,995	1,944			6,939	
- Shotcrete pump	17.00		60%	1	90 h				17.00			0	0	0	1,530	0			1,530	
- Hoses			25%	1	38 h		35.00					0	1,330	0	0	0			1,330	
- Nozzle	66 m³ / un				2 un		275.00					0	550	0	0	0			550	
												0	0	0	0	0			0	
Arches installation	452 m	27 m / un			17 un							0	0	0	0	0			0	
Production of	2 un / sh				9 sh							0	0	0	0	0			0	
		10 h / sh			90 h							0	0	0	0	0			0	
- M-P				5	450 h		24.00					10,800	0	0	0	0			10,800	450
												0	0	0	0	0			0	
- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	81 h				37.00	20.00		0	0	0	2,997	1,620			4,617	
- Miscellaneous materials					17 un		200.00					0	3,400	0	0	0			3,400	
												0	0	0	0	0			0	
Dewatering	Duration	24 months			738 m							0	0	0	0	0			0	
												0	0	0	0	0			0	
Purchase of equipment and materials												0	0	0	0	0			0	
- Pumps					1 ls		20,000					0	20,000	0	0	0			20,000	
- Miscellaneous					738 m		15.00					0	11,070	0	0	0			11,070	
												0	0	0	0	0			0	
- M-P	2.0 h / m				1,476 h		24.00					35,424	0	0	0	0			35,424	1,476
												0	0	0	0	0			0	

Item : 3520

WBS	DESCRIPTION	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS					
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power				Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption	
											24.00 \$					0.72 \$			
	Outside Installation			30 h							0	0	0	0	0	0	0		
	- M-P		7	210 h		24.00					0	0	0	0	0	0	0		210
	- Equipment			30 h					200.00		0	0	0	6,000	0	0	0	6,000	
	Pumping	104 weeks	6 d/w	624 days							0	0	0	0	0	0	0		
		20 h/day		12,480 h							0	0	0	0	0	0	0		
	- M-P		1	12,480 h		24.00					0	0	0	0	0	0	0	299,520	12,480
	- Miscellaneous			104 weeks			110.00				0	11,440	0	0	0	0	0	11,440	
	Industrial Water Supply										0	0	0	0	0	0	0		
	Purchase of equipment and materials	Duration	24 months								0	0	0	0	0	0	0		
	- Pumps			2 un			20,000				0	40,000	0	0	0	0	0	40,000	
	- Miscellaneous			738 m			21.00				0	15,498	0	0	0	0	0	15,498	
	- M-P	2.0 h/m		1,476 h		24.00					0	0	0	0	0	0	0	35,424	1,476
	Compressed Air	Duration	24 months								0	0	0	0	0	0	0		
	- M-P	3.5 h/m		2,583 h		24.00					0	0	0	0	0	0	0	61,992	2,583
	- Miscellaneous materials			738 m			24.00				0	17,712	0	0	0	0	0	17,712	
	Ventilation & Heating										0	0	0	0	0	0	0		
	- M-P	3.0 h/m		2,214 h		24.00					0	0	0	0	0	0	0	53,136	2,214
	- Miscellaneous materials			738 m			10.00				0	7,380	0	0	0	0	0	7,380	
	Electrical services										0	0	0	0	0	0	0		
	- M-P	3.5 h/m		2,583 h		24.00					0	0	0	0	0	0	0	61,992	2,583
	- Miscellaneous materials			738 m			22.00				0	16,236	0	0	0	0	0	16,236	
	Outside services are included in TBM Power tunnel										0	0	0	0	0	0	0		
3520	Power tunnel Access			68,265 m							1,015,455	1,084,886	316,303	248,717	196,146		2,861,507	41.92	42,311

Item : 3530

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		

3530 Intake excavation

3530 Intake excavation		15,000 m³															
Rock Excavation		15,000 m³															
Access road and working platform		5,000															
		20,000 m³															
Drilling																	
Drilling grid ,9 x 1,2		0.90	1.20	1.08	m²					0	0	0	0	0	0	0	0
Drilling length				18,519 m						0	0	0	0	0	0	0	0
Production of		200 m / machine / sh		93 sh						0	0	0	0	0	0	0	0
		3 machines		31 sh						0	0	0	0	0	0	0	0
		10 h / s		310 h						0	0	0	0	0	0	0	0
- M-P				6		1,860 h		24.00		44,640		0		0		44,640	
- Hydraulic Drilling Machine		19.40	15.00	90%	3	837 h				0		0		16,238		25,278	
- Drilling materials						18,519 m		0.70		0		12,963		0		12,963	
Blasting																	
Average depth of holes		8 m															
Number of holes		2,315 un															
- Dynamite		1 kg / m³	15,000 m³	Losses	5%	15,750 kg		5.60		0		88,200		0		88,200	
- Caps				Losses	5%	2,431 un		4.50		0		10,940		0		10,940	
- M-P						4		1,240 h		24.00		29,760		0		29,760	
- Explosives Truck		5.00	15.00	90%	1	279 h				5.00		15.00		1,395		3,013	
- Misc. Blasting materials						15,000 m³		0.10		0		1,500		0		1,500	
Mucking																	
Production of		484 m³ / sh															
1.5 loose >>>>		726 m³ / sh				31 sh											
						10 h / s											
						310 h											
- M-P						11		3,410 h		24.00		81,840		0		81,840	
- Cat D7R II LGP Track-Type Tractor		38.25	28.00	90%	1	279 h				38.25		28.00		10,672		5,625	
- Cat 345 Hydraulic Excavator		40.00	60.00	90%	1	279 h				40.00		60.00		11,160		12,053	
- Cat 740 Articulated Dumper 40 T		32.00	27.90	90%	2	558 h				32.00		27.90		17,856		11,209	
- Generator 5 kW (Tower light)		3.50	2.20	90%	2	558 h				3.50		2.20		1,953		884	
- Cat 329DL Hydraulic Excavator		19.00	29.00	90%	1	279 h				19.00		29.00		5,301		5,826	
Hauling distance		4.00 km															
Loading		4															
Trip up		10	25 km / h														
Unloading		4															
Back trip		7															

Item : 3530

WBS	DESCRIPTION	UNIT PRICES				TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS											
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h				Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption						
	25 min.										0	0	0	0	0	0	0	0	0					
	Efficiency : 85%										0	0	0	0	0	0	0	0	0	0				
	29 min. / trip										0	0	0	0	0	0	0	0	0	0				
	0.49 h / trip										0	0	0	0	0	0	0	0	0	0				
	9 h / sh										0	0	0	0	0	0	0	0	0	0				
	19 trips / sh										0	0	0	0	0	0	0	0	0	0				
	Cat 740 Articulated Dumper 40 T										0	0	0	0	0	0	0	0	0	0				
	21.0 m³										0	0	0	0	0	0	0	0	0	0				
	399 m³/mach/sh										0	0	0	0	0	0	0	0	0	0				
	Number of trucks per shift 2										0	0	0	0	0	0	0	0	0	0				
	Rock Support										0	0	0	0	0	0	0	0	0	0				
	Invert at 655										0	0	0	0	0	0	0	0	0	0				
	Hoist building at 670										0	0	0	0	0	0	0	0	0	0				
	Intake L H Area										0	0	0	0	0	0	0	0	0	0				
	2 sides 40 15 600										0	0	0	0	0	0	0	0	0	0				
	Face 7 8 57										0	0	0	0	0	0	0	0	0	0				
	Hoist platform 2 sides 40 10 400										0	0	0	0	0	0	0	0	0	0				
	Face 20 10 200										0	0	0	0	0	0	0	0	0	0				
	1,257 m²										0	0	0	0	0	0	0	0	0	0				
	Supply										0	0	0	0	0	0	0	0	0	0				
	- Rock bolts 6 m 30 m² / un 42 un Losses 3% 43 un 110.00										0	4,730	0	0	0	0	0	0	0	0	4,730			
	- Wire mesh 1,257 m² Lapping 15% 1,446 m² 4.60										0	6,652	0	0	0	0	0	0	0	0	6,652			
	- Spikes 0,7 m 1.56 m² / un 806 un 3% 830 un 4.50										0	3,735	0	0	0	0	0	0	0	0	3,735			
	- Wire 1,257 m² 0.04										0	50	0	0	0	0	0	0	0	0	50			
	Rock bolts drilling and Installation										0	0	0	0	0	0	0	0	0	0				
	Production of 100 m / sh 3 sh										0	0	0	0	0	0	0	0	0	0				
	6 m bolt 252 m 10 h / sh 30 h										0	0	0	0	0	0	0	0	0	0				
	- M-P 6 180 h 24.00										4,320	0	0	0	0	0	0	0	0	0	4,320			180
	- Crane - Rough terrain 50 t (L-Belt) 37.00 20.00 90% 1 27 h 37.00 20.00										0	0	0	999	389	1,388								
	- Fork lift 15 T 13.00 9.00 90% 1 27 h 13.00 9.00										0	0	0	351	175	526								
	- Boom truck 17 tons 13.65 18.00 90% 1 27 h 13.65 18.00										0	0	0	369	350	719								
	- Drilling rig (on fork lift) 90% 1 27 h 0.00 0.00										0	0	0	0	0	0								
	Wire mesh Installation										0	0	0	0	0	0	0	0	0	0				
	Production of 100 m² / sh 13 sh										0	0	0	0	0	0	0	0	0	0				
	10 h / sh 130 h										0	0	0	0	0	0	0	0	0	0				
	- M-P 5 650 h 24.00										15,600	0	0	0	0	0	0	0	0	0	15,600			650
	- Crane - Rough terrain 50 t (L-Belt) 37.00 20.00 90% 1 117 h 37.00 20.00										0	0	0	4,329	1,685	6,014								
	- Jack leg 2.00 30% 1 39 h 2.00 0.00										0	0	0	78	0	78								
	- Fork lift 15 T 13.00 9.00 90% 1 117 h 13.00 9.00										0	0	0	1,521	758	2,279								
	- Misc. Drilling materials 806 un 0.7 m 564 m 1.00										0	564	0	0	0	564								
	Wire mesh removing										0	0	0	0	0	0	0	0	0	0				
	(For intake concrete structure)										0	0	0	0	0	0	0	0	0	0				
	L H Area m²										0	0	0	0	0	0	0	0	0	0				
	45 60 2,700										0	0	0	0	0	0	0	0	0	0				
	Production of 600 m² / sh 5 sh										0	0	0	0	0	0	0	0	0	0				
	10 h / sh 50 h										0	0	0	0	0	0	0	0	0	0				
	- M-P 5 250 h 24.00										6,000	0	0	0	0	0	0	0	0	0	6,000			250
	- Crane - Rough terrain 50 t (L-Belt) 37.00 20.00 90% 1 45 h 37.00 20.00										0	0	0	0	0	0	0	0	0	0	0			
											0	0	0	1,665	648	2,313								

Item : 3530

WBS	DESCRIPTION			UNIT PRICES							TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
- Boom truck 17 tons	13.65 18.00	90%	1	45 h				13.65	18.00	0	0	0	614	583	1,197			
Dewatering	Duration 2 months									0	0	0	0	0	0			
Purchase of equipment and materials										0	0	0	0	0	0			
- Pumps				1 ls			20,000			0	20,000	0	0	0	20,000			
- Miscellaneous				1,000 m			15.00			0	15,000	0	0	0	15,000			
Installation				30 h						0	0	0	0	0	0			
- M-P			7	210 h	24.00					5,040	0	0	0	0	5,040		210	
- Equipment				30 h				200.00		0	0	0	6,000	0	6,000			
Pumping	9 weeks 6 d / w			54 days						0	0	0	0	0	0			
	20 h / day			1,080 h						0	0	0	0	0	0			
- M-P			1	1,080 h	24.00					25,920	0	0	0	0	25,920		1,080	
- Miscellaneous				9 weeks			110.00			0	990	0	0	0	990			
										0	0	0	0	0	0			
										0	0	0	0	0	0			
										0	0	0	0	0	0			
										0	0	0	0	0	0			
										0	0	0	0	0	0			
										0	0	0	0	0	0			
3530 Intake excavation				15,000						213,120	165,324	0	80,501	52,238	511,183	34.08	8,880	

Item : 3540

WBS	DESCRIPTION	UNIT PRICES									TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$			
3540	Intake structure			760 m³															
	Concrete works																		
	Intake Structure			760 m²															
-	Concreting	5.00 h / m³		3,800 h	24.00						91,200						91,200		3,800
-	Construction materials			760 m³		80.00						60,800					60,800		
-	Construction equipment			760 m³				48.00	40.00					36,480	21,888		58,368		
-	Concrete supply	760	4.04 h / m³	2%	775 m²	96.85	5.10	186.47	35.08	13.03	75,057	3,953	144,512	27,186	7,271	257,979		3,133	
	Reinforcing Steel																		
-	Supply and Fabrication	60 kg / m³	17.27 h / mt		46 mt	414.40	323.08	987.76	79.99	44.86	18,897	14,733	45,042	3,648	1,473	83,793		787	
	Installation																		
-	M-P	16.00 h / mt			730 h	24.00					17,510					17,510		730	
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	20% 1	146 h				37.00	20.00				5,402	2,102	7,504			
-	Boom truck 17 tons	13.65	18.00	50% 1	365 h				13.65	18.00				4,982	4,730	9,712			
	Concrete transportation from the Batching Plan																		
	Average production	50 m³ / sh			775 m³														
			10 h / sh		16 sh														
					160 h														
-	M-P			3	480 h	24.00					11,520					11,520		480	
-	Readymix 8 m³	13.60	14.00	90% 2	288 h				13.60	14.00				3,917	2,903	6,820			
	Average hauling distance :		1.00 km																
	Loading	10																	
	Going	2	30 km / h																
	Unloading	15																	
	Return	2	35 km / h																
		29 min.																	
	Efficacité :	85%	34 min. / trip																
			0.57 h / trip																
			9 h / sh																
			16 trips / sh																
	Readymix 8 m³		8 m³																
			128 m³ / truck-sh																
	Number of trucks :		2 (1+1)																
3540	Intake structure			760							214,184	79,486	189,554	81,615	40,367	605,206	796.32	8,930	

Item : (3611-3614)

WBS	DESCRIPTION			Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n				Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
	Evacuation of excavated materials																		
	Production of	423	m³ / sh																
	1.5 loose »»»»	635	m³ / sh	8	sh														
	Plus 1 sh for overburden			1															
				9	sh														
	10 h / s			90	h														
	- M-P			8	720 h	24.00												720	
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	81 h			38.25	28.00				3 098	1 633		4 731		
	- Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	81 h			19.00	29.00				1 539	1 691		3 230		
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	4	324 h			32.00	27.90				10 368	6 509		16 877		
	- Generator 5 kW (Tower light)	3.50	2.20	90%	2	162 h			3.50	2.20				567	257		824		
	Hauling distance		12.00 km																
	Loading	4																	
	Trip up	21	35 km / h																
	Unloading	4																	
	Back trip	21	35 km / h																
	50 min.																		
	Efficiency :	85%	59 min. / trip																
			0.98 h / trip																
			9 h / sh																
			10 trips / sh																
	Cat 740 Articulated Dumper 40 T		21.0 m³																
			210 m³/mach/sh																
	Number of trucks per shift		4																
	Rock Support																		
	L	H	Area																
	2 sides	40	5	200															
	2 ends	10	10	100															
				300 m²															
	Supply																		
	- Rock bolts 6 m	30 m² / un	10 un	Losses	3%	10 un	110.00												
	- Wire mesh	200 m²		Lapping	15%	230 m²	4.60												
	- Spikes 0,7 m	1.56 m² / un	128 un		3%	132 un	4.50												
	- Wire		200 m²				0.04												
	Rock bolts drilling and Installation																		
	Production of		100 m / sh																
	6 m bolt	60 m		10 h / sh															
	- M-P			6	60 h	24.00												60	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	9 h			37.00	20.00				333	130		463		
	- Fork lift 15 T	13.00	9.00	90%	1	9 h			13.00	9.00				117	58		175		
	- Boom truck 17 tons	13.65	18.00	90%	1	9 h			13.65	18.00				123	117		240		
	- Drilling rig (on fork lift)			90%	1	9 h			0.00	0.00				0	0		0		

Item : (3611-3614)

WBS	DESCRIPTION			Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n				Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
	Wire mesh Installation																		
	Production of	100 m ² / sh		2 sh															
		10 h / sh		20 h															
-	M-P			5	100 h	24.00					24.00 \$					0.72 \$			
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	18 h			37.00	20.00	0	0	0	0	0	0	0	0	
-	Jack leg	2.00		30%	1	6 h			2.00	0.00	0	0	0	0	12	0	12	0	
-	Fork lift 15 T	13.00	9.00	90%	1	18 h			13.00	9.00	0	0	0	234	117	351	0	0	
-	Misc. Drilling materials	128 un	0.7 m		90 m		1.00				0	90	0	0	0	90	0	0	
	Diversion tunnel																		
	Drilling with Boomer E2 C																		
	D Shape	5 x 6	28.24 m ³		108 m														
			<u>Area (m²)</u>			3 050 m ³													
	Arc	5.80	4.49																
	Height	6.00																	
	Wall	4.75																	
	Width	5.00	23.75																
			28.24																
	Excavation																		
	Progression	4.66 m																	
	Number of rounds	24																	
		24																	
	Number of shifts	34	Prod. Factor 1.4																
	<u>Number of holes</u>		(m)	(Feet)															
	Production	23	55 mm dia.	2 777	9 107														
	Contour	24	55 mm dia.	2 897	9 503														
		47																	
	Cut	3	109 mm dia.	362	1 188														
		50																	
	Drilling depth	5.03 m	6 036	19 798															
	<u>Durations</u>		(hours)	24 rounds															
	Drilling	100 m / h	2.52	60 h															
	Blasting	1.15 min / hole	0.96	23 h															
	Scaling & W. mesh		2.00	48 h															
	Mucking	205 m ³ / h	0.64	15 h															
	<u>Drilling labour</u>																		
		H-H	Bolting	W. Mesh	Remaining														
		8	2 720	381	326	2 013													
			14%	12%															
	Drilling	2.5	24	60 h															
			9 h / sh	7 sh															
	8 men / sh	10 h / sh		537 h-h															
	Loading & Blasting	0.96	24	23 h															
			9 h / sh	3 sh															
	8 men / sh	10 h / sh		240 h-h															

Item : (3611-3614)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
	Disposal of excavated materials									24.00 \$				0.72 \$			
	Average hauling distance :	2.00 km								0	0	0	0	0	0	0	0
	Loading	8								0	0	0	0	0	0	0	0
	Going	4	30 km / h							0	0	0	0	0	0	0	0
	Unloading	3								0	0	0	0	0	0	0	0
	Return	4	30 km / h							0	0	0	0	0	0	0	0
		19 min.								0	0	0	0	0	0	0	0
	Efficacité :	85%	22 min. / trip							0	0	0	0	0	0	0	0
			0.37 h / trip							0	0	0	0	0	0	0	0
			9 h / sh							0	0	0	0	0	0	0	0
			25 trips / sh							0	0	0	0	0	0	0	0
	Cat 725 Articulated Dumper 25 T	12 m³								0	0	0	0	0	0	0	0
		300 m³ / truck-sh								0	0	0	0	0	0	0	0
	Number of trucks :	1								0	0	0	0	0	0	0	0
	Rolling Path									0	0	0	0	0	0	0	0
	Length	108								0	0	0	0	0	0	0	0
	Width	8.00								0	0	0	0	0	0	0	0
	Thickness	0.30								0	0	0	0	0	0	0	0
	Volume	259								0	0	0	0	0	0	0	0
	Production	800 m³ / sh		1 sh						0	0	0	0	0	0	0	0
			10 h / s	10 h						0	0	0	0	0	0	0	0
	- M-P			8	80 h	24.00				1 920	0	0	0	0	0	1 920	80
	- Cat 988H Wheel Loader	39.20	48.00	90%	1	9 h		39.20	48.00	0	0	0	353	311	664		
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	9 h		38.25	28.00	0	0	0	344	181	525		
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	1	9 h		24.00	20.00	0	0	0	216	130	346		
	Rock Support																
	D Shape	5 x 6	28.24 m³		108 m		3 050 m³										
			<u>Area (m²)</u>														
	Arc	5.80	4.49														
	Height	6.00															
	Wall	4.75	23.75														
	Width	5.00	28.24														
			<u>Tunnel</u>														
	<u>Required</u>	<u>Length</u>	<u>Arch (m)</u>							0	0	0	0	0	0	0	0
	Class 1	81.0	5.80	75%						0	0	0	0	0	0	0	0
	Class 2	16.2	5.80	15%						0	0	0	0	0	0	0	0
	Class 3	7.6	5.80	7.0%						0	0	0	0	0	0	0	0
	Class 4	2.7	5.80	2.5%						0	0	0	0	0	0	0	0
	Class 5	0.5	5.80	0.5%						0	0	0	0	0	0	0	0
	Class 1	108		100%						0	0	0	0	0	0	0	0
			<u>Qty</u>							0	0	0	0	0	0	0	0
	Rock bolts 2,5 m	1 un / m	81 un							0	0	0	0	0	0	0	0
	Shotcrete 50 mm	9.30 m² / m	113 m²	15%						0	0	0	0	0	0	0	0
	Wire mesh	9.30 m² / m	640 m²	85%						0	0	0	0	0	0	0	0

Item : (3611-3614)

WBS	DESCRIPTION			Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n				Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
	Class 2											24.00 \$					0.72 \$		
	Rock bolts 2,5 m	1.1 un / m		19 un								0	0	0	0	0	0	0	
	Shotcrete 50 mm	9.30 m² / m		23 m²								0	0	0	0	0	0	0	
	Wire mesh	9.30 m² / m		128 m²								0	0	0	0	0	0	0	
	Class 3											0	0	0	0	0	0	0	
	Rock bolts 3 m	1.5 un / m		11 un								0	0	0	0	0	0	0	
	Shotcrete 50 mm	9.30 m² / m		35 m²								0	0	0	0	0	0	0	
	Wire mesh	9.30 m² / m		35 m²								0	0	0	0	0	0	0	
	Class 4											0	0	0	0	0	0	0	
	Rock bolts 4 m	2.6 un / m		7 un								0	0	0	0	0	0	0	
	Shotcrete 50 mm	3.5 m² / m		3 m²								0	0	0	0	0	0	0	
	Wire mesh	3.5 m² / m		7 m²								0	0	0	0	0	0	0	
	Shotcrete 100 mm	5.8 m² / m		16 m²								0	0	0	0	0	0	0	
	Reinf. Mesh	5.8 m² / m		16 m²								0	0	0	0	0	0	0	
	Steel arch (W 100)	1.5 m c/c		2 un								0	0	0	0	0	0	0	
		15.3 m / arch		31 m								0	0	0	0	0	0	0	
	Class 5											0	0	0	0	0	0	0	
	Rock bolts 5 m	5.8 un / m		3 un								0	0	0	0	0	0	0	
	Shotcrete 50 mm	3.5 m² / m		1 m²								0	0	0	0	0	0	0	
	Wire mesh	3.5 m² / m		1 m²								0	0	0	0	0	0	0	
	Shotcrete 100 mm	5.8 m² / m		3 m²								0	0	0	0	0	0	0	
	Reinf. Mesh	5.8 m² / m		3 m²								0	0	0	0	0	0	0	
	Steel arch (W 150)	0.75 m c/c		1 un								0	0	0	0	0	0	0	
		15.3 m / arch		15 m								0	0	0	0	0	0	0	
	Supply		<u>Lenqht (m)</u>									0	0	0	0	0	0	0	
-	Rock bolts 2,5 m	100 un	250	Losses	3%	103 un			60.00			0	0	6 180	0	0	6 180		
-	Rock bolts 3 m	11 un	33	Losses	3%	11 un			70.00			0	0	770	0	0	770		
-	Rock bolts 4 m	7 un	28	Losses	3%	7 un			80.00			0	0	560	0	0	560		
-	Rock bolts 5 m	3 un	15	Losses	3%	3 un			105.00			0	0	315	0	0	315		
		121	326									0	0	0	0	0	0		
-	Injection tubes	150 m roll			3%	2 rolls			110.00			0	0	220	0	0	220		
-	Oakum	130 bolts / box			3%	1 box			280.00			0	0	280	0	0	280		
-	Grease	154 bolts / box			3%	1 box			336.00			0	0	336	0	0	336		
												0	0	0	0	0	0		
-	Wire mesh	811 m²			15%	933 m²			4.60			0	0	4 292	0	0	4 292		
-	Reinf. Mesh	19 m²			15%	22 m²			5.60			0	0	123	0	0	123		
		830 m²										0	0	0	0	0	0		
-	Spikes 1,1 m	1.25 m c/c		664 un	3%	684 un			4.50			0	0	3 078	0	0	3 078		
-	Wire			0.04 \$ / m²		830 m²			0.04			0	0	33	0	0	33		
		m²	m²									0	0	0	0	0	0		
	Shotcrete 50 mm	174 0.05		9								0	0	0	0	0	0		
	Shotcrete 100 mm	19 0.1		2								0	0	0	0	0	0		
				11								0	0	0	0	0	0		
-	Cement (40 kg Bags)	0.03 m³ / bag		Losses	7.5%	379 bags			10.00			0	0	3 790	0	0	3 790		
		33.33 bags / m³		353 bags								0	0	0	0	0	0		
-	Sand	1.40 mt / m³		0.08 h / mt		15 mt			1.84	1.30	0.00	2.08	3.08	27	19	0	31	33	
												0	0	0	0	0	0		
-	Monoset (3% of cement)	14 116 kg			3%	423 kg			3.40			0	0	1 438	0	0	1 438		
												0	0	0	0	0	0		
-	Steel arch (W 100)	19.0 kg / m		31 m		581 kg			4.00			0	0	2 326	0	0	2 326		
-	Steel arch (W 150)	22.0 kg / m		15 m		337 kg			5.00			0	0	1 683	0	0	1 683		

Item : (3611-3614)

WBS	DESCRIPTION			Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS			
		%	n				Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption							
	Rock bolts Installation			34	sh							24.00 \$					0.72 \$					
	326 m																					
	121 un																					
		10 m / sh																				
		4 un / sh																				
		0.5 h / un. including positioning																				
		2 h / sh																				
	1) Drilling with Jumbo			68	h																	
	- Jumbo		90%	1	61	h			102.50													
	- Cat GEP 550 - 400KW	6.50		102.40		1			6.50	102.40												
	2) Install with 50t crane with basket																					
	121 un																					
		4 un / sh																				
		0.5 h / un incl. Positionning																				
		1.8 h / sh																				
	- M-P			3	182	h	24.00					4 356	0	0	0	0	0	0	0	0	4 356	182
	- Crane - Rough terrain 50 t (L-Belt)	37.00		20.00	90%	1			37.00	20.00					1 998	778					2 776	
	- Impact tool			1	un			300.00					300	0	0	0	0	0	0	0	300	
	- Test rig			1	un			1 200.00					1 200	0	0	0	0	0	0	0	1 200	
	- Torque wrench			1	un			280.00					280	0	0	0	0	0	0	0	280	
	3) Injection	40 bolts / sh			3	sh																
		10 h / sh																				
	- M-P			4	120	h	24.00					2 880	0	0	0	0	0	0	0	0	2 880	120
	- Crane - Rough terrain 50 t (L-Belt)	37.00		20.00	90%	1			37.00	20.00					999	389					1 388	
	- Moyno pump	2.00			75%	1			2.00						46	0					46	
	- Cement (bags)	326 m			100%				10.00						540	0					540	
		1 069 ft		0.022698 sf																		
		2 in. Dia hole		24 cu ft																		
		0.91 cu ft / bag		27 bags																		
	- Intraplast "N"	0.4 kg / bag		11 kg	1%				3.00						33	0					33	
	- Miscellaneous			121 un				0.30					36	0	0	0	0	0	0	0	36	
	Wire mesh installation																					
	Installation by Jumbo team																					
	Production of	200 m ² / sh		830 m ²																		
				10 h / sh																		
	Plus																					
	- Crane - Rough terrain 50 t (L-Belt)	37.00		20.00	90%	1			37.00	20.00					1 369	533					1 902	
	- Jack leg	2.00			30%				2.00						24	0					24	
	- Miscellaneous materials	Spike drilling		730 m				1.00					730	0	0	0	0	0	0	0	730	

Item : (3611-3614)

WBS	DESCRIPTION			Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n				Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption						
	Compressed Air	Duration	1 months									24.00 \$					0.72 \$				
	- M-P	3.5 h / m		378 h		24.00						0	0	0	0	0	0	0	0	0	378
	- Miscellaneous materials			108 m			24.00					0	2 592	0	0	0	0	0	0	0	2 592
	Ventilation & Heating											0	0	0	0	0	0	0	0	0	
	- M-P	3.0 h / m		324 h		24.00						0	0	0	0	0	0	0	0	0	324
	- Miscellaneous materials			108 m			10.00					0	1 080	0	0	0	0	0	0	0	1 080
	Electrical services											0	0	0	0	0	0	0	0	0	
	- M-P	3.5 h / m		378 h		24.00						0	0	0	0	0	0	0	0	0	378
	- Miscellaneous materials			108 m			22.00					0	2 376	0	0	0	0	0	0	0	2 376
												0	0	0	0	0	0	0	0	0	
												0	0	0	0	0	0	0	0	0	
3611	Dam 1 - Diversion Tunnel			7 125								131 168	152 619	25 997	46 612	32 473	388 869	54.58	5 465		

3614	Dam 4 - Diversion Tunnel			5 788 m³																	
	Overburden excavation																				
	Upstream Portal			770 m³								0	0	0	0	0	0	0	0	0	
				770 m³								0	0	0	0	0	0	0	0	0	
	Production of	700 m³ / sh	Say	2 sh								0	0	0	0	0	0	0	0	0	
		10 h / sh		20 h								0	0	0	0	0	0	0	0	0	
	- M-P			60 h		24.00						0	0	0	0	0	0	0	0	0	60
	- Cat 345 Hydraulic Excavator	40.00	60.00	90%	1				40.00	60.00		0	0	0	720	778	1 498				
	- Cat D6T LGP Track-Type Tractor	28.40	26.10	90%	1				28.40	26.10		0	0	0	511	338	849				
	Rock excavation																				
	Upstream Portal			1 310 m³																	
	Downstream Portal			2 200 m³																	
				3 510 m³																	
	Drilling											0	0	0	0	0	0	0	0	0	
	Drilling grid, 9 x 1,2	0.90	1.20	1.08 m²								0	0	0	0	0	0	0	0	0	
	Drilling length			3 250 m								0	0	0	0	0	0	0	0	0	
	Production of	200 m / machine / sh		16 sh								0	0	0	0	0	0	0	0	0	
		2 machines		8 sh								0	0	0	0	0	0	0	0	0	
		10 h / s		80 h								0	0	0	0	0	0	0	0	0	
	- M-P			400 h		24.00						0	0	0	0	0	0	0	0	0	400
												0	0	0	0	0	0	0	0	0	

Item : (3611-3614)

WBS	DESCRIPTION			Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n				Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption						
	Supply											24.00 \$					0.72 \$				
	- Rock bolts 6 m	30 m ² / un	20 un	Losses	3%	21 un		110.00				0	0	0	0	0	0	0		2 310	
	- Wire mesh	500 m ²		Lapping	15%	575 m ²		4.60				0	2 310	0	0	0	0	0		2 645	
	- Spikes 0,7 m	1.56 m ² / un	321 un		3%	331 un		4.50				0	1 490	0	0	0	0	0		1 490	
	- Wire		500 m ²					0.04				0	20	0	0	0	0	0		20	
	Rock bolts drilling and installation																				
	Production of		100 m / sh			2 sh						0	0	0	0	0	0	0		0	
	6 m bolt	120 m		10 h / sh		20 h						0	0	0	0	0	0	0		0	
	- M-P					6	120 h	24.00				2 880	0	0	0	0	0	0		2 880	120
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	18 h			37.00	20.00	0	0	0	666	259	925				
	- Fork lift 15 T	13.00	9.00		90%	1	18 h			13.00	9.00	0	0	0	234	117	351				
	- Boom truck 17 tons	13.65	18.00		90%	1	18 h			13.65	18.00	0	0	0	246	233	479				
	- Drilling rig (on fork lift)				90%	1	18 h			0.00	0.00	0	0	0	0	0	0				
	Wire mesh Installation																				
	Production of		100 m ² / sh			5 sh						0	0	0	0	0	0	0		0	
				10 h / sh		50 h						0	0	0	0	0	0	0		0	
	- M-P					5	250 h	24.00				6 000	0	0	0	0	0	0		6 000	250
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00		90%	1	45 h			37.00	20.00	0	0	0	1 665	648	2 313				
	- Jack leg	2.00			30%	1	15 h			2.00	0.00	0	0	0	30	0	30				
	- Fork lift 15 T	13.00	9.00		90%	1	45 h			13.00	9.00	0	0	0	585	292	877				
	- Misc. Drilling materials		321 un	0.7 m			225 m	1.00				0	225	0	0	0	225				
	Diversion tunnel																				
	Drilling with Boomer E2 C																				
	D Shape	8 x 7	51.20 m ³	98 m			5 018 m ³														
			<u>Area (m²)</u>																		
	Arc	5.80	11.20																		
	Height	7.00																			
	Wall	5.00	40.00																		
	Width	8.00																			
			51.20																		
	Excavation																				
	Progression	4.66 m																			
	Number of rounds	21																			
		21																			
	Number of shifts	29	Prod. Factor 1.4																		
	<u>Number of holes</u>		<u>(m)</u>	<u>(Feet)</u>																	
	Production	41	55 mm dia.	4 331		14 205															
	Contour	25	55 mm dia.	2 641		8 662															
		66																			
	Cut	3	109 mm dia.	317		1 039															
		69																			
	Drilling depth	5.03 m																			
			7 288	23 906																	

Item : (3611-3614)

WBS	DESCRIPTION	%	n	Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
							Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
	Tunnel										24.00 \$					0.72 \$			
	<u>Required</u>	<u>Length</u>	<u>Arch (m)</u>																
	Class 1	73.5	5.80	75%							0	0	0	0	0	0	0	0	
	Class 2	14.7	5.80	15%							0	0	0	0	0	0	0	0	
	Class 3	6.9	5.80	7.0%							0	0	0	0	0	0	0	0	
	Class 4	2.5	5.80	2.5%							0	0	0	0	0	0	0	0	
	Class 5	0.5	5.80	0.5%							0	0	0	0	0	0	0	0	
		98		100%							0	0	0	0	0	0	0	0	
	<u>Class 1</u>			<u>Qty</u>							0	0	0	0	0	0	0	0	
	Rock bolts 2,5 m	1 un / m	74 un								0	0	0	0	0	0	0	0	
	Shotcrete 50 mm	9.80 m ² / m	108 m ²	15%							0	0	0	0	0	0	0	0	
	Wire mesh	9.80 m ² / m	612 m ²	85%							0	0	0	0	0	0	0	0	
	<u>Class 2</u>										0	0	0	0	0	0	0	0	
	Rock bolts 2,5 m	1.1 un / m	17 un								0	0	0	0	0	0	0	0	
	Shotcrete 50 mm	9.80 m ² / m	22 m ²	15%							0	0	0	0	0	0	0	0	
	Wire mesh	9.80 m ² / m	122 m ²	85%							0	0	0	0	0	0	0	0	
	<u>Class 3</u>										0	0	0	0	0	0	0	0	
	Rock bolts 3 m	1.5 un / m	10 un								0	0	0	0	0	0	0	0	
	Shotcrete 50 mm	9.80 m ² / m	34 m ²	50%							0	0	0	0	0	0	0	0	
	Wire mesh	9.80 m ² / m	34 m ²	50%							0	0	0	0	0	0	0	0	
	<u>Class 4</u>										0	0	0	0	0	0	0	0	
	Rock bolts 4 m	2.6 un / m	6 un								0	0	0	0	0	0	0	0	
	Shotcrete 50 mm	4.0 m ² / m	3 m ²	30%							0	0	0	0	0	0	0	0	
	Wire mesh	4.0 m ² / m	7 m ²	70%							0	0	0	0	0	0	0	0	
	Shotcrete 100 mm	5.8 m ² / m	14 m ²	100%							0	0	0	0	0	0	0	0	
	Reinf. Mesh	5.8 m ² / m	14 m ²	100%							0	0	0	0	0	0	0	0	
	Steel arch (W 100)	1.5 m c/c	2 un								0	0	0	0	0	0	0	0	
		15.8 m / arch	32 m								0	0	0	0	0	0	0	0	
	<u>Class 5</u>										0	0	0	0	0	0	0	0	
	Rock bolts 5 m	5.8 un / m	3 un								0	0	0	0	0	0	0	0	
	Shotcrete 50 mm	4.0 m ² / m	1 m ²	30%							0	0	0	0	0	0	0	0	
	Wire mesh	4.0 m ² / m	1 m ²	70%							0	0	0	0	0	0	0	0	
	Shotcrete 100 mm	5.8 m ² / m	3 m ²	100%							0	0	0	0	0	0	0	0	
	Reinf. Mesh	5.8 m ² / m	3 m ²	100%							0	0	0	0	0	0	0	0	
	Steel arch (W 150)	0.75 m c/c	1 un								0	0	0	0	0	0	0	0	
		15.8 m / arch	16 m								0	0	0	0	0	0	0	0	
	<u>Supply</u>		<u>Length (m)</u>								0	0	0	0	0	0	0	0	
	- Rock bolts 2,5 m	91 un	228 Losses	3%	94 un			60.00			0	0	5 640	0	0	5 640	0	0	
	- Rock bolts 3 m	10 un	30 Losses	3%	10 un			70.00			0	0	700	0	0	700	0	0	
	- Rock bolts 4 m	6 un	24 Losses	3%	6 un			80.00			0	0	480	0	0	480	0	0	
	- Rock bolts 5 m	3 un	15 Losses	3%	3 un			105.00			0	0	315	0	0	315	0	0	
		110	297								0	0	0	0	0	0	0	0	
	- Injection tubes	150 m roll		3%	2 rolls			110.00			0	0	220	0	0	220	0	0	
	- Oakum	130 bolts / box		3%	1 box			280.00			0	0	280	0	0	280	0	0	
	- Grease	154 bolts / box		3%	1 box			336.00			0	0	336	0	0	336	0	0	
											0	0	0	0	0	0	0	0	
	- Wire mesh	777 m ²		15%	893 m ²			4.60			0	0	4 108	0	0	4 108	0	0	
	- Reinf. Mesh	17 m ²		15%	20 m ²			5.60			0	0	112	0	0	112	0	0	
		794 m ²									0	0	0	0	0	0	0	0	
	- Spikes 1,1 m	1.25 m c/c	635 un	3%	654 un			4.50			0	0	2 943	0	0	2 943	0	0	
	- Wire		0.04 \$ / m ²		794 m ²			0.04			0	0	32	0	0	32	0	0	
		<u>m²</u>	<u>m²</u>								0	0	0	0	0	0	0	0	

Item : (3621-3624-3625)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$				0.72 \$			
	Trip up	3		35 km / h						0	0	0	0	0	0	0	0
	Unloading	4								0	0	0	0	0	0	0	0
	Back trip	3		35 km / h						0	0	0	0	0	0	0	0
		14	min.							0	0	0	0	0	0	0	0
	Efficiency :	85%		16 min. / trip						0	0	0	0	0	0	0	0
				0.27 h / trip						0	0	0	0	0	0	0	0
				9 h / sh						0	0	0	0	0	0	0	0
				33 trips / sh						0	0	0	0	0	0	0	0
	Cat 725 Articulated Dumper 25 T	12.0	m³							0	0	0	0	0	0	0	0
		396	m³/mach/sh							0	0	0	0	0	0	0	0
	Number of trucks per shift	3								0	0	0	0	0	0	0	0
	- Pavement material	1.8 mt / m³		0.08 h / mt	10,800 mt		1.84	1.30	0.00	2.08	3.08	19,872	14,040	0	22,464	23,950	80,326
										0	0	0	0	0	0	0	0
	Overburden excavation				22,000 m²					0	0	0	0	0	0	0	0
	Production of	700 m² / sh / mach			31 sh					0	0	0	0	0	0	0	0
		2 machines			16 sh					0	0	0	0	0	0	0	0
		10 h / sh			155 h					0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0
	- M-P				8	1,240 h			24.00	29,760	0	0	0	0	0	0	29,760
										0	0	0	0	0	0	0	0
	- Cat 345 Hydraulic Excavator	40.00	60.00	90%	2	279				40.00	60.00	0	0	0	11,160	12,053	23,213
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	140				38.25	28.00	0	0	0	5,355	2,822	8,177
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	3	419				32.00	27.90	0	0	0	13,408	8,417	21,825
										0	0	0	0	0	0	0	0
	Evacuation of excavated materials									0	0	0	0	0	0	0	0
	Average hauling distance :	2.00 km								0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0
	Loading	4								0	0	0	0	0	0	0	0
	Going	4		30 km / h						0	0	0	0	0	0	0	0
	Unloading	3								0	0	0	0	0	0	0	0
	Return	3		35 km / h						0	0	0	0	0	0	0	0
		14	min.							0	0	0	0	0	0	0	0
	Efficacité :	85%		16 min. / trip						0	0	0	0	0	0	0	0
				0.27 h / trip						0	0	0	0	0	0	0	0
				9 h / sh						0	0	0	0	0	0	0	0
				33 trips / sh						0	0	0	0	0	0	0	0
	Cat 740 Articulated Dumper 40 T	21	m³							0	0	0	0	0	0	0	0
		693	m³ / truck-sh							0	0	0	0	0	0	0	0
	Number of trucks :	3								0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0
	Dumped material				9,100 m²					0	0	0	0	0	0	0	0
	Loading on stockpile at	5 km								0	0	0	0	0	0	0	0
	Production of	1,200 m³ / sh			8 sh					0	0	0	0	0	0	0	0
				10 h / sh	80 h					0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0
	- M-P				8	640			24.00	15,360	0	0	0	0	0	0	15,360
										0	0	0	0	0	0	0	0
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	4	288 h				32.00	27.90	0	0	0	9,216	5,785	15,001
	- Cat 988H Wheel Loader	39.20	48.00	90%	1	72 h				39.20	48.00	0	0	0	2,822	2,488	5,310
	- Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	72 h				19.00	29.00	0	0	0	1,368	1,503	2,871
					6					0	0	0	0	0	0	0	0
	Average hauling distance :	5.00 km								0	0	0	0	0	0	0	0

Item : (3621-3624-3625)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	2	90 h				38.25	28.00	0	0	0	0	3,443	1,814	5,257
	- Cat 345 Hydraulic Excavator	40.00	60.00	90%	2	90 h				40.00	60.00	0	0	0	0	3,600	3,888	7,488
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	2	90 h				32.00	27.90	0	0	0	0	2,880	1,808	4,688
	- Generator 5 kW (Tower light)	3.50	2.20	90%	2	90 h				3.50	2.20	0	0	0	0	315	143	458
	- Cat 988H Wheel Loader	39.20	48.00	90%	1	45 h				39.20	48.00	0	0	0	0	1,764	1,555	3,319
					9													
	Hauling distance	2.00	km									0	0	0	0	0	0	0
	Loading	4										0	0	0	0	0	0	0
	Trip up	5	25 km / h									0	0	0	0	0	0	0
	Unloading	4										0	0	0	0	0	0	0
	Back trip	3	35 km / h									0	0	0	0	0	0	0
		16	min.									0	0	0	0	0	0	0
	Efficiency :	85%	19 min. / trip									0	0	0	0	0	0	0
			0.31 h / trip									0	0	0	0	0	0	0
			9 h / sh									0	0	0	0	0	0	0
			29 trips / sh									0	0	0	0	0	0	0
	Cat 740 Articulated Dumper 40	21.0	m³									0	0	0	0	0	0	0
		609	m³/mach/sh									0	0	0	0	0	0	0
	Number of trucks per shift	2										0	0	0	0	0	0	0
	From crusher					5,000 m³												
	Production of	1,000	m³ / sh			5 sh						0	0	0	0	0	0	0
		10	h / sh			50 h						0	0	0	0	0	0	0
												0	0	0	0	0	0	0
	- M-P				10	500	24.00					12,000	0	0	0	0	0	12,000
																		500
	- Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	45 h				19.00	29.00	0	0	0	0	855	940	1,795
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	2	90 h				32.00	27.90	0	0	0	0	2,880	1,808	4,688
	- Cat D8T LGP Track-Type Tractor	47.45	38.60	90%	1	45 h				47.45	38.60	0	0	0	0	2,135	1,251	3,386
	- Cat 988H Wheel Loader	39.20	48.00	90%	1	45 h				39.20	48.00	0	0	0	0	1,764	1,555	3,319
	- Cat 345 Hydraulic Excavator	40.00	60.00	90%	1	45 h				40.00	60.00	0	0	0	0	1,800	1,944	3,744
	- Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	90%	1	45 h				14.85	20.00	0	0	0	0	668	648	1,316
	Average hauling distance :	2.00	km			7						0	0	0	0	0	0	0
												0	0	0	0	0	0	0
	Loading	4										0	0	0	0	0	0	0
	Going	4	30 km / h									0	0	0	0	0	0	0
	Unloading	3										0	0	0	0	0	0	0
	Return	3	35 km / h									0	0	0	0	0	0	0
		14	min.									0	0	0	0	0	0	0
	Efficacité :	85%	16 min. / trip									0	0	0	0	0	0	0
			0.27 h / trip									0	0	0	0	0	0	0
			9 h / sh									0	0	0	0	0	0	0
			33 trips / sh									0	0	0	0	0	0	0
	Cat 740 Articulated Dumper 40 T	21	m³									0	0	0	0	0	0	0
		693	m³ / truck-sh									0	0	0	0	0	0	0
	Number of trucks :	2										0	0	0	0	0	0	0
	Geotextile					2,200 m²												
	Production of	550	m² / sh			4 sh						0	0	0	0	0	0	0
						40 h						0	0	0	0	0	0	0
												0	0	0	0	0	0	0

Item : (3621-3624-3625)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption	
										24.00 \$				0.72 \$					
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	81 h				19.00	29.00		1,539	1,691	3,230				
	Supply	4,700 m ²		15%		5,405 m ²			7.50				0	0	40,538				
	Geomembrane					3,000 m ²							0	0	0				
	Production of	400 m ² / sh				8 sh							0	0	0				
			10 h / sh			80 h							0	0	0				
-	M-P				8	640	24.00						15,360	0	0	15,360	640		
-	Boom truck 17 tons	13.65	18.00	90%	1	72 h				13.65	18.00		0	0	0	983	933		
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	72 h				19.00	29.00		0	0	1,368	1,503	2,871		
	Supply	3,000 m ²		15%		3,450 m ²			12.00				0	0	41,400	0	41,400		
	Pumping		30 weeks			180 days							0	0	0	0	0		
			6 d / w			3,600 h							0	0	0	0	0		
			20 h / day										0	0	0	0	0		
-	M-P				1	3,600 h	24.00						86,400	0	0	86,400	3,600		
-	Miscellaneous					1 ls		20,000					0	20,000	0	20,000	0		
3624	Dam 4 - Cofferdams												200,670	31,815	81,938	73,698	63,604	451,725	8,328

3625	Dam 5 - Cofferdams					6,450 m ³													
	Overburden excavation					32,000 m ²													
	Dumped material					(m ³)													
	Upstream cofferdam	3				400													
		3 A				600													
		3 B				250													
	Canal 4 cofferdam	3				3,100													
		3 A				1,400													
		3 B				700													
						6,450 m ²													
	Construction roads												0	0	0	0	0	0	0
	Widening primary road	(m)	(m ² / m)	(m ³)									0	0	0	0	0	0	0
		2,000	5	10,000									0	0	0	0	0	0	0
		2,000		10,000									0	0	0	0	0	0	0
	Backfill from excavated materials												0	0	0	0	0	0	0
	Foundation					10,000 m ³							0	0	0	0	0	0	0
	Production of	1,200 m ³ / sh				8 sh							0	0	0	0	0	0	0
			10 h / s			80 h							0	0	0	0	0	0	0
-	M-P				4	320 h	24.00						7,680	0	0	7,680	320	0	
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	72 h				38.25	28.00		0	0	0	2,754	1,452	4,206	
-	Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	45%	1	36 h				14.85	20.00		0	0	0	535	518	1,053	

Item : (3631 to 3635)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
3630	Foundation																
3631	Dam 1 - Foundation																
	Rock Excavation			3,000 m³													
	Drilling																
	Drilling grid ,9 x 0,9	0.90	0.90	0.81 m²													
	Drilling length			3,704 m													
	Production of			200 m / machine / sh	19 sh												
				2 machines	10 sh												
				10 h / s	95 h												
	- M-P				4	380 h	24.00										380
	- Hydraulic Drilling Machine	19.40	15.00	90%	2	171 h			19.40	15.00				3,317	1,847		5,164
	- Drilling materials					3,704 m	0.70						2,593				2,593
	Blasting																
	Average depth of holes			2 m													
	Number of holes			1,852 un													
	- Dynamite	1 kg / m³	3,000 kg	Losses 5%		3,150 kg	5.60						17,640				17,640
	- Caps			Losses 5%		1,945 un	4.50						8,753				8,753
	- M-P				4	380 h	24.00										380
	- Explosives Truck	5.00	15.00	90%	1	86 h			5.00	15.00				430	929		1,359
	- Misc. Blasting materials					3,000 m³	0.10						300				300
	Evacuation of excavated materials																
	Production of			316 m³ / sh													
	1.5 loose »»»»			474 m³ / sh													
				10 h / s													
	- M-P				6	570 h	24.00										570
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	86 h			38.25	28.00				3,290	1,734		5,024
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	1	86 h			32.00	27.90				2,752	1,728		4,480
	- Generator 5 kW (Tower light)	3.50	2.20	90%	2	171 h			3.50	2.20				599	271		870
	- Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	86 h			19.00	29.00				1,634	1,796		3,430
	Hauling distance			1.00 km													
	Loading			4													
	Trip up			2													
	Unloading			4													
	Back trip			2													
				12 min.													

Item : (3631 to 3635)

WBS	DESCRIPTION		UNIT PRICES							TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
			Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
	%	n															
										24.00 \$					0.72 \$		
	Efficiency :	85%	14 min. / trip							0	0	0	0	0	0	0	
			0.24 h / trip							0	0	0	0	0	0	0	
			9 h / sh							0	0	0	0	0	0	0	
			39 trips / sh							0	0	0	0	0	0	0	
	Cat 740 Articulated Dumper 40 T		21.0 m³							0	0	0	0	0	0	0	
			819 m³/mach/sh							0	0	0	0	0	0	0	
	Number of trucks per shift		1							0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	
	Foundation preparation		300 x 4				1,200 m²			0	0	0	0	0	0	0	
	Production of		50 m² / sh				24 sh			0	0	0	0	0	0	0	
			10 h / s				240 h			0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	
	- M-P				10		2,400 h	24.00		57,600	0	0	0	0	0	57,600	2,400
										0	0	0	0	0	0	0	
	- Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1		216 h		19.00	29.00	0	0	4,104	4,510	8,614		
	- Compressor - 750 cfm	14.30	27.00	90%	1		216 h		14.30	27.00	0	0	3,089	4,199	7,288		
	- Compressor XAHS 237 (500 cfm)	15.00	29.00	90%	1		216 h		15.00	29.00	0	0	3,240	4,510	7,750		
	- Generator 5 kW (Tower light)	3.50	2.20	90%	2		432 h		3.50	2.20	0	0	1,512	684	2,196		
	- Miscellaneous						1,200 m²	3.00		0	3,600	0	0	0	3,600		
	Industrial water supply									0	0	0	0	0	0		
										0	0	0	0	0	0		
	Marerials						1,000 m	200.00		0	200,000	0	0	0	200,000		
										0	0	0	0	0	0		
	Installation and Dismantling						8 sh			0	0	0	0	0	0		
			10 h / s				80 h			0	0	0	0	0	0		
										0	0	0	0	0	0		
	- M-P				6		480 h	24.00		11,520	0	0	0	0	11,520	480	
										0	0	0	0	0	0		
	- Cat 329DL Hydraulic Excavator	19.00	29.00	45%	1		36 h		19.00	20.88	0	0	684	752	1,436		
	- Boom truck 17 tons	13.65	18.00	90%	1		72 h		13.65	12.96	0	0	983	933	1,916		
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	20%	1		16 h		37.00	14.40	0	0	592	230	822		
										0	0	0	0	0	0		
										0	0	0	0	0	0		
3631	Dam 1 - Foundation									101,040	232,886	0	26,226	24,123	384,275	4,210	

3632	Dam 2 - Foundation															
	Overburden excavation						23,000 m²			0	0	0	0	0	0	
	Production of		900 m² / sh				26 sh			0	0	0	0	0	0	
			10 h / sh				260 h			0	0	0	0	0	0	
										0	0	0	0	0	0	
	- M-P				6		1,560 h	24.00		37,440	0	0	0	0	37,440	1,560
										0	0	0	0	0	0	
	- Cat 385CL Hydraulic Excavator	50.00	70.75	90%	1		234 h		50.00	70.75	0	0	11,700	11,920	23,620	
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1		234 h		38.25	28.00	0	0	8,951	4,717	13,668	

Item : (3631 to 3635)

WBS	UNIT PRICES										TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
	DESCRIPTION		Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
	%	n																
										24.00 \$				0.72 \$				
-	Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	2	468 h			32.00	27.90	0	0	0	14,976	9,401	24,377		
-	Generator 5 kW (Tower light)	3.50	2.20	90%	2	468 h			3.50	2.20	0	0	0	1,638	741	2,379		
	Evacuation of excavated materials																	
	Average hauling distance :	1.00 km																
	Loading	4																
	Going	2	30 km / h															
	Unloading	3																
	Return	2	35 km / h															
		11 min.																
	Efficacité :	85%	13 min. / trip															
			0.22 h / trip															
			9 h / sh															
			42 trips / sh															
	Cat 740 Articulated Dumper 40 T	21 m³																
		882 m³ / truck-sh																
	Number of trucks :	2																
	Rock Excavation					3,825 m³												
	Drilling																	
	Drilling grid ,9 x 0,9	0.90	0.90	0.81 m²														
	Drilling length		4,722 m															
	Production of	200 m / machine / sh		24 sh														
		2 machines		12 sh														
		10 h / s		120 h														
-	M-P				4	480 h	24.00				11,520	0	0	0	0	11,520	480	
-	Hydraulic Drilling Machine	19.40	15.00	90%	2	216 h			19.40	15.00	0	0	0	4,190	2,333	6,523		
-	Drilling materials					4,722 m	0.70				0	3,305	0	0	0	3,305		
	Blasting																	
	Average depth of holes		2 m															
	Number of holes	2,361 un																
-	Dynamite 1 kg / m³	3,825 kg	Losses 5%			4,016 kg	5.60				0	22,490	0	0	0	22,490		
-	Caps		Losses 5%			2,479 un	4.50				0	11,156	0	0	0	11,156		
-	M-P				4	480 h	24.00				11,520	0	0	0	0	11,520	480	
-	Explosives Truck	5.00	15.00	90%	1	108 h			5.00	15.00	0	0	0	540	1,166	1,706		
-	Misc. Blasting materials					3,825 m³	0.10				0	383	0	0	0	383		
	Evacuation of excavated materials																	
	Production of	319 m³ / sh																
	1.5 loose »»»»	478 m³ / sh				12 sh												
			10 h / s			120 h												
-	M-P				6	720 h	24.00				17,280	0	0	0	0	17,280	720	

Item : (3631 to 3635)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$				0.72 \$			
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	108	h				38.25	28.00	0	0	0	0	0
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	1	108	h				32.00	27.90	0	0	0	4,131	2,177
	- Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	108	h				19.00	29.00	0	0	0	2,052	2,255
	- Generator 5 kW (Tower light)	3.50	2.20	90%	2	216	h				3.50	2.20	0	0	0	756	342
	Hauling distance	1.00	km										0	0	0	0	0
	Loading	4											0	0	0	0	0
	Trip up	2	25 km / h										0	0	0	0	0
	Unloading	4											0	0	0	0	0
	Back trip	2	35 km / h										0	0	0	0	0
	Efficiency :	85%	14 min. / trip										0	0	0	0	0
			0.24 h / trip										0	0	0	0	0
			9 h / sh										0	0	0	0	0
			39 trips / sh										0	0	0	0	0
	Cat 740 Articulated Dumper 40 T	21.0	m³										0	0	0	0	0
		819	m³/mach/sh										0	0	0	0	0
	Number of trucks per shift	1											0	0	0	0	0
	Foundation preparation	475	x			4							0	0	0	0	0
	Production of	50	m² / sh										0	0	0	0	0
			10 h / s										0	0	0	0	0
													0	0	0	0	0
	- M-P				10	3,800	h		24.00				91,200	0	0	0	0
													0	0	0	0	0
	- Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	342	h				19.00	29.00	0	0	0	6,498	7,141
	- Compressor - 750 cfm	14.30	27.00	90%	1	342	h				14.30	27.00	0	0	0	4,891	6,648
	- Compressor XAHS 237 (500 cfm)	15.00	29.00	90%	1	342	h				15.00	29.00	0	0	0	5,130	7,141
	- Generator 5 kW (Tower light)	3.50	2.20	90%	2	684	h				3.50	2.20	0	0	0	2,394	1,083
	- Miscellaneous					1,900	m²		3.00				0	5,700	0	0	0
	Industrial water supply												0	0	0	0	0
	Marerials					1,000	m		200.00				0	200,000	0	0	0
	Installation and Dismantling					8	sh						0	0	0	0	0
			10 h / s										0	0	0	0	0
						80	h						0	0	0	0	0
	- M-P				6	480	h		24.00				11,520	0	0	0	0
													0	0	0	0	0
	- Cat 329DL Hydraulic Excavator	19.00	29.00	45%	1	36	h				19.00	20.88	0	0	0	684	752
	- Boom truck 17 tons	13.65	18.00	90%	1	72	h				13.65	12.96	0	0	0	983	933
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	20%	1	16	h				37.00	14.40	0	0	0	592	230
													0	0	0	0	0
3632	Dam 2 - Foundation												180,480	243,034	0	73,562	61,150
																558,226	7,520

Item : (3631 to 3635)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
3633	Dam 3 - Foundation																
	Overburden excavation			138,000 m²													
	Production of	2,000 m ² / sh		69 sh													
		10 h / sh		690 h													
	- M-P			10 6,900 h	24.00					165,600						165,600	6,900
	- Cat 385CL Hydraulic Excavator	50.00	70.75	90% 2 1,242 h				50.00	70.75	0	0	0	62,100	63,267		125,367	
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90% 2 1,242 h				38.25	28.00	0	0	0	47,507	25,039		72,546	
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90% 3 1,863 h				32.00	27.90	0	0	0	59,616	37,424		97,040	
	- Cat 345 Hydraulic Excavator	40.00	60.00	45% 1 311 h				40.00	60.00	0	0	0	12,440	13,435		25,875	
	- Generator 5 kW (Tower light)	3.50	2.20	90% 4 2,484 h				3.50	2.20	0	0	0	8,694	3,935		12,629	
	Evacuation of excavated materials																
	Average hauling distance :	1.00 km															
	Loading	4															
	Going	2	30 km / h														
	Unloading	3															
	Return	2	35 km / h														
		11 min.															
	Efficacité :	85%	13 min. / trip														
			0.22 h / trip														
			9 h / sh														
			42 trips / sh														
	Cat 740 Articulated Dumper 40 T		21 m ³														
			882 m ³ / truck-sh														
	Number of trucks :	3															
	Rock Excavation			3,800 m³													
	Drilling																
	Drilling grid ,9 x 0,9	0.90	0.90	0.81 m ²													
	Drilling length			4,691 m													
	Production of	200 m / machine / sh		23 sh													
		2 machines		12 sh													
		10 h / s		115 h													
	- M-P			4 460 h	24.00					11,040						11,040	460
	- Hydraulic Drilling Machine	19.40	15.00	90% 2 207 h				19.40	15.00	0	0	0	4,016	2,236		6,252	
	- Drilling materials			4,691 m		0.70				0	3,284	0	0	0		3,284	
	Blasting																
	Average depth of holes	2 m															
	Number of holes	2,346 un															
	- Dynamite	1 kg / m ³	3,800 kg	Losses 5%						0	22,344	0	0	0		22,344	

Item : (3631 to 3635)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
3634	Dam 4 - Foundation																
	Rock Excavation			1,410 m³													
	Drilling																
	Drilling grid ,9 x 0,9	0.90	0.90	0.81 m²													
	Drilling length			1,741 m													
	Production of			200 m / machine / sh	9 sh												
				2 machines	5 sh												
				10 h / s	45 h												
	- M-P				4	180 h	24.00									4,320	180
	- Hydraulic Drilling Machine	19.40	15.00		90%	2			19.40	15.00						0	
	- Drilling materials						0.70									1,219	
	Blasting																
	Average depth of holes			2 m													
	Number of holes			871 un													
	- Dynamite	1 kg / m³		1,410 kg	Losses 5%	1,481 kg	5.60									8,294	8,294
	- Caps				Losses 5%	914 un	4.50									4,113	4,113
	- M-P					4	180 h	24.00								4,320	180
	- Explosives Truck	5.00	15.00		90%	1			5.00	15.00						205	648
	- Misc. Blasting materials						0.10									141	141
	Evacuation of excavated materials																
	Production of			313 m³ / sh													
	1.5 loose »»»»			470 m³ / sh													
				10 h / s	5 sh												
					45 h												
	- M-P					6	270 h	24.00								6,480	270
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00		90%	1			38.25	28.00						1,568	2,395
	- Cat 740 Articulated Dumper 40 T	32.00	27.90		90%	1			32.00	27.90						1,312	2,136
	- Generator 5 kW (Tower light)	3.50	2.20		90%	2			3.50	2.20						284	412
	- Cat 329DL Hydraulic Excavator	19.00	29.00		90%	1			19.00	29.00						779	1,635
					5												
	Hauling distance			1.00 km													
	Loading			4													
	Trip up	2		25 km / h													
	Unloading	4															
	Back trip	2		35 km / h													
		12		min.													
	Efficiency :	85%		14 min. / trip													
				0.24 h / trip													
				9 h / sh													

Item : (3631 to 3635)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
	Blasting									0	0	0	0	0	0	0	
	Average depth of holes	2 m								0	0	0	0	0	0	0	
	Number of holes	1,852 un								0	0	0	0	0	0	0	
	- Dynamite	1 kg / m³	3,000 kg	Losses	5%	3,150 kg		5.60		0	17,640	0	0	0	0	17,640	
	- Caps			Losses	5%	1,945 un		4.50		0	8,753	0	0	0	0	8,753	
	- M-P					4	380 h	24.00		0	0	0	0	0	0	0	380
	- Explosives Truck	5.00	15.00		90%	1	86 h		5.00	15.00	0	0	430	929	1,359		
	- Misc. Blasting materials						3,000 m³	0.10			0	300	0	0	0	300	
	Evacuation of excavated materials										0	0	0	0	0	0	
	Production of	316 m³ / sh					10 sh				0	0	0	0	0	0	
	1.5 loose »»»»	474 m³ / sh					95 h				0	0	0	0	0	0	
				10 h / s							0	0	0	0	0	0	
	- M-P					6	570 h	24.00			13,680	0	0	0	0	13,680	570
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	86 h			38.25	28.00	0	0	3,290	1,734	5,024		
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	1	86 h			32.00	27.90	0	0	2,752	1,728	4,480		
	- Generator 5 kW (Tower light)	3.50	2.20	90%	2	171 h			3.50	2.20	0	0	599	271	870		
	- Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	86 h			19.00	29.00	0	0	1,634	1,796	3,430		
						5											
	Hauling distance	1.00 km									0	0	0	0	0	0	
	Loading	4									0	0	0	0	0	0	
	Trip up	2	25 km / h								0	0	0	0	0	0	
	Unloading	4									0	0	0	0	0	0	
	Back trip	2	35 km / h								0	0	0	0	0	0	
		12 min.									0	0	0	0	0	0	
	Efficiency :	85%	14 min. / trip								0	0	0	0	0	0	
			0.24 h / trip								0	0	0	0	0	0	
			9 h / sh								0	0	0	0	0	0	
			39 trips / sh								0	0	0	0	0	0	
	Cat 740 Articulated Dumper 40 T	21.0 m³									0	0	0	0	0	0	
		819 m³/mach/sh									0	0	0	0	0	0	
	Number of trucks per shift	1									0	0	0	0	0	0	
	Foundation preparation	275 x	4				1,100 m²				0	0	0	0	0	0	
	Production of	50 m² / sh					22 sh				0	0	0	0	0	0	
				10 h / s			220 h				0	0	0	0	0	0	
	- M-P					10	2,200 h	24.00			52,800	0	0	0	0	52,800	2,200
	- Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	198 h			19.00	29.00	0	0	3,762	4,134	7,896		
	- Compressor - 750 cfm	14.30	27.00	90%	1	198 h			14.30	27.00	0	0	2,831	3,849	6,680		
	- Compressor XAHS 237 (500 cfm)	15.00	29.00	90%	1	198 h			15.00	29.00	0	0	2,970	4,134	7,104		
	- Generator 5 kW (Tower light)	3.50	2.20	90%	2	396 h			3.50	2.20	0	0	1,386	627	2,013		
	- Miscellaneous						1,100 m²	3.00			0	3,300	0	0	0	3,300	

Item : (3641 to 3645)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
	14,300 m³									24.00 \$					0.72 \$		
	Number of passes																
	Height 20 m																
	Thickness 0.225 m	89 layers															
	Asphalt 0.090 m³ / m of layer																
	Total length 14,444 m	162 m / layer															
	Progression 70 m / h	2.31 h / layer															
	10 hours shift	4.32 layers / sh	OK														
	Mechanical placement	3 layers / sh								0	0	0	0	0	0		
	Testing bench 25 m	4 layers								6 sh							
										30 sh							
										36 sh							
		10 h / sh								360 h				0	0		
-	M-P		10	3,600 h	24.00					0	0	0	0	0	0		
										86,400	0	0	0	0	86,400		
										0	0	0	0	0	0		
										0	0	0	0	0	0		
-	Paver	50.00 40.00	90%	1 324 h				50.00 40.00		0	0	16,200	9,331	25,531			
-	Cat 950H Wheel Loader	18.35 9.05	90%	1 324 h				18.35 9.05		0	0	5,945	2,111	8,056			
-	Cat 329DL Hydraulic Excavator	19.00 29.00	90%	1 324 h				19.00 29.00		0	0	6,156	6,765	12,921			
-	10 Wheeler Truck	24.00 20.00	90%	1 324 h				24.00 20.00		0	0	7,776	4,666	12,442			
-	Cat CB 225 Compactor	14.85 20.00	90%	2 648 h				14.85 20.00		0	0	9,623	9,331	18,954			
-	Plate damper 1T	2.00 1.45	90%	1 324 h				2.00 1.45		0	0	648	338	986			
			7							0	0	0	0	0			
-	Miscellaneous (propane and accessories, uppers, etc)			14,444 m		3.00				0	43,332	0	0	0	43,332		
	Asphalt Transportation from Batch Plan									0	0	0	0	0	0		
	Production 44 m³ / sh									0	0	0	0	0	0		
	Average hauling distance : 2.00 km									0	0	0	0	0	0		
	Loading 10									0	0	0	0	0	0		
	Going 4 30 km / h									0	0	0	0	0	0		
	Unloading 30									0	0	0	0	0	0		
	Return 3 35 km / h									0	0	0	0	0	0		
	47 min.									0	0	0	0	0	0		
	Efficiency : 85%	55 min. / trip								0	0	0	0	0	0		
		0.92 h / trip								0	0	0	0	0	0		
		9 h / sh								0	0	0	0	0	0		
		10 trips / sh								0	0	0	0	0	0		
	10 Wheeler Truck	8 m³								0	0	0	0	0	0		
		80 m³ / truck-sh								0	0	0	0	0	0		
	Number of trucks : 1									0	0	0	0	0	0		
										0	0	0	0	0	0		
	Manual placement	2 h / layer								0	0	0	0	0	0		
		178 h								20 sh							
		89 layers								200 h							
		9 h / sh eff.								0	0	0	0	0	0		
		10 h / sh								0	0	0	0	0	0		
-	M-P		9	1,800 h	24.00					43,200	0	0	0	0	43,200		
										0	0	0	0	0	0		
										0	0	0	0	0	0		
-	Cat 950H Wheel Loader	18.35 9.05	45%	1 90 h				18.35 9.05		0	0	1,652	586	2,238			
-	Plate damper 1T	2.00 1.45	90%	2 360 h				2.00 1.45		0	0	720	376	1,096			
-	Boom truck 17 tons	13.65 18.00	45%	1 90 h				13.65 18.00		0	0	1,229	1,166	2,395			
										0	0	0	0	0	0		
-	Miscellaneous materials (formwork, spikes, etc...)			89 lay		75.00				0	6,675	0	0	0	6,675		
										0	0	0	0	0	0		

Item : (3641 to 3645)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS			
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption		
										24.00 \$					0.72 \$					
	Supply	(m³)	(mt)																	
	- Asphalt	1,300	1,950	1.94 h / mt	10%	2,145	mt	46.38	5.53	68.96	21.69	6.36	0	11,860	147,911	46,517	9,830	216,118	4,152	
	- 2B Crushed stone	13,000	23,400	0.08 h / mt	5%	24,570	mt	1.84	1.30	0.00	2.08	3.08	45,209	31,941	0	51,106	54,486	182,742	1,966	
	Filter 2B	Transportation from crusher stockpile				13,000	m³													
	Thickness	0.225	m	89	layers															
	2B	0.810	m³ / m of layer																	
		14,444	m	162	m / layer															
	Progression	70	m / h	2.31	h / layer															
	10 hours shift			4.32	layers / sh															
	Production	394	m³ / sh																	
	Average hauling distance :	3.00	km																	
	Loading	5																		
	Going	6		30	km / h															
	Unloading	5																		
	Return	5		35	km / h															
	Efficiency :	21	min.	85%	25 min. / trip															
					0.41 h / trip															
					9 h / sh															
					22 trips / sh															
	10 Wheeler Truck				8 m³															
					176 m³ / truck-sh															
	Number of trucks :	3																		
					30 sh															
					10 h / sh															
	- M-P				5	1,500	h	24.00					36,000	0	0	0	0	36,000		1,500
	- Cat 950H Wheel Loader		18.35	9.05	90%	1	270	h				18.35	9.05	0	0	4,955	1,759	6,714		
	- 10 Wheeler Truck		24.00	20.00	90%	3	810	h				24.00	20.00	0	0	19,440	11,664	31,104		
													0	0	0	0	0	0		
													0	0	0	0	0	0		
3641	Dam 1 - Impervious core												417,582	199,835	336,997	244,061	172,509	1,370,984		21,639

3642	Dam 2 - Impervious core					13,700	m³														
	Impervious core					13,700	m³														
	Asphalt core		1,200																		
	2B Crushed stone		12,500																		
			13,700	m³																	
	Concrete Plinth					1,300	m³						0	0	0	0	0	0	0		
	- Concreting		3.05 h / m³			3,965	h	24.00					95,160	0	0	0	0	0	95,160		3,965
	- Construction materials					1,300	m³		107.00				0	139,100	0	0	0	0	139,100		
	- Construction equipment					1,300	m³				35.00	26.00	0	0	0	45,500	24,336	69,836			
	- Miscellaneous					1,300	m³		1.50				0	1,950	0	0	0	1,950			

Item : (3641 to 3645)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
-	Concrete supply	1,300	4.04 h / m³	2%	1,326	m²	96.85	5.10	186.47	35.08	13.03	128,420	6,763	247,255	46,514	12,440	441,392	5,360
	Reinforcing Steel																	
-	Supply and Fabrication	60 kg / m³	17.27 h / mt		78	mt	414.40	323.08	987.76	79.99	44.86	32,323	25,201	77,045	6,239	2,519	143,327	1,347
	Installation																	
-	M-P	16.00	h / mt		1,248	h	24.00					29,952	0	0	0	0	29,952	1,248
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	20% 1	250	h				37.00	20.00	0	0	0	9,250	3,600	12,850	
-	Boom truck 17 tons	13.65	18.00	50% 1	624	h				13.65	18.00	0	0	0	8,518	8,087	16,605	
	Concrete transportation from the Batching Plan				1,326	m³						0	0	0	0	0	0	
	Average production	50	m³ / sh		27	sh						0	0	0	0	0	0	
			10	h / sh	270	h						0	0	0	0	0	0	
-	M-P				3	810	h	24.00				19,440	0	0	0	0	19,440	810
-	Ready-mix 8 m³	13.60	14.00	90% 1	243	h				13.60	14.00	0	0	0	3,305	2,449	5,754	
	Average hauling distance :	2.00	km									0	0	0	0	0	0	
	Loading	10										0	0	0	0	0	0	
	Going	4	30	km / h								0	0	0	0	0	0	
	Unloading	15										0	0	0	0	0	0	
	Return	3	35	km / h								0	0	0	0	0	0	
		32	min.									0	0	0	0	0	0	
	Efficiency :	85%	38	min. / trip								0	0	0	0	0	0	
			0.63	h / trip								0	0	0	0	0	0	
			9	h / sh								0	0	0	0	0	0	
			15	trips / sh								0	0	0	0	0	0	
	Ready-mix 8 m³	8	m³									0	0	0	0	0	0	
		120	m³ / truck-sh									0	0	0	0	0	0	
	Number of trucks :	1										0	0	0	0	0	0	
												0	0	0	0	0	0	
	Impervious core				13,700	m³						0	0	0	0	0	0	
	Asphalt core	1,200										0	0	0	0	0	0	
	2B Crushed stone	12,500										0	0	0	0	0	0	
		13,700	m³									0	0	0	0	0	0	
	Number of passes											0	0	0	0	0	0	
	Height	15	m									0	0	0	0	0	0	
	Thickness	0.225	m	67	layers							0	0	0	0	0	0	
	Asphalt	0.090	m³ / m of layer									0	0	0	0	0	0	
	Total length	13,333	m	199	m / layer							0	0	0	0	0	0	
	Progression	70	m / h	2.84	h / layer							0	0	0	0	0	0	
	10 hours shift			3.52	layers / sh	OK						0	0	0	0	0	0	
	Mechanical placement	3	layers / sh		22	sh						0	0	0	0	0	0	
					22	sh						0	0	0	0	0	0	
		10	h / sh		220	h						0	0	0	0	0	0	
												0	0	0	0	0	0	

Item : (3641 to 3645)

WBS	DESCRIPTION	UNIT PRICES									TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$			
	Loading	5									0	0	0	0	0	0	0		
	Going	4		30 km / h							0	0	0	0	0	0	0		
	Unloading	5									0	0	0	0	0	0	0		
	Return	3		35 km / h							0	0	0	0	0	0	0		
		17	min.								0	0	0	0	0	0	0		
	Efficiency :	85%		20 min. / trip							0	0	0	0	0	0	0		
				0.33 h / trip							0	0	0	0	0	0	0		
				9 h / sh							0	0	0	0	0	0	0		
				27 trips / sh							0	0	0	0	0	0	0		
	10 Wheeler Truck			8 m³							0	0	0	0	0	0	0		
				216 m³ / truck-sh							0	0	0	0	0	0	0		
				Number of trucks : 3							0	0	0	0	0	0	0		
				22 sh							0	0	0	0	0	0	0		
				10 h / sh							0	0	0	0	0	0	0		
	- M-P		5	1,100 h	24.00						26,400	0	0	0	0	0	0	26,400	1,100
	- Cat 950H Wheel Loader	18.35	9.05	198 h				18.35	9.05		0	0	0	3,633	1,290	4,923			
	- 10 Wheeler Truck	24.00	20.00	594 h				24.00	20.00		0	0	0	14,256	8,554	22,810			
											0	0	0	0	0	0			
											0	0	0	0	0	0			
											0	0	0	0	0	0			
											0	0	0	0	0	0			
3642	Dam 2 - Impervious core										460,365	259,699	460,833	260,334	146,232	1,587,463		23,102	

3644 Dam 4 - Impervious core				7,300 m³															
Impervious core				7,300 m³															
	Asphalt core	700																	
	2B Crushed stone	6,600																	
		7,300 m³																	
Concrete Plinth				500 m³							0	0	0	0	0	0	0	0	0
	- Concreting	3.05 h / m³		1,525 h	24.00						36,600	0	0	0	0	0	0	36,600	1,525
	- Construction materials			500 m³		107.00					0	53,500	0	0	0	0	0	53,500	
	- Construction equipment			500 m³				35.00	26.00		0	0	0	17,500	9,360	0	0	26,860	
	- Miscellaneous			500 m³		1.50					0	750	0	0	0	0	0	750	
	- Concrete supply	500	5.23 h / m³	2%	510 m²	125.64	5.45	180.16	49.26	14.09	64,076	2,779	91,881	25,121	5,173	189,030	2,666		
											0	0	0	0	0	0	0	0	
Reinforcing Steel				30 mt							0	0	0	0	0	0	0	0	0
	- Supply and Fabrication	60 kg / m³	20.00 h / mt		30 mt	480.00	397.44	987.76	121.86	48.96	14,400	11,923	29,633	3,656	1,057	60,669	600		
											0	0	0	0	0	0	0	0	
Installation				480 h							11,520	0	0	0	0	11,520	480		
	- M-P	16.00 h / mt		480 h	24.00						0	0	0	0	0	0	0	0	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	20%	96 h				37.00	20.00	0	0	0	3,552	1,382	4,934			
	- Boom truck 17 tons	13.65	18.00	50%	240 h				13.65	18.00	0	0	0	3,276	3,110	6,386			

Item : (3641 to 3645)

WBS	DESCRIPTION	UNIT PRICES									TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$			
	Efficiency :	47 min.									0	0	0	0	0	0	0		
		85%	55 min. / trip								0	0	0	0	0	0	0		
			0.92 h / trip								0	0	0	0	0	0	0		
			9 h / sh								0	0	0	0	0	0	0		
	10 Wheeler Truck		10 trips / sh								0	0	0	0	0	0	0		
			8 m³								0	0	0	0	0	0	0		
			80 m³ / truck-sh								0	0	0	0	0	0	0		
	Number of trucks :		1								0	0	0	0	0	0	0		
	Manual placement										0	0	0	0	0	0	0		
		2 h / layer	133 layers								0	0	0	0	0	0	0		
		266 h	9 h / sh eff.								0	0	0	0	0	0	0		
			30 sh								0	0	0	0	0	0	0		
			10 h / sh								0	0	0	0	0	0	0		
			300 h								0	0	0	0	0	0	0		
- M-P				9	2,700 h	24.00					64,800	0	0	0	0	0	0	64,800	2,700
- Cat 950H Wheel Loader		18.35	9.05	45%	135 h				18.35	9.05	0	0	0	2,477	880	3,357			
- Plate damper 1T		2.00	1.45	90%	540 h				2.00	1.45	0	0	0	1,080	564	1,644			
- Boom truck 17 tons		13.65	18.00	45%	135 h				13.65	18.00	0	0	0	1,843	1,750	3,593			
- Miscellaneous materials (formwork, spikes, etc...)					133 lay	75.00					0	9,975	0	0	0	9,975			
Supply	(m³) (mt)										0	0	0	0	0	0	0		
- Asphalt	700	1,050	1.87 h / mt	10%	1,155 mt	44.95	6.19	69.06	21.95	6.14	0	7,148	79,767	25,349	5,107	117,371	2,158		
- 2B Crushed stone	6,600	11,880	0.07 h / mt	5%	12,474 mt	1.80	1.38	0.00	2.03	3.15	22,453	17,214	0	25,322	28,291	93,280	873		
Filter 2B	Transportation from crusher stockpile				6,600 m³						0	0	0	0	0	0	0		
	Thickness	0.225 m	133 layers								0	0	0	0	0	0	0		
	2B	0.810 m³ / m of layer									0	0	0	0	0	0	0		
		7,778 m	58 m / layer								0	0	0	0	0	0	0		
	Progression	50 m / h	1.16 h / layer								0	0	0	0	0	0	0		
		10 hours shift	8.62 layers / sh								0	0	0	0	0	0	0		
	Production	141 m³ / sh									0	0	0	0	0	0	0		
	Average hauling distance :	2.00 km									0	0	0	0	0	0	0		
	Loading	5									0	0	0	0	0	0	0		
	Going	4	30 km / h								0	0	0	0	0	0	0		
	Unloading	5									0	0	0	0	0	0	0		
	Return	3	35 km / h								0	0	0	0	0	0	0		
		17 min.									0	0	0	0	0	0	0		
	Efficiency :	85%	20 min. / trip								0	0	0	0	0	0	0		
			0.33 h / trip								0	0	0	0	0	0	0		
			9 h / sh								0	0	0	0	0	0	0		
	10 Wheeler Truck		27 trips / sh								0	0	0	0	0	0	0		
			8 m³								0	0	0	0	0	0	0		
			216 m³ / truck-sh								0	0	0	0	0	0	0		
	Number of trucks :		1								0	0	0	0	0	0	0		
					44 sh						0	0	0	0	0	0	0		
			10 h / sh		440 h						0	0	0	0	0	0	0		
- M-P				5	2,200 h	24.00					52,800	0	0	0	0	0	0	52,800	2,200
- Cat 950H Wheel Loader		18.35	9.05	90%	396 h				18.35	9.05	0	0	0	7,267	2,580	9,847			
- 10 Wheeler Truck		24.00	20.00	90%	396 h				24.00	20.00	0	0	0	9,504	5,702	15,206			

Item : (3641 to 3645)

WBS	DESCRIPTION	UNIT PRICES									TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$			
											0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		
3644	Dam 4 - Impervious core			7,300							420,489	136,199	204,431	188,348	141,866		1,091,333		19,612

3645 Dam 5 - Impervious core		16,500 m³																	
Concrete Plinth		750 m³									0	0	0	0	0	0	0		
Rock Heating and Injection											0	0	0	0	0	0	0		
	Defrosting holes	360																	
	Injection holes	170																	
		530 m																	
Impervious core		16,500 m³																	
	Asphalt core	1,500																	
	2B Crushed stone	15,000																	
		16,500																	
Concrete Plinth		750 m³									0	0	0	0	0	0	0		
	- Concreting	3.05 h / m³				2,288 h	24.00				54,900	0	0	0	0	0	0	54,900	2,288
	- Construction materials					750 m³	107.00				0	80,250	0	0	0	0	0	80,250	
	- Construction equipment					750 m³			35.00	26.00	0	0	0	26,250	14,040	0	40,290		
	- Miscellaneous					750 m³	1.50				0	1,125	0	0	0	0	1,125		
	- Concrete supply	750 5.23 h / m³ 2%				765 m²	125.64	5.45	180.16	49.26	14.09	96,115	4,169	137,822	37,682	7,760	283,548	3,999	
											0	0	0	0	0	0	0		
Reinforcing Steel											0	0	0	0	0	0	0		
	- Supply and Fabrication	60 kg / m³ 20.00 h / mt				45 mt	480.00	397.44	987.76	121.86	48.96	21,600	17,885	44,449	5,484	1,586	91,004	900	
											0	0	0	0	0	0	0		
Installation											0	0	0	0	0	0	0		
	- M-P	16.00 h / mt				720 h	24.00				17,280	0	0	0	0	0	17,280	720	
	- Crane - Rough terrain 50 t (L-Belt)	37.00 20.00 20% 1				144 h			37.00	20.00	0	0	0	5,328	2,074	7,402			
	- Boom truck 17 tons	13.65 18.00 50% 1				360 h			13.65	18.00	0	0	0	4,914	4,666	9,580			
											0	0	0	0	0	0	0		
Concrete transportation from the Batching Plan											0	0	0	0	0	0	0		
	Average production	50 m³ / sh				16 sh					0	0	0	0	0	0	0		
						160 h					0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		
	- M-P					480 h	24.00				11,520	0	0	0	0	0	11,520	480	
											0	0	0	0	0	0	0		
	- Ready-mix 8 m³	13.60 14.00 90% 1				144 h			13.60	14.00	0	0	0	1,958	1,452	3,410			
											0	0	0	0	0	0	0		
	Average hauling distance :	2.00 km									0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		

Item : (3641 to 3645)

WBS	DESCRIPTION			Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n				Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
		3 weeks	168 h/w	528 h							24.00 \$					0.72 \$			
-	M-P			2	1,056 h	24.00						0	0	0	0	0	0	25,344	1,056
-	Boiler - 1500 kW	4.00	90.00	100%	528 h				4.00	90.00		0	0	0	2,112	34,214		36,326	
-	Miscellaneous (hoses, pipes, etc...)	Holes lenthg proportion : 75%			398 m		18.00					0	7,164	0	0	0	0	7,164	
	Impervious core				16,500 m³							0	0	0	0	0	0	0	
	Asphalt core	1,500										0	0	0	0	0	0	0	
	2B Crushed stone	15,000										0	0	0	0	0	0	0	
		16,500	m³									0	0	0	0	0	0	0	
	Number of passes											0	0	0	0	0	0	0	
	Heigth	30 m										0	0	0	0	0	0	0	
	Thickness	0.225 m	133 layers									0	0	0	0	0	0	0	
	Asphalt	0.090 m³ / m of layer										0	0	0	0	0	0	0	
	Total lenthg	16,667 m	125 m / layer									0	0	0	0	0	0	0	
	Progression	70 m / h	1.79 h / layer									0	0	0	0	0	0	0	
	10 hours shift		5.60 layers / sh	OK								0	0	0	0	0	0	0	
	Mechanical placement		3 layers / sh		44 sh							0	0	0	0	0	0	0	
	Testing bench	25 m	4 layers		6 sh							0	0	0	0	0	0	0	
					50 sh							0	0	0	0	0	0	0	
			10 h / sh		500 h							0	0	0	0	0	0	0	
-	M-P			10	5,000 h	24.00						120,000	0	0	0	0	0	120,000	5,000
-	Paver	50.00	40.00	90%	450 h				50.00	40.00		0	0	0	22,500	12,960		35,460	
-	Cat 950H Wheel Loader	18.35	9.05	90%	450 h				18.35	9.05		0	0	0	8,258	2,932		11,190	
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	450 h				19.00	29.00		0	0	0	8,550	9,396		17,946	
-	10 Wheeler Truck	24.00	20.00	90%	450 h				24.00	20.00		0	0	0	10,800	6,480		17,280	
-	Cat CB 225 Compactor	14.85	20.00	90%	900 h				14.85	20.00		0	0	0	13,365	12,960		26,325	
-	Plate damper 1T	2.00	1.45	90%	450 h				2.00	1.45		0	0	0	900	470		1,370	
-	Miscellaneous (propane and accessories, uppers, etc)			7	16,667 m		3.00					0	50,001	0	0	0	0	50,001	
	Asphalt Transportation from Batch Plan											0	0	0	0	0	0	0	
	Production	34 m³ / sh										0	0	0	0	0	0	0	
	Average hauling distance :	1.50 km										0	0	0	0	0	0	0	
	Loading	10										0	0	0	0	0	0	0	
	Going	3	30 km / h									0	0	0	0	0	0	0	
	Unloading	30										0	0	0	0	0	0	0	
	Return	3	35 km / h									0	0	0	0	0	0	0	
		46 min.										0	0	0	0	0	0	0	
	Efficiency :	85%	54 min. / trip									0	0	0	0	0	0	0	
			0.90 h / trip									0	0	0	0	0	0	0	
			9 h / sh									0	0	0	0	0	0	0	
			10 trips / sh									0	0	0	0	0	0	0	
	10 Wheeler Truck		8 m³									0	0	0	0	0	0	0	
			80 m³ / truck-sh									0	0	0	0	0	0	0	
	Number of trucks :	1										0	0	0	0	0	0	0	
	Manual placement	2 h / layer	200 layers (total for 1 depression)									0	0	0	0	0	0	0	

Item : (3651 to 3655)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			

3650 Rockfill

3651 Dam 1 - Rockfill				82,950 m³														
3D	0-900	Rockfill	51,000															
3E	0-225	Crushed stone	23,000															
3F	0-450	Rockfill	1,450															
4	400-600	Riprap	7,500															
			<u>82,950 m³</u>															
3D	0-900	Rockfill	51,000															
3F	0-450	Rockfill	1,450															
			<u>52,450 m³</u>															
Quarry exploitation			60,300 m³															
Needed		82,950 m³ loose																
		7,500 (Rip rap)																
		<u>90,450</u>																
1.5		60,300 m³ bank																
Drilling																		
Drilling grid ,9 x 1,2		0.90	1.20	1.08 m²														
Drilling length				55,833 m														
Production of				200 m / machine / sh	279 sh													
				6 machines	47 sh													
				10 h / s	465 h													
-	M-P			11	5,115 h	24.00												5,115
-	Hydraulic Drilling Machine	19.40	15.00	90%	6	2,511 h		19.40	15.00									
-	Drilling materials					55,833 m	0.70											
Blasting																		
Average depth of holes			10 m															
Number of holes			5,583 un															
-	Dynamite	1 kg / m³	60,300 m³	Losses 5%	63,315 kg	5.60												354,564
-	Caps			Losses 5%	5,862 un	4.50												26,379
-	M-P			8	3,720 h	24.00												3,720
-	Explosives Truck	5.00	15.00	90%	2	837 h		5.00	15.00									
-	Misc. Blasting materials					60,300 m³	0.10											
Mucking (Hauling to crusher 2 or dam site)																		
Production of			1,297 m³ / sh															
1.5	loose >>>>		1,945 m³ / sh															
					47 sh													
					465 h													
-	M-P			12	5,580 h	24.00												5,580

Item : (3651 to 3655)

WBS	DESCRIPTION	%	n	Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS					
							Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption									
											24.00 \$					0.72 \$								
	Hauling distance			1.00 km							0	0	0	0	0	0	0	0	0					
	Loading	4									0	0	0	0	0	0	0	0	0					
	Trip up	2		25 km / h							0	0	0	0	0	0	0	0	0					
	Unloading	4									0	0	0	0	0	0	0	0	0					
	Back trip	2		35 km / h							0	0	0	0	0	0	0	0	0					
		12	min.								0	0	0	0	0	0	0	0	0					
	Efficiency :	85%		14 min. / trip							0	0	0	0	0	0	0	0	0					
				0.24 h / trip							0	0	0	0	0	0	0	0	0					
				9 h / sh							0	0	0	0	0	0	0	0	0					
				39 trips / sh							0	0	0	0	0	0	0	0	0					
	Cat 725 Articulated Dumper 25 T			12.0 m³							0	0	0	0	0	0	0	0	0					
				468 m³/mach/sh							0	0	0	0	0	0	0	0	0					
	Number of trucks per shift			2							0	0	0	0	0	0	0	0	0					
	- Supply From crusher	1.8		0.08 h / mt	41,400	5%					43,470 mt	1.84	1.30	0.00	2.08	3.08	79,985	56,511	0	90,418	96,399	323,313	3,478	
	4 400-600 Riprap										7,500 m²						0	0	0	0	0	0		
	Selection in Quarry excavation																0	0	0	0	0	0		
	Production of			400 m³ / sh							19 sh						0	0	0	0	0	0		
											190 h						0	0	0	0	0	0		
				10 h / s													0	0	0	0	0	0		
	- M-P										5						950 h	24.00					22,800	950
	- Cat 329DL Hydraulic Excavator			19.00	29.00	90%	1				171 h			19.00	29.00		0	0	0	3,249	3,570	6,819		
	- Cat 345 Hydraulic Excavator			40.00	60.00	90%	1				171 h			40.00	60.00		0	0	0	6,840	7,387	14,227		
	- Cat D7R II LGP Track-Type Tractor			38.25	28.00	90%	1				171 h			38.25	28.00		0	0	0	6,541	3,447	9,988		
	- Miscellaneous										7,500 m³		0.30				0	0	0	0	0	0	2,250	
																	0	0	0	0	0	0	0	
3651	Dam 1 - Rockfill										82,950 m³						618,665	490,062	0	401,931	326,667	1,837,325	25,923	

3652	Dam 2 - Rockfill										64,700 m³													
	3D	0-900	Rockfill								29,000													
	3E	0-225	Crushed stone								23,500													
	3F	0-450	Rockfill								2,200													
	4	400-600	Riprap								10,000													
											64,700 m³													
	3D 0-900 Rockfill										29,000													
	3F 0-450 Rockfill										2,200													
											31,200 m³													
	Quarry exploitation										49,800 m³													
	Needed			64,700 m³ loose																				
				10,000 (rip rap)																				
				74,700																				
	1.5			49,800 m³ bank																				

Item : (3651 to 3655)

WBS	DESCRIPTION	%	n	Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
							Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$			
	Transport included in Quarry excavation			31,200 m³															
	Production of	1,200 m³ / sh		26 sh							0	0	0	0	0	0	0	0	
				260 h							0	0	0	0	0	0	0	0	
			10 h / s								0	0	0	0	0	0	0	0	
-	M-P			9	2,340 h	24.00					56,160	0	0	0	0	0	56,160	2,340	
-	Generator 5 kW (Tower light)	3.50	2.20	90%	4	936 h			3.50	2.20	0	0	0	3,276	1,483	4,759			
-	Cat D8T LGP Track-Type Tractor	47.45	38.60	90%	1	234 h			47.45	38.60	0	0	0	11,103	6,503	17,606			
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	234 h			38.25	28.00	0	0	0	8,951	4,717	13,668			
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	2	468 h			19.00	29.00	0	0	0	8,892	9,772	18,664			
-	Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	90%	2	468 h			14.85	20.00	0	0	0	6,950	6,739	13,689			
				6							0	0	0	0	0	0	0	0	
-	Miscellaneous				31,200 m³		0.10				0	3,120	0	0	0	0	3,120		
3E	0-225 Crushed stone			23,500 m³															
	Transport from crusher										0	0	0	0	0	0	0	0	
	Production of	900 m³ / sh		26 sh							0	0	0	0	0	0	0	0	
				260 h							0	0	0	0	0	0	0	0	
			10 h / s								0	0	0	0	0	0	0	0	
-	M-P			9	2,340 h	24.00					56,160	0	0	0	0	0	56,160	2,340	
-	Cat 988H Wheel Loader	39.20	48.00	90%	1	234 h			39.20	48.00	0	0	0	9,173	8,087	17,260			
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	234 h			38.25	28.00	0	0	0	8,951	4,717	13,668			
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	234 h			19.00	29.00	0	0	0	4,446	4,886	9,332			
-	Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	90%	1	234 h			14.85	20.00	0	0	0	3,475	3,370	6,845			
-	Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	2	468 h			24.00	20.00	0	0	0	11,232	6,739	17,971			
				6							0	0	0	0	0	0	0	0	
	Hauling distance	1.00 km									0	0	0	0	0	0	0	0	
	Loading	4									0	0	0	0	0	0	0	0	
	Trip up	2	25 km / h								0	0	0	0	0	0	0	0	
	Unloading	4									0	0	0	0	0	0	0	0	
	Back trip	2	35 km / h								0	0	0	0	0	0	0	0	
		12 min.									0	0	0	0	0	0	0	0	
	Efficiency :	85%	14 min. / trip								0	0	0	0	0	0	0	0	
			0.24 h / trip								0	0	0	0	0	0	0	0	
			9 h / sh								0	0	0	0	0	0	0	0	
			39 trips / sh								0	0	0	0	0	0	0	0	
	Cat 725 Articulated Dumper 25 T		12.0 m³								0	0	0	0	0	0	0	0	
			468 m³/mach/sh								0	0	0	0	0	0	0	0	
	Number of trucks per shift	2									0	0	0	0	0	0	0	0	
-	Supply From crusher	1.8	0.08 h / mt	42,300	5%	44,415 mt	1.84	1.30	0.00	2.08	3.08	81,724	57,740	0	92,383	98,495	330,342	3,553	
4	400-600 Riprap			10,000 m²															
	Selection in Quarry excavation										0	0	0	0	0	0	0	0	
	Production of	400 m³ / sh		25 sh							0	0	0	0	0	0	0	0	
				250 h							0	0	0	0	0	0	0	0	
			10 h / s								0	0	0	0	0	0	0	0	

Item : (3651 to 3655)

WBS	DESCRIPTION			UNIT PRICES							TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
										24.00 \$					0.72 \$				
-	M-P			5	1,250 h	24.00									0			0	
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	225 h			19.00	29.00			4,275	4,698	0			30,000	
-	Cat 345 Hydraulic Excavator	40.00	60.00	90%	1	225 h			40.00	60.00			9,000	9,720	0			18,720	
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	225 h			38.25	28.00			8,606	4,536	0			13,142	
-	Miscellaneous				10,000 m³		0.30						0	0	0			3,000	
3652	Dam 2 - Rockfill				64,700								501,244	415,731	0	349,118	289,240	1,555,333	21,033

3653 Dam 3 - Rockfill				160,000 m³															
3C	0-20	Crushed stone	13,500																
3D	0-900	Rockfill	77,000										0	0	0	0	0	0	0
3E	0-225	Crushed stone	20,000										0	0	0	0	0	0	0
		Random Fill	40,000										0	0	0	0	0	0	0
4	400-600	Riprap	9,500										0	0	0	0	0	0	0
			160,000 m³										0	0	0	0	0	0	0
3D 0-900 Rockfill			77,000 m³										0	0	0	0	0	0	0
		Transport included in Quarry excavation											0	0	0	0	0	0	0
Quarry exploitation			57,667 m³										0	0	0	0	0	0	0
	Needed	77,000 m³ loose											0	0	0	0	0	0	0
		9,500 (Rip rap)											0	0	0	0	0	0	0
		86,500											0	0	0	0	0	0	0
	1.5	57,667 m³ bank											0	0	0	0	0	0	0
Drilling													0	0	0	0	0	0	0
	Drilling grid ,9 x 1,2	0.90	1.20	1.08 m²									0	0	0	0	0	0	0
													0	0	0	0	0	0	0
	Drilling length			53,395 m									0	0	0	0	0	0	0
	Production of			200 m / machine / sh	267 sh								0	0	0	0	0	0	0
				6 machines	45 sh								0	0	0	0	0	0	0
				10 h / s	445 h								0	0	0	0	0	0	0
-	M-P			11	4,895 h	24.00							117,480	0	0	0	0	117,480	4,895
-	Hydraulic Drilling Machine	19.40	15.00	90%	6	2,403 h			19.40	15.00			0	0	0	46,618	25,952	72,570	0
-	Drilling materials				53,395 m		0.70						0	37,377	0	0	0	37,377	0
	Blasting												0	0	0	0	0	0	0
	Average depth of holes		10 m										0	0	0	0	0	0	0
	Number of holes		5,340 un										0	0	0	0	0	0	0
													0	0	0	0	0	0	0
-	Dynamite	1 kg / m³	57,667 m³	Losses 5%	60,550 kg		5.60						0	339,080	0	0	0	339,080	0
-	Caps			Losses 5%	5,606 un		4.50						0	25,227	0	0	0	25,227	0
													0	0	0	0	0	0	0
-	M-P			8	3,560 h	24.00							85,440	0	0	0	0	85,440	3,560
-	Explosives Truck	5.00	15.00	90%	2	801 h			5.00	15.00			0	0	0	4,005	8,651	12,656	0

Item : (3651 to 3655)

WBS	DESCRIPTION	%	n	Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
							Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$			
-	Misc. Blasting materials			57,667 m³			0.10				0	5,767	0	0	0	0	5,767		
	Mucking to Dam 3										0	0	0	0	0	0	0		
	Production of	1,296 m³ / sh									0	0	0	0	0	0	0		
	1.5 loose >>>>	1,944 m³ / sh		45 sh							0	0	0	0	0	0	0		
			10 h / s	445 h							0	0	0	0	0	0	0		
-	M-P		11	4,895 h		24.00					117,480	0	0	0	0	0	117,480	4,895	
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	2	801 h			38.25	28.00	0	0	0	30,638	16,148	46,786			
-	Cat 345 Hydraulic Excavator	40.00	60.00	90%	2	801 h			40.00	60.00	0	0	0	32,040	34,603	66,643			
-	Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	4	1,602 h			32.00	27.90	0	0	0	51,264	32,181	83,445			
-	Generator 5 kW (Tower light)	3.50	2.20	90%	2	801 h			3.50	2.20	0	0	0	2,804	1,269	4,073			
-	Cat 988H Wheel Loader	39.20	48.00	90%	1	401 h			39.20	48.00	0	0	0	15,719	13,859	29,578			
				11							0	0	0	0	0	0	0		
	Hauling distance	2.00 km									0	0	0	0	0	0	0		
	Loading	4									0	0	0	0	0	0	0		
	Trip up	5	25 km / h								0	0	0	0	0	0	0		
	Unloading	4									0	0	0	0	0	0	0		
	Back trip	3	35 km / h								0	0	0	0	0	0	0		
		16 min.									0	0	0	0	0	0	0		
	Efficiency :	85%	19 min. / trip								0	0	0	0	0	0	0		
			0.31 h / trip								0	0	0	0	0	0	0		
			9 h / sh								0	0	0	0	0	0	0		
			29 trips / sh								0	0	0	0	0	0	0		
	Cat 740 Articulated Dumper 40 T	21.0 m³									0	0	0	0	0	0	0		
		609 m³/mach/sh									0	0	0	0	0	0	0		
	Number of trucks per shift	4									0	0	0	0	0	0	0		
3D	0-900 Rockfill			77,000 m³															
	Transport included in Quarry excavation																		
	Production of	1,200 m³ / sh		64 sh							0	0	0	0	0	0	0		
			10 h / s	640 h							0	0	0	0	0	0	0		
-	M-P		9	5,760 h		24.00					138,240	0	0	0	0	0	138,240	5,760	
-	Generator 5 kW (Tower light)	3.50	2.20	90%	4	2,304 h			3.50	2.20	0	0	0	8,064	3,650	11,714			
-	Cat D8T LGP Track-Type Tractor	47.45	38.60	90%	1	576 h			47.45	38.60	0	0	0	27,331	16,008	43,339			
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	576 h			38.25	28.00	0	0	0	22,032	11,612	33,644			
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	2	1,152 h			19.00	29.00	0	0	0	21,888	24,054	45,942			
-	Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	90%	2	1,152 h			14.85	20.00	0	0	0	17,107	16,589	33,696			
				6							0	0	0	0	0	0	0		
-	Miscellaneous			77,000 m³			0.10				0	7,700	0	0	0	0	7,700		
3E	0-225 Crushed stone			20,000 m³															
3C	0-20 Crushed stone			13,500 m³															
	Transport from crusher			33,500 m³							0	0	0	0	0	0	0		
	Production of	500 m³ / sh		67 sh							0	0	0	0	0	0	0		
			10 h / s	670 h							0	0	0	0	0	0	0		
-	M-P		10	6,700 h		24.00					160,800	0	0	0	0	0	160,800	6,700	

Item : (3651 to 3655)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption	
										24.00 \$					0.72 \$				
-	Cat 988H Wheel Loader	39.20	48.00	90%	1	603 h				39.20	48.00	0	0	0	0	23,638	20,840	44,478	
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	603 h				38.25	28.00	0	0	0	0	23,065	12,156	35,221	
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	603 h				19.00	29.00	0	0	0	0	11,457	12,591	24,048	
-	Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	90%	1	603 h				14.85	20.00	0	0	0	0	8,955	8,683	17,638	
-	Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	2	1,206 h				24.00	20.00	0	0	0	0	28,944	17,366	46,310	
					6														
	Hauling distance	2.00 km																	
	Loading	4																	
	Trip up	5	25 km / h																
	Unloading	4																	
	Back trip	3	35 km / h																
		16 min.																	
	Efficiency :	85%	19 min. / trip																
			0.31 h / trip																
			9 h / sh																
			29 trips / sh																
	Cat 725 Articulated Dumper 25 T	12.0 m³																	
		348 m³/mach/sh																	
	Number of trucks per shift	2																	
-	Supply From crusher	1.8	0.08 h / mt	60,300	5%	63,315 mt	1.84	1.97	0.00	2.04	3.90	116,500	124,731	0	129,163	177,789	548,183	5,065	
4	400-600 Riprap					9,500 m²													
	Selection in Quarry excavation																		
	Production of	400 m³ / sh				24 sh													
			10 h / s			240 h													
-	M-P				5	1,200 h	24.00					28,800	0	0	0	0	0	28,800	1,200
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	216 h				19.00	29.00	0	0	0	4,104	4,510	8,614		
-	Cat 345 Hydraulic Excavator	40.00	60.00	90%	1	216 h				40.00	60.00	0	0	0	8,640	9,331	17,971		
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	216 h				38.25	28.00	0	0	0	8,262	4,355	12,617		
-	Miscellaneous					9,500 m³	0.30					0	2,850	0	0	0	2,850		
	Geotextile					20,000 m²													
	Production of	550 m² / sh				36 sh													
			10 h / sh			360 h													
-	M-P				8	2,880	24.00					69,120	0	0	0	0	0	69,120	2,880
-	Boom truck 17 tons	13.65	18.00	90%	1	324 h				13.65	18.00	0	0	0	4,423	4,199	8,622		
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	324 h				19.00	29.00	0	0	0	6,156	6,765	12,921		
	Supply	20,000 m²		15%		23,000 m²	7.50					0	0	172,500	0	0	172,500		
	Geomembrane					9,500 m²													
	Production of	400 m² / sh				24 sh													
			10 h / sh			240 h													
-	M-P				8	1,920	24.00					46,080	0	0	0	0	0	46,080	1,920

Item : (3651 to 3655)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
3654	Dam 4 - Rockfill			32,900 m³													
	3D 0-900 Rockfill			21,000						0	0	0	0	0	0	0	0
	3E 0-225 Crushed stone			10,500						0	0	0	0	0	0	0	0
	4 400-600 Riprap			1,400						0	0	0	0	0	0	0	0
				<u>32,900 m³</u>													
	3D 0-900 Rockfill			21,000 m³													
	Transport included in Canal 3 excavation																
	Production of	1,200 m³ / sh		18 sh						0	0	0	0	0	0	0	0
			10 h / s	180 h						0	0	0	0	0	0	0	0
-	M-P			1,620 h	24.00					38,880	0	0	0	0	0	38,880	1,620
-	Generator 5 kW (Tower light)	3.50	2.20	90% 4	648 h			3.50	2.20	0	0	0	2,268	1,026	0	3,294	
-	Cat D8T LGP Track-Type Tractor	47.45	38.60	90% 1	162 h			47.45	38.60	0	0	0	7,687	4,502	0	12,189	
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90% 1	162 h			38.25	28.00	0	0	0	6,197	3,266	0	9,463	
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90% 2	324 h			19.00	29.00	0	0	0	6,156	6,765	0	12,921	
-	Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	90% 2	324 h			14.85	20.00	0	0	0	4,811	4,666	0	9,477	
-	Miscellaneous			21,000 m³	0.10					0	2,100	0	0	0	0	2,100	
	3E 0-225 Crushed stone			10,500 m³													
	Transport from crusher																
	Production of	900 m³ / sh		12 sh						0	0	0	0	0	0	0	0
			10 h / s	120 h						0	0	0	0	0	0	0	0
-	M-P			960 h	24.00					23,040	0	0	0	0	0	23,040	960
-	Cat 988H Wheel Loader	39.20	48.00	90% 1	108 h			39.20	48.00	0	0	0	4,234	3,732	0	7,966	
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90% 1	108 h			38.25	28.00	0	0	0	4,131	2,177	0	6,308	
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90% 1	108 h			19.00	29.00	0	0	0	2,052	2,255	0	4,307	
-	Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	90% 1	108 h			14.85	20.00	0	0	0	1,604	1,555	0	3,159	
-	Cat 725 Articulated Dumper 25 T	24.00	20.00	90% 2	216 h			24.00	20.00	0	0	0	5,184	3,110	0	8,294	
	Hauling distance	1.00 km		6						0	0	0	0	0	0	0	0
	Loading	4								0	0	0	0	0	0	0	0
	Trip up	2	25 km / h							0	0	0	0	0	0	0	0
	Unloading	4								0	0	0	0	0	0	0	0
	Back trip	2	35 km / h							0	0	0	0	0	0	0	0
	Efficiency :	12 min.								0	0	0	0	0	0	0	0
		85%	14 min. / trip							0	0	0	0	0	0	0	0
			0.24 h / trip							0	0	0	0	0	0	0	0
			9 h / sh							0	0	0	0	0	0	0	0
			39 trips / sh							0	0	0	0	0	0	0	0
	Cat 725 Articulated Dumper 25 T		12.0 m³							0	0	0	0	0	0	0	0
			468 m³/mach/sh							0	0	0	0	0	0	0	0
	Number of trucks per shift	2								0	0	0	0	0	0	0	0

Item : (3651 to 3655)

WBS	DESCRIPTION	%	n	Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
						M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$			
-	Supply From crusher	1.8	0.07 h / mt	18,900	5%	19,845	mt	1.80	1.38	0.00	2.03	3.15	35,721	27,386	0	40,285	45,008	148,400	1,389
4	400-600 Riprap			1,400	m³														
	Selection in Canal excavation																		
	Production of		400 m³ / sh			4	sh												
					10 h / s	40	h												
-	M-P					5	200	h	24.00				4,800	0	0	0	0	4,800	200
-	Cat 329DL Hydraulic Excavator		19.00	29.00	90%	1	36	h			19.00	29.00	0	0	0	684	752	1,436	
-	Cat 345 Hydraulic Excavator		40.00	60.00	90%	1	36	h			40.00	60.00	0	0	0	1,440	1,555	2,995	
-	Cat D7R II LGP Track-Type Tractor		38.25	28.00	90%	1	36	h			38.25	28.00	0	0	0	1,377	726	2,103	
-	Miscellaneous						1,400	m³		0.30			0	420	0	0	0	420	
3654	Dam 4 - Rockfill			32,900									102,441	29,906	0	88,110	81,095	301,552	4,169

3655 Dam 5 - Rockfill										105,200	m³									
	3D	0-900	Rockfill				77,000													
	3E	0-225	Crushed stone				23,000													
	4	400-600	Riprap				5,200													
							<u>105,200</u>	m³												
3D	0-900		Rockfill				77,000	m³					0	0	0	0	0	0	0	0
			Transport included in Canal 3 excavation										0	0	0	0	0	0	0	0
	Production of		1,200 m³ / sh			64	sh						0	0	0	0	0	0	0	0
					10 h / s	640	h						0	0	0	0	0	0	0	0
-	M-P					9	5,760	h	24.00				138,240	0	0	0	0	138,240	5,760	
-	Generator 5 kW (Tower light)		3.50	2.20	90%	4	2,304	h			3.50	2.20	0	0	0	8,064	3,650	11,714		
-	Cat D8T LGP Track-Type Tractor		47.45	38.60	90%	1	576	h			47.45	38.60	0	0	0	27,331	16,008	43,339		
-	Cat D7R II LGP Track-Type Tractor		38.25	28.00	90%	1	576	h			38.25	28.00	0	0	0	22,032	11,612	33,644		
-	Cat 329DL Hydraulic Excavator		19.00	29.00	90%	2	1,152	h			19.00	29.00	0	0	0	21,888	24,054	45,942		
-	Cat CS76 XT Vibratory Soil Compactor		14.85	20.00	90%	2	1,152	h			14.85	20.00	0	0	0	17,107	16,589	33,696		
-	Miscellaneous						77,000	m³		0.10			0	0	0	0	0	0	0	
3E	0-225		Crushed stone				23,000	m³					0	0	0	0	0	0	0	
			Transport from crusher										0	0	0	0	0	0	0	
	Production of		900 m³ / sh			26	sh						0	0	0	0	0	0	0	
					10 h / s	260	h						0	0	0	0	0	0	0	
-	M-P					8	2,080	h	24.00				49,920	0	0	0	0	49,920	2,080	

Item : (3661-3662)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
	1.5 loose »»»» 210 m³ / sh			2 sh						24.00 \$					0.72 \$			
	10 h / s			20 h						0	0	0	0	0	0	0	0	0
-	M-P		10	200 h	24.00					4,800	0	0	0	0	0	4,800		200
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	18 h		38.25	28.00	0	0	0	689	363	1,052			
-	Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	1	18 h		32.00	27.90	0	0	0	576	362	938			
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	2	36 h		19.00	29.00	0	0	0	684	752	1,436			
	Hauling distance 0.50 km									0	0	0	0	0	0	0	0	0
	Loading 4									0	0	0	0	0	0	0	0	0
	Trip up 1 25 km / h									0	0	0	0	0	0	0	0	0
	Unloading 4									0	0	0	0	0	0	0	0	0
	Back trip 1 35 km / h									0	0	0	0	0	0	0	0	0
	Efficiency : 85%									0	0	0	0	0	0	0	0	0
	12 min. / trip									0	0	0	0	0	0	0	0	0
	0.20 h / trip									0	0	0	0	0	0	0	0	0
	9 h / sh									0	0	0	0	0	0	0	0	0
	46 trips / sh									0	0	0	0	0	0	0	0	0
	Cat 740 Articulated Dumper 40 T 21.0 m³									0	0	0	0	0	0	0	0	0
	966 m³/mach/sh									0	0	0	0	0	0	0	0	0
	Number of trucks per shift 1									0	0	0	0	0	0	0	0	0
	Concrete Weir			750 m³						0	0	0	0	0	0	0	0	0
	Foundation preparation			300 m²						0	0	0	0	0	0	0	0	0
	Production of 100 m² / sh			3 sh						0	0	0	0	0	0	0	0	0
	10 h / sh			30 h						0	0	0	0	0	0	0	0	0
-	M-P		5	150 h	24.00					3,600	0	0	0	0	3,600		150	
-	Compressor - 750 cfm	14.30	27.00	90%	1	27 h		14.30	27.00	0	0	0	386	525	911			
-	Generator 5 kW (Tower light)	3.50	2.20	30%	1	9 h		3.50	2.20	0	0	0	32	14	46			
-	Miscellaneous			300 m²		0.50				0	150	0	0	0	150			
	Concrete			750 m³						0	0	0	0	0	0	0	0	
-	Concreting	5.00	h / m³	3,750 h	24.00					90,000	0	0	0	0	90,000		3,750	
-	Construction materials			750 m³		80.00				0	60,000	0	0	0	60,000			
-	Construction equipment			750 m³				38.00	26.00	0	0	0	28,500	14,040	42,540			
-	Concrete supply	750	4.04 h / m³	2%	765 m²	96.85	5.10	186.47	35.08	13.03	74,089	3,902	142,647	26,835	7,177	254,650	3,092	
	Reinforcing Steel									0	0	0	0	0	0	0	0	
-	Supply and Fabrication	60	kg / m³	17.27 h / mt	45 mt	414.40	323.08	987.76	79.99	44.86	18,648	14,539	44,449	3,600	1,453	82,689	777	
	Installation									0	0	0	0	0	0	0	0	

Item : (3661-3662)

WBS	DESCRIPTION		UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
			%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$			
- M-P	16.00	h / mt		720	h	24.00					17,280	0	0	0	0	17,280		720
- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	20% 1	144	h				37.00	20.00	0	0	0	5,328	2,074	7,402		
- Boom truck 17 tons	13.65	18.00	50% 1	360	h			13.65	18.00	0	0	0	4,914	4,666	9,580			
Concrete transportation from the Batching Plan				765	m³						0	0	0	0	0	0		
Average production	80	m³ / sh		10	sh						0	0	0	0	0	0		
		10	h / sh		100	h					0	0	0	0	0	0		
- M-P			4	400	h	24.00					9,600	0	0	0	0	9,600		400
- Readymix 8 m³	13.60	14.00	90% 3	270	h			13.60	14.00	0	0	0	3,672	2,722	6,394			
Average hauling distance :											0	0	0	0	0	0		
Loading											0	0	0	0	0	0		
Going											0	0	0	0	0	0		
Unloading											0	0	0	0	0	0		
Return											0	0	0	0	0	0		
											0	0	0	0	0	0		
Efficacité :											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
Readymix 8 m³											0	0	0	0	0	0		
											0	0	0	0	0	0		
Number of trucks :											0	0	0	0	0	0		
											0	0	0	0	0	0		
											0	0	0	0	0	0		
3661	Spillway 1			0							231,937	80,649	187,096	81,447	38,680	619,809		9,669

Item : (3661-3662)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
3662	Spillway 2																
	Overburden excavation			2,310 m³						0	0	0	0	0	0	0	
	Production of 700 m³ / sh			4 sh						0	0	0	0	0	0	0	
				4 sh						0	0	0	0	0	0	0	
	10 h / sh			40 h						0	0	0	0	0	0	0	
	- M-P			360 h	24.00					8,640	0	0	0	0	0	8,640	360
	- Cat 329DL Hydraulic Excavator	19.00	29.00	36 h				19.00	29.00	0	0	0	684	752	1,436		
	- Cat D6T LGP Track-Type Tractor	28.40	26.10	36 h				28.40	26.10	0	0	0	1,022	677	1,699		
	- Generator 5 kW (Tower light)	3.50	2.20	36 h				3.50	2.20	0	0	0	126	57	183		
	- Tractor truck & Load Carrier - 65 T	11.50	15.00	36 h				11.50	15.00	0	0	0	414	389	803		
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	72 h				24.00	20.00	0	0	0	1,728	1,037	2,765		
										0	0	0	0	0	0		
	Rock Excavation			500 m³						0	0	0	0	0	0	0	
	Drilling									0	0	0	0	0	0	0	
	Drilling grid ,9 x 1,2	0.90	1.20	1.08 m²						0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	
	Drilling length			463 m						0	0	0	0	0	0	0	
	Production of 200 m / machine / sh			3 sh						0	0	0	0	0	0	0	
	10 h / s			30 h						0	0	0	0	0	0	0	
	- M-P			90 h	24.00					2,160	0	0	0	0	0	2,160	90
										0	0	0	0	0	0	0	
	- Hydraulic Drilling Machine	19.40	15.00	27 h				19.40	15.00	0	0	0	524	292	816		
	- Drilling materials			463 m		0.70				0	324	0	0	0	324		
										0	0	0	0	0	0		
	Blasting									0	0	0	0	0	0	0	
	Average depth of holes		6 m							0	0	0	0	0	0	0	
	Number of holes		77 un							0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	
	- Dynamite 1 kg / m³		500 m³	Losses 5%			5.60			0	2,940	0	0	0	2,940		
	- Caps			Losses 5%			4.50			0	365	0	0	0	365		
										0	0	0	0	0	0		
	- M-P			240 h	24.00					5,760	0	0	0	0	5,760		240
										0	0	0	0	0	0		
	- Explosives Truck	5.00	15.00	54 h				5.00	15.00	0	0	0	270	583	853		
	- Misc. Blasting materials			500 m³		0.10				0	50	0	0	0	50		
										0	0	0	0	0	0		
	Mucking									0	0	0	0	0	0	0	
	Production of 167 m³ / sh			3 sh						0	0	0	0	0	0	0	
	1.5 loose »»»» 250 m³ / sh			30 h						0	0	0	0	0	0	0	
	10 h / s									0	0	0	0	0	0	0	
	- M-P			300 h	24.00					7,200	0	0	0	0	7,200		300
										0	0	0	0	0	0		

Item : (3661-3662)

WBS	DESCRIPTION				UNIT PRICES							TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
			%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.72 \$			
-	Cat D7R II LGP Track-Type Tractor	38.25 28.00	90%	1	27 h				38.25	28.00	0	0	0	1,033	544	1,577			
-	Cat 740 Articulated Dumper 40 T	32.00 27.90	90%	1	27 h				32.00	27.90	0	0	0	864	542	1,406			
-	Cat 329DL Hydraulic Excavator	19.00 29.00	90%	2	54 h				19.00	29.00	0	0	0	1,026	1,128	2,154			
	Hauling distance	2.00 km									0	0	0	0	0	0			
	Loading	4									0	0	0	0	0	0			
	Trip up	5 25 km / h									0	0	0	0	0	0			
	Unloading	4									0	0	0	0	0	0			
	Back trip	3 35 km / h									0	0	0	0	0	0			
	Efficiency :	16 min. 85%									0	0	0	0	0	0			
		19 min. / trip									0	0	0	0	0	0			
		0.31 h / trip									0	0	0	0	0	0			
		9 h / sh									0	0	0	0	0	0			
		29 trips / sh									0	0	0	0	0	0			
	Cat 740 Articulated Dumper 40 T	21.0 m³									0	0	0	0	0	0			
		609 m³/mach/sh									0	0	0	0	0	0			
	Number of trucks per shift	1									0	0	0	0	0	0			
	Concrete Weir				1,065 m³						0	0	0	0	0	0			
	Foundation preparation	300 m²									0	0	0	0	0	0			
	Production of	100 m² / sh			3 sh						0	0	0	0	0	0			
		10 h / sh			30 h						0	0	0	0	0	0			
											0	0	0	0	0	0			
-	M-P			5	150 h	24.00					3,600	0	0	0	0	3,600		150	
-	Compressor - 750 cfm	14.30 27.00	90%	1	27 h				14.30	27.00	0	0	0	386	525	911			
-	Generator 5 kW (Tower light)	3.50 2.20	30%	1	9 h				3.50	2.20	0	0	0	32	14	46			
-	Miscellaneous				300 m²		0.50				0	150	0	0	0	150			
	Concrete				1,065 m³						0	0	0	0	0	0			
-	Concreting	5.00 h / m³			5,325 h	24.00					127,800	0	0	0	0	127,800		5,325	
-	Construction materials				1,065 m³		80.00				0	85,200	0	0	0	85,200			
-	Construction equipment				1,065 m³				38.00	26.00	0	0	0	40,470	19,937	60,407			
-	Concrete supply	1,065 5.23 h / m³	2%		1,086 m²	125.64	5.45	180.16	49.26	14.09	136,445	5,918	195,653	53,493	11,016	402,525		5,677	
	Reinforcing Steel										0	0	0	0	0	0			
-	Supply and Fabrication	60 kg / m³ 20.00 h / mt			64 mt	480.00	397.44	987.76	121.86	48.96	30,672	25,397	63,118	7,787	2,252	129,226		1,278	
	Installation										0	0	0	0	0	0			
-	M-P	16.00 h / mt			1,022 h	24.00					24,538	0	0	0	0	24,538		1,022	
-	Crane - Rough terrain 50 t (L-Belt)	37.00 20.00	20%	1	204 h				37.00	20.00	0	0	0	7,548	2,938	10,486			
-	Boom truck 17 tons	13.65 18.00	50%	1	511 h				13.65	18.00	0	0	0	6,975	6,623	13,598			

Item : (3661-3662)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
	Concrete transportation from the Batching Plan			1,086 m ³						0	0	0	0	0	0	0		
	Average production 80 m ³ / sh			14 sh						0	0	0	0	0	0	0		
				140 h						0	0	0	0	0	0	0		
			10 h / sh							0	0	0	0	0	0	0		
	- M-P			3	420 h	24.00				10,080	0	0	0	0	0	0	10,080	420
	- Readymix 8 m ³	13.60	14.00	90%	1					0	0	0	0	0	0	0	0	
					126 h					0	0	0	1,714	1,270	0	0	2,984	
										0	0	0	0	0	0	0	0	
	Average hauling distance :		1.00 km							0	0	0	0	0	0	0	0	
	Loading		10							0	0	0	0	0	0	0	0	
	Going		2							0	0	0	0	0	0	0	0	
	Unloading		15							0	0	0	0	0	0	0	0	
	Return		2							0	0	0	0	0	0	0	0	
			29 min.							0	0	0	0	0	0	0	0	
	Efficacité :		85%							0	0	0	0	0	0	0	0	
					34 min. / trip					0	0	0	0	0	0	0	0	
					0.57 h / trip					0	0	0	0	0	0	0	0	
					9 h / sh					0	0	0	0	0	0	0	0	
					16 trips / sh					0	0	0	0	0	0	0	0	
	Readymix 8 m ³				8 m ³					0	0	0	0	0	0	0	0	
					128 m ³ / truck-sh					0	0	0	0	0	0	0	0	
	Number of trucks :				1					0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
3662	Spillway 2			0						356,895	120,344	258,771	126,096	50,576		912,682		14,862

Item : (3671-3672)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
	Production of	200 m / machine / sh		18 sh						0	0	0	0	0	0			
		2 machines		9 sh						0	0	0	0	0	0			
		10 h / s		90 h						0	0	0	0	0	0			
-	M-P		5	450 h	24.00					10,800	0	0	0	0	0	10,800		450
-	Hydraulic Drilling Machine	19.40 15.00	90%	2	162 h			19.40	15.00	0	0	0	3,143	1,750	4,893			
-	Drilling materials			3,519 m		0.70				0	2,463	0	0	0	2,463			
	Blasting									0	0	0	0	0	0			
	Average depth of holes	8 m								0	0	0	0	0	0			
	Number of holes	440 un								0	0	0	0	0	0			
-	Dynamite	1 kg / m³ 3,800 m³	Losses 5%	3,990 kg		5.60				0	22,344	0	0	0	22,344			
-	Caps		Losses 5%	462 un		4.50				0	2,079	0	0	0	2,079			
-	M-P		4	360 h	24.00					8,640	0	0	0	0	8,640		360	
-	Explosives Truck	5.00 15.00	90%	1	81 h			5.00	15.00	0	0	0	405	875	1,280			
-	Misc. Blasting materials			3,800 m³		0.10				0	380	0	0	0	380			
	Evacuation of excavated materials									0	0	0	0	0	0			
	Production of	422 m³ / sh		9 sh						0	0	0	0	0	0			
	1.5 loose >>>>	633 m³ / sh		90 h						0	0	0	0	0	0			
				10 h / s						0	0	0	0	0	0			
-	M-P		6	540 h	24.00					12,960	0	0	0	0	12,960		540	
-	Cat D7R II LGP Track-Type Tractor	38.25 28.00	90%	1	81 h			38.25	28.00	0	0	0	3,098	1,633	4,731			
-	Cat 329DL Hydraulic Excavator	19.00 29.00	90%	1	81 h			19.00	29.00	0	0	0	1,539	1,691	3,230			
-	Cat 725 Articulated Dumper 25 T	24.00 20.00	90%	2	162 h			24.00	20.00	0	0	0	3,888	2,333	6,221			
-	Generator 5 kW (Tower light)	3.50 2.20	90%	1	81 h			3.50	2.20	0	0	0	284	128	412			
	Hauling distance	0.50 km								0	0	0	0	0	0			
	Loading	4								0	0	0	0	0	0			
	Trip up	1 35 km / h								0	0	0	0	0	0			
	Unloading	4								0	0	0	0	0	0			
	Back trip	1 35 km / h								0	0	0	0	0	0			
		10 min.								0	0	0	0	0	0			
	Efficiency :	85%		12 min. / trip						0	0	0	0	0	0			
				0.20 h / trip						0	0	0	0	0	0			
				9 h / sh						0	0	0	0	0	0			
				46 trips / sh						0	0	0	0	0	0			
	Cat 725 Articulated Dumper 25 T	12.0 m³								0	0	0	0	0	0			
		552 m³/mach/sh								0	0	0	0	0	0			
	Number of trucks per shift	2								0	0	0	0	0	0			
	Rock Support									0	0	0	0	0	0			
	Invert at	660								0	0	0	0	0	0			
	Top at	680								0	0	0	0	0	0			
	Intake	L H		Area						0	0	0	0	0	0			
	2 sides	60 8		480						0	0	0	0	0	0			

Item : (3671-3672)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
	Drilling length			49,306 m							0	0	0	0	0	0		
	Production of			200 m / machine / sh				247 sh			0	0	0	0	0	0		
				4 machines				62 sh			0	0	0	0	0	0		
				10 h / s				618 h			0	0	0	0	0	0		
-	M-P				8			4,940 h	24.00		118,560	0	0	0	0	0	118,560	4,940
-	Hydraulic Drilling Machine	19.40	15.00					2,223 h			0	0	0	43,126	24,008		67,134	
-	Drilling materials							49,306 m	0.70		0	34,514	0	0	0	0	34,514	
	Blasting										0	0	0	0	0	0	0	
	Average depth of holes			10 m							0	0	0	0	0	0	0	
	Number of holes			4,931 un							0	0	0	0	0	0	0	
-	Dynamite	1 kg / m³		53,250 m³		Losses 5%		55,913 kg	5.60		0	313,113	0	0	0	0	313,113	
-	Caps					Losses 5%		5,177 un	4.50		0	23,297	0	0	0	0	23,297	
-	M-P				4			2,470 h	24.00		59,280	0	0	0	0	0	59,280	2,470
-	Explosives Truck	5.00	15.00					556 h			0	0	0	2,780	6,005		8,785	
-	Misc. Blasting materials							53,250 m³	0.10		0	5,325	0	0	0	0	5,325	
	Evacuation of excavated materials										0	0	0	0	0	0	0	
	Production of			862 m³ / sh							0	0	0	0	0	0	0	
	1.5 loose »»»»			1,294 m³ / sh				62 sh			0	0	0	0	0	0	0	
								618 h			0	0	0	0	0	0	0	
-	M-P				9			5,558 h	24.00		133,380	0	0	0	0	0	133,380	5,558
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00			90%	1	556 h			0	0	0	21,267	11,209		32,476	
-	Cat 345 Hydraulic Excavator	40.00	60.00			90%	1	556 h			0	0	0	22,240	24,019		46,259	
-	Cat 988H Wheel Loader	39.20	48.00			90%	1	556 h			0	0	0	21,795	19,215		41,010	
-	Cat 725 Articulated Dumper 25 T	24.00	20.00			90%	3	1,667 h			0	0	0	40,008	24,005		64,013	
-	Generator 5 kW (Tower light)	3.50	2.20			90%	1	556 h			0	0	0	1,946	881		2,827	
	Hauling distance			0.50 km							0	0	0	0	0	0	0	
	Loading			4							0	0	0	0	0	0	0	
	Trip up			1				35 km / h			0	0	0	0	0	0	0	
	Unloading			4							0	0	0	0	0	0	0	
	Back trip			1				35 km / h			0	0	0	0	0	0	0	
				10 min.							0	0	0	0	0	0	0	
	Efficiency :			85%				12 min. / trip			0	0	0	0	0	0	0	
								0.20 h / trip			0	0	0	0	0	0	0	
								9 h / sh			0	0	0	0	0	0	0	
								46 trips / sh			0	0	0	0	0	0	0	
	Cat 725 Articulated Dumper 25 T			12.0 m³							0	0	0	0	0	0	0	
				552 m³/mach/sh							0	0	0	0	0	0	0	
	Number of trucks per shift			3							0	0	0	0	0	0	0	
	Rock Support										0	0	0	0	0	0	0	
	Invert at			660							0	0	0	0	0	0	0	
	Top at			690							0	0	0	0	0	0	0	

Item : (3671-3672)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
-	Jumbo E 2C	14.00		4.5 h	1,634 h				14.00	0	0	0	22,869	0	22,869		
-	Cat GEP 550 - 400KW	6.50	102.40	12 h / r	4,356 h				6.50	102.40	0	0	28,314	321,159	349,473		
	Feet	ft / un															
-	Bits 2"Ø	293,457	1,600		183 un		85.00				0	15,555	0	0	15,555		
-	Bits 4"Ø	17,967	1,500		12 un		500.00				0	6,000	0	0	6,000		
-	Rod 18'	311,424	7,500		42 un		485.00				0	20,370	0	0	20,370		
-	Coupling	311,424	3,700		84 un		50.00				0	4,200	0	0	4,200		
-	Shank	311,424	12,500		25 un		300.00				0	7,500	0	0	7,500		
-	Misc. Materials	311,424			311,424 ft		0.04				0	12,457	0	0	12,457		
	Loading & Blasting				400 h						0	0	0	0	0		
-	M-P				3,200 h	24.00					76,800	0	0	0	76,800		3,200
-	Explosives Truck	5.00	15.00	90%	360 h			5.00	15.00		0	0	1,800	3,888	5,688		
	5.03 m holes																
		363 Rounds															
		<u>Number</u>	<u>Total</u>	<u>Length (m)</u>													
	Contour holes	25	9,075	45,647							0	0	0	0	0		
	Production holes	24	8,712	43,821							0	0	0	0	0		
		49	17,787								0	0	0	0	0		
-	Prima cord	5.5 m		49,913 5%	52,408 m		1.00				0	52,408	0	0	52,408		
-	Cap 6m			17,787 13%	20,099 un		3.50				0	70,347	0	0	70,347		
-	Dynamite RXL 438	49,686 m³		Powder fact 1.6	79,498 kg		5.60				0	445,187	0	0	445,187		
-	XACTEX	9,075 holes		24,956 5%	26,204 kg		7.50				0	196,530	0	0	196,530		
		2.75 kg / hole									0	0	0	0	0		
	Mucking	49,686 m³									0	0	0	0	0		
	1.5 Loose »»»»	74,529 m³									0	0	0	0	0		
		205 m³ / round									0	0	0	0	0		
	Production	140 m³ / h		1.47 h							0	0	0	0	0		
		363 rounds		532 h x 10/9 »»	592 h						0	0	0	0	0		
-	M-P				4,141 h	24.00					99,372	0	0	0	99,372		4,141
-	Cat 329DL Hydraulic Excavator	19.00	29.00	50%	296 h			19.00	29.00		0	0	5,624	6,180	11,804		
-	Cat 988H Wheel Loader	39.20	48.00	90%	532 h			39.20	48.00		0	0	20,854	18,386	39,240		
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	532 h			38.25	28.00		0	0	20,349	10,725	31,074		
-	Cat 725 Articulated Dumper 25 T	24.00	20.00	90%	1,065 h			24.00	20.00		0	0	25,560	15,336	40,896		
	Disposal of excavated materials										0	0	0	0	0		
	Average hauling distance :	2.00 km									0	0	0	0	0		
	Loading	8									0	0	0	0	0		
	Going	4		30 km / h							0	0	0	0	0		
	Unloading	3									0	0	0	0	0		
	Return	4		30 km / h							0	0	0	0	0		
		19 min.									0	0	0	0	0		
	Efficacité :	85%		22 min. / trip							0	0	0	0	0		
				0.37 h / trip							0	0	0	0	0		
				9 h / sh							0	0	0	0	0		

Item : (3671-3672)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
	Cat 725 Articulated Dumper 25 T	25 trips / sh								0	0	0	0	0	0	0		
		12 m³								0	0	0	0	0	0	0		
		300 m³ / truck-sh								0	0	0	0	0	0	0		
	Number of trucks :	2 (1+1)								0	0	0	0	0	0	0		
	Rolling Path																	
		Length 1,690								0	0	0	0	0	0	0		
		Width 8.00								0	0	0	0	0	0	0		
		Thickness 0.30								0	0	0	0	0	0	0		
		Volume 4,056								0	0	0	0	0	0	0		
	Production																	
		800 m³ / sh								0	0	0	0	0	0	0		
				5 sh						0	0	0	0	0	0	0		
				10 h / s						0	0	0	0	0	0	0		
				50 h						0	0	0	0	0	0	0		
	- M-P			8	400 h	24.00				9,600	0	0	0	0	0	0	9,600	400
	- Cat 988H Wheel Loader	39.20 48.00	90%	1	45 h				39.20 48.00	0	0	0	1,764	1,555	0	0	3,319	
	- Cat D7R II LGP Track-Type Tractor	38.25 28.00	90%	1	45 h				38.25 28.00	0	0	0	1,721	907	0	0	2,628	
	- Cat 725 Articulated Dumper 25 T	24.00 20.00	90%	1	45 h				24.00 20.00	0	0	0	1,080	648	0	0	1,728	
	Rock Support																	
	D Shape	5 x 6	29.40 m³	1,690 m	49,686 m³													
		Arc 5.80	Area (m²) 4.40															
		Height 6.25																
		Wall 5.00	25.00															
		Width 5.00																
			29.40															
	Required																	
		Length	Arch (m)							0	0	0	0	0	0	0	0	0
	Class 1	1,267.5	5.80	75%						0	0	0	0	0	0	0	0	0
	Class 2	253.5	5.80	15%						0	0	0	0	0	0	0	0	0
	Class 3	118.3	5.80	7.0%						0	0	0	0	0	0	0	0	0
	Class 4	42.3	5.80	2.5%						0	0	0	0	0	0	0	0	0
	Class 5	8.5	5.80	0.5%						0	0	0	0	0	0	0	0	0
		1,690		100%						0	0	0	0	0	0	0	0	0
	Class 1									0	0	0	0	0	0	0	0	0
	Rock bolts 2,5 m	1 un / m	1,268 un							0	0	0	0	0	0	0	0	0
	Shotcrete 50 mm	9.80 m² / m	1,863 m²	15%						0	0	0	0	0	0	0	0	0
	Wire mesh	9.80 m² / m	10,558 m²	85%						0	0	0	0	0	0	0	0	0
	Class 2									0	0	0	0	0	0	0	0	0
	Rock bolts 2,5 m	1.1 un / m	290 un							0	0	0	0	0	0	0	0	0
	Shotcrete 50 mm	9.80 m² / m	373 m²	15%						0	0	0	0	0	0	0	0	0
	Wire mesh	9.80 m² / m	2,112 m²	85%						0	0	0	0	0	0	0	0	0
	Class 3									0	0	0	0	0	0	0	0	0
	Rock bolts 3 m	1.5 un / m	172 un							0	0	0	0	0	0	0	0	0
	Shotcrete 50 mm	9.80 m² / m	580 m²	50%						0	0	0	0	0	0	0	0	0
	Wire mesh	9.80 m² / m	580 m²	50%						0	0	0	0	0	0	0	0	0
	Class 4									0	0	0	0	0	0	0	0	0
	Rock bolts 4 m	2.6 un / m	109 un							0	0	0	0	0	0	0	0	0
	Shotcrete 50 mm	4.0 m² / m	51 m²	30%						0	0	0	0	0	0	0	0	0
	Wire mesh	4.0 m² / m	118 m²	70%						0	0	0	0	0	0	0	0	0
	Shotcrete 100 mm	5.8 m² / m	245 m²	100%						0	0	0	0	0	0	0	0	0

Item : (3671-3672)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
	- Miscellaneous materials			39 un			200.00			0	7,800	0	0	0	0	7,800		
	Dewatering	Duration	12 months	1,690 m						0	0	0	0	0	0	0		
	Purchase of equipment and materials									0	0	0	0	0	0	0		
	- Pumps			1 ls			20,000			0	20,000	0	0	0	0	20,000		
	- Miscellaneous			1,690 m			15.00			0	25,350	0	0	0	0	25,350		
	- M-P	2.0 h / m		3,380 h		24.00				81,120	0	0	0	0	81,120		3,380	
	Outside Installation			60 h						0	0	0	0	0	0			
	- M-P			420 h		24.00				10,080	0	0	0	0	10,080		420	
	- Equipment			60 h				200.00		0	0	0	12,000	0	12,000			
	Pumping	52 weeks	6 d / w	312 days						0	0	0	0	0	0			
		20 h / day		6,240 h						0	0	0	0	0	0			
	- M-P			6,240 h		24.00				149,760	0	0	0	0	149,760		6,240	
	- Miscellaneous			52 weeks			110.00			0	5,720	0	0	0	5,720			
	Industrial Water Supply									0	0	0	0	0	0			
	Purchase of equipment and materials	Duration	12 months							0	0	0	0	0	0			
	- Pumps			2 un			20,000			0	40,000	0	0	0	40,000			
	- Miscellaneous			1,690 m			21.00			0	35,490	0	0	0	35,490			
	- M-P	2.0 h / m		3,380 h		24.00				81,120	0	0	0	0	81,120		3,380	
	Compressed Air	Duration	12 months							0	0	0	0	0	0			
	- M-P	3.5 h / m		5,915 h		24.00				141,960	0	0	0	0	141,960		5,915	
	- Miscellaneous materials			1,690 m			24.00			0	40,560	0	0	0	40,560			
	Ventilation & Heating									0	0	0	0	0	0			
	- M-P	3.0 h / m		5,070 h		24.00				121,680	0	0	0	0	121,680		5,070	
	- Miscellaneous materials			1,690 m			10.00			0	16,900	0	0	0	16,900			
	- Furnace - 2 500 000 BTU	2.00	91.00	5,070 h					2.00	91.00	0	0	10,140	461,370	471,510			
	Electrical services									0	0	0	0	0	0			
	- M-P	3.5 h / m		5,915 h		24.00				141,960	0	0	0	0	141,960		5,915	
	- Miscellaneous materials			1,690 m			22.00			0	37,180	0	0	0	37,180			
										0	0	0	0	0	0			
										0	0	0	0	0	0			
3671	Tunnel T1 Excavation			0						1,874,496	1,789,893	406,553	573,566	1,027,010	5,671,518		78,105	

Item : (3671-3672)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
3672	Tunnel T1 Intake structure			900 m³														
	Concrete works																	
	Intake Structure			900 m²														
	- Concreting	5.00 h / m³		4,500 h	24.00					108,000	0	0	0	0	0	108,000	4,500	
	- Construction materials			900 m³		80.00				0	72,000	0	0	0	72,000			
	- Construction equipment			900 m³				48.00	40.00	0	0	0	43,200	25,920	69,120			
	- Concrete supply	900	1.87 h / m³	2%	918 m²	44.80	10.69	308.59	13.56	10.78	41,124	9,812	283,288	12,445	7,124	353,793	1,719	
	Reinforcing Steel																	
	- Supply and Fabrication	60 kg / m³	20.71 h / mt		54 mt	497.14	737.38	985.52	102.06	54.07	26,846	39,819	53,218	5,511	2,102	127,496	1,119	
	Installation																	
	- M-P	16.00 h / mt			864 h	24.00					20,736	0	0	0	0	20,736	864	
	- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	20% 1	173 h				37.00	20.00	0	0	0	6,401	2,491	8,892		
	- Boom truck 17 tons	13.65	18.00	50% 1	432 h				13.65	18.00	0	0	0	5,897	5,599	11,496		
	Concrete transportation from the Batching Plan																	
	Average production	50 m³ / sh			918 m³						0	0	0	0	0	0		
					19 sh						0	0	0	0	0	0		
			10 h / sh		190 h						0	0	0	0	0	0		
	- M-P			3	570 h	24.00					13,680	0	0	0	0	13,680	570	
	- Readymix 8 m³	13.60	14.00	90% 2	342 h				13.60	14.00	0	0	0	4,651	3,447	8,098		
	Average hauling distance :	1.00 km									0	0	0	0	0	0		
	Loading	10									0	0	0	0	0	0		
	Going	2	30 km / h								0	0	0	0	0	0		
	Unloading	15									0	0	0	0	0	0		
	Return	2	35 km / h								0	0	0	0	0	0		
		29 min.									0	0	0	0	0	0		
	Efficacité :	85%	34 min. / trip								0	0	0	0	0	0		
			0.57 h / trip								0	0	0	0	0	0		
			9 h / sh								0	0	0	0	0	0		
			16 trips / sh								0	0	0	0	0	0		
	Readymix 8 m³		8 m³								0	0	0	0	0	0		
			128 m³ / truck-sh								0	0	0	0	0	0		
	Number of trucks :	2 (1+1)									0	0	0	0	0	0		
											0	0	0	0	0	0		
3672	Tunnel T1 Intake structure			900 m³							210,386	121,631	336,506	78,105	46,683	793,311	881.46	8,772

Item : (3681 to 3684)

WBS	DESCRIPTION		UNIT PRICES							TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
			%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials				Equipment Operation	Fuel Consumption
										24.00 \$					0.72 \$				
- M-P			4	200 h	24.00						0	0	0	0	0	0	0	0	200
- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	45 h				38.25	28.00	0	0	0	1,721	907	2,628			
- Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	45%	1	23 h				14.85	20.00	0	0	0	342	331	673			
- Cat 329DL Hydraulic Excavator	19.00	29.00	25%	1	13 h				19.00	29.00	0	0	0	247	271	518			
Pavement	0.3	x	10		3 m³ / m						0	0	0	0	0	0			
Production of	800	m³ / sh			4 sh						0	0	0	0	0	0			
					10 h / s						0	0	0	0	0	0			
					38 h						0	0	0	0	0	0			
											0	0	0	0	0	0			
- M-P			##	375 h	24.00						9,000	0	0	0	0	0	9,000		375
- Cat D6T LGP Track-Type Tractor	28.40	26.10	90%	1	34 h				28.40	26.10	0	0	0	966	639	1,605			
- Cat 725 Articulated Dumper 25 T	24.00	20.00	45%	3	51 h				24.00	20.00	0	0	0	1,224	734	1,958			
- Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	25%	1	9 h				14.85	20.00	0	0	0	134	130	264			
- Cat 14M Motorgrader	16.65	25.75	90%	1	34 h				16.65	25.75	0	0	0	566	630	1,196			
- Cat 980H Wheel Loader	29.00	23.45	90%	1	34 h				29.00	23.45	0	0	0	986	574	1,560			
Hauling distance from crusher		3.50 km									0	0	0	0	0	0			
Loading	4										0	0	0	0	0	0			
Trip up	6	35 km / h									0	0	0	0	0	0			
Unloading	4										0	0	0	0	0	0			
Back trip	6	35 km / h									0	0	0	0	0	0			
	20	min.									0	0	0	0	0	0			
Efficiency :	85%	24 min. / trip									0	0	0	0	0	0			
		0.39 h / trip									0	0	0	0	0	0			
		9 h / sh									0	0	0	0	0	0			
		23 trips / sh									0	0	0	0	0	0			
Cat 725 Articulated Dumper 25 T		12.0 m³									0	0	0	0	0	0			
		276 m³/mach/sh									0	0	0	0	0	0			
Number of trucks per shift		3									0	0	0	0	0	0			
- Pavement material	1.8 mt / m³	0.08 h / mt	5%		5,670 mt	1.84	1.97	0.00	2.04	3.90	10,433	11,170	0	11,567	15,921	49,091		454	
Rock excavation - Dry					5,400 m³														
Drilling											0	0	0	0	0	0			
Drilling grid ,9 x 1,2	0.90	1.20			1.08 m²						0	0	0	0	0	0			
											0	0	0	0	0	0			
Drilling length					5,000 m						0	0	0	0	0	0			
Production of		200 m / machine / sh			25 sh						0	0	0	0	0	0			
		4 machines			6 sh						0	0	0	0	0	0			
		10 h / s			63 h						0	0	0	0	0	0			
											0	0	0	0	0	0			
- M-P			8	500 h	24.00						12,000	0	0	0	0	0	12,000		500
- Hydraulic Drilling Machine	19.40	15.00	90%	4	225 h				19.40	15.00	0	0	0	4,365	2,430	6,795			
- Drilling materials					5,000 m		0.70				0	3,500	0	0	0	0	3,500		
											0	0	0	0	0	0			
Blasting											0	0	0	0	0	0			

Item : (3681 to 3684)

WBS	DESCRIPTION		UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
			%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$			
	Average depth of holes	10 m								0	0	0	0	0	0	0		
	Number of holes	500 un								0	0	0	0	0	0	0		
	- Dynamite	1 kg / m³	5,400 m³	Losses	5%	5,670 kg		5.60		0	31,752	0	0	0	0	31,752		
	- Caps			Losses	5%	525 un		4.50		0	2,363	0	0	0	0	2,363		
	- M-P					4	250 h	24.00		6,000	0	0	0	0	0	6,000		250
	- Explosives Truck	5.00	15.00		90%	1	56 h			0	0	0	280	605	885			
	- Misc. Blasting materials						5,400 m³	0.10		0	540	0	0	0	540			
	Mucking									0	0	0	0	0	0	0		
	Production of	864 m³ / sh								0	0	0	0	0	0	0		
	1.5 loose »»»»	1,296 m³ / sh					6 sh			0	0	0	0	0	0	0		
							10 h / s			0	0	0	0	0	0	0		
							63 h			0	0	0	0	0	0	0		
	- M-P					6	375 h	24.00		9,000	0	0	0	0	9,000			375
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00		90%	1	56 h			38.25	28.00	0	0	2,142	1,129	3,271		
	- Cat 345 Hydraulic Excavator	40.00	60.00		90%	1	56 h			40.00	60.00	0	0	2,240	2,419	4,659		
	- Cat 740 Articulated Dumper 40 T	32.00	27.90		90%	2	113 h			32.00	27.90	0	0	3,616	2,270	5,886		
	- Generator 5 kW (Tower light)	3.50	2.20		90%	2	113 h			3.50	2.20	0	0	396	179	575		
	Hauling distance	0.50 km								0	0	0	0	0	0	0		
	Loading	4								0	0	0	0	0	0	0		
	Trip up	1	25 km / h							0	0	0	0	0	0	0		
	Unloading	4								0	0	0	0	0	0	0		
	Back trip	1	35 km / h							0	0	0	0	0	0	0		
		10 min.								0	0	0	0	0	0	0		
	Efficiency :	85%	12 min. / trip							0	0	0	0	0	0	0		
			0.20 h / trip							0	0	0	0	0	0	0		
			9 h / sh							0	0	0	0	0	0	0		
			46 trips / sh							0	0	0	0	0	0	0		
	Cat 740 Articulated Dumper 40 T	21.0 m³								0	0	0	0	0	0	0		
		966 m³/mach/sh								0	0	0	0	0	0	0		
	Number of trucks per shift	2								0	0	0	0	0	0	0		
	Overburden and Rock excavation - Wet						8,000 m³											
	(Including working platform)																	
	Drilling area	150	20	3,000 m²														
	Depth	5 m																
	Over drilling	2.44 m																
	Total drilling	7.44 m		Volume to blast		22,320 m³												
	Drilling									0	0	0	0	0	0	0		
	Drilling grid 3,9 x 3,9	3.90	3.90	15.21 m²						0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
	Drilling length			1,467 m						0	0	0	0	0	0	0		
	Production of	50 m / machine / sh				29 sh				0	0	0	0	0	0	0		
		2 mach				15 sh				0	0	0	0	0	0	0		

Item : (3681 to 3684)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
	10 h / s			150 h						0	0	0	0	0	0	0		
-	M-P		5	750 h	24.00					0	0	0	0	0	0	0		
-	Hydraulic Drilling Machine	19.40	15.00	90% 2	270 h			19.40	15.00	0	0	0	5,238	2,916	0	0	18,000	750
-	Drilling materials (plastic casing, bits, etc...)			1,467 m		5.00				0	7,337	0	0	0	0	0	0	
	Blasting									0	0	0	0	0	0	0	0	
	Average depth of holes	7.44 m								0	0	0	0	0	0	0	0	
	Number of holes	197 un								0	0	0	0	0	0	0	0	
-	Dynamite	1.30 kg / m³	29,016 kg	Losses 5%	30,467 kg		5.60			0	170,615	0	0	0	0	0	170,615	
-	Caps			Losses 5%	207 un		4.50			0	932	0	0	0	0	0	932	
	0.5 h / hole				99 h					0	0	0	0	0	0	0	0	
-	M-P		4	394 h	24.00					9,467	0	0	0	0	0	0	9,467	394
-	Explosives Truck	5.00	15.00	90% 1	89 h			5.00	15.00	0	0	0	444	959	0	0	0	
-	Misc. Blasting materials			29,016 m³		0.30				0	8,705	0	0	0	0	0	8,705	
	Mucking									0	0	0	0	0	0	0	0	
	Volume of material to excavate including sides slopes	44,640		44,640 m³						0	0	0	0	0	0	0	0	
	Production of	900 m³ / sh		50 sh						0	0	0	0	0	0	0	0	
	10 h / s			500 h						0	0	0	0	0	0	0	0	
-	M-P		5	2,500 h	24.00					60,000	0	0	0	0	0	0	60,000	2,500
-	Cat 385CL Hydraulic Excavator	50.00	70.75	90% 1	450 h			50.00	70.75	0	0	0	22,500	22,923	0	0	45,423	
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90% 1	450 h			38.25	28.00	0	0	0	17,213	9,072	0	0	26,285	
-	Cat 740 Articulated Dumper 40 T	32.00	27.90	90% 1	450 h			32.00	27.90	0	0	0	14,400	9,040	0	0	23,440	
	Hauling distance	0.50 km								0	0	0	0	0	0	0	0	
	Loading	4								0	0	0	0	0	0	0	0	
	Trip up	1	35 km / h							0	0	0	0	0	0	0	0	
	Unloading	4								0	0	0	0	0	0	0	0	
	Back trip	1	35 km / h							0	0	0	0	0	0	0	0	
	10 min.									0	0	0	0	0	0	0	0	
	Efficiency :	85%	12 min. / trip							0	0	0	0	0	0	0	0	
			0.20 h / trip							0	0	0	0	0	0	0	0	
			9 h / sh							0	0	0	0	0	0	0	0	
			46 trips / sh							0	0	0	0	0	0	0	0	
	Cat 740 Articulated Dumper 40 T	21.0 m³								0	0	0	0	0	0	0	0	
		966 m³/mach/sh								0	0	0	0	0	0	0	0	
	Number of trucks per shift	1								0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
3681	Canals 1 and 2			20,000 m³						148,780	236,914	0	99,549	80,602		565,845	28.29	6,218

Item : (3681 to 3684)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
3683	Canal 3			92,000 m³													
	Overburden excavation			60,000 m³						0	0	0	0	0	0	0	
	Production of 900 m² / sh / mach			67 sh						0	0	0	0	0	0	0	
	2 machines			34 sh						0	0	0	0	0	0	0	
	10 h / sh			335 h						0	0	0	0	0	0	0	
	- M-P		9	3,015 h	24.00					72,360	0	0	0	0	0	72,360	3,015
	- Cat 345 Hydraulic Excavator	40.00	60.00	90% 2	603 h			40.00	60.00	0	0	0	24,120	26,050	50,170		
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90% 1	302 h			38.25	28.00	0	0	0	11,552	6,088	17,640		
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90% 4	1,206 h			32.00	27.90	0	0	0	38,592	24,226	62,818		
	Evacuation of excavated materials									0	0	0	0	0	0	0	
	Production of 1,800 m³ / sh									0	0	0	0	0	0	0	
	1.5 loose »»»» 2,700 m³ / sh									0	0	0	0	0	0	0	
	Average hauling distance : 1.00 km									0	0	0	0	0	0	0	
	Loading 4									0	0	0	0	0	0	0	
	Going 2		30 km / h							0	0	0	0	0	0	0	
	Unloading 3									0	0	0	0	0	0	0	
	Return 2		35 km / h							0	0	0	0	0	0	0	
	11 min.																
	Efficacité : 85%		13 min. / trip														
			0.22 h / trip														
			9 h / sh														
			42 trips / sh														
	Cat 740 Articulated Dumper 40 T		21 m³														
			882 m³ / truck-sh														
	Number of trucks : 4																
	Construction roads	(m)	(m² / m)	(m³)						0	0	0	0	0	0	0	
		1,000	11	11,000						0	0	0	0	0	0	0	
		1,000		11,000						0	0	0	0	0	0	0	
	Backfill from excavated materials									0	0	0	0	0	0	0	
	Foundation			11,000 m³													
	Production of 1,200 m³ / sh			9 sh						0	0	0	0	0	0	0	
			10 h / s	90 h						0	0	0	0	0	0	0	
	- M-P		4	360 h	24.00					8,640	0	0	0	0	0	8,640	360
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90% 1	81 h			38.25	28.00	0	0	0	3,098	1,633	4,731		
	- Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	45% 1	41 h			14.85	20.00	0	0	0	609	590	1,199		
	- Cat 329DL Hydraulic Excavator	19.00	29.00	25% 1	23 h			19.00	29.00	0	0	0	437	480	917		
	- Miscellaneous (culverts, signalisation, etc...)			1,000 m		2.00				0	2,000	0	0	0	2,000		
	Pavement	0.3	x	10	3 m³ / m					0	0	0	0	0	0	0	

Item : (3681 to 3684)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
	Production of 1,000 m³ / sh			3 sh						0	0	0	0	0	0	0	
				10 h / s						0	0	0	0	0	0	0	
				30 h						0	0	0	0	0	0	0	
	- M-P		##	300 h	24.00					7,200	0	0	0	0	0	7,200	300
	- Cat D6T LGP Track-Type Tractor	28.40	26.10	90% 1	27 h			28.40	26.10	0	0	0	767	507	1,274		
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	45% 3	41 h			24.00	20.00	0	0	0	984	590	1,574		
	- Cat CS76 XT Vibratory Soil Compactor	14.85	20.00	25% 1	8 h			14.85	20.00	0	0	0	119	115	234		
	- Cat 14M Motorgrader	16.65	25.75	90% 1	27 h			16.65	25.75	0	0	0	450	501	951		
	- Cat 980H Wheel Loader	29.00	23.45	90% 1	27 h			29.00	23.45	0	0	0	783	456	1,239		
	Hauling distance from crusher	2.00 km								0	0	0	0	0	0	0	
	Loading	4								0	0	0	0	0	0	0	
	Trip up	3	35 km / h							0	0	0	0	0	0	0	
	Unloading	4								0	0	0	0	0	0	0	
	Back trip	3	35 km / h							0	0	0	0	0	0	0	
	Efficiency :	14 min.	85%	16 min. / trip						0	0	0	0	0	0	0	
				0.27 h / trip						0	0	0	0	0	0	0	
				9 h / sh						0	0	0	0	0	0	0	
				33 trips / sh						0	0	0	0	0	0	0	
	Cat 725 Articulated Dumper 25 T	12.0 m³								0	0	0	0	0	0	0	
		396 m³/mach/sh								0	0	0	0	0	0	0	
	Number of trucks per shift	3								0	0	0	0	0	0	0	
	- Pavement material	1.8 mt / m³	0.07 h / mt		5,400 mt	1.80	1.38	0.00	2.03	3.15	9,720	7,452	0	10,962	12,247	40,381	378
	Rock Excavation				32,000 m³					0	0	0	0	0	0	0	
	Drilling									0	0	0	0	0	0	0	
	Drilling grid ,9 x 1,2	0.90	1.20	1.08 m²						0	0	0	0	0	0	0	
	Drilling length			29,630 m						0	0	0	0	0	0	0	
	Production of			200 m / machine / sh	148 sh					0	0	0	0	0	0	0	
				6 machines	25 sh					0	0	0	0	0	0	0	
				10 h / s	247 h					0	0	0	0	0	0	0	
	- M-P		##	2,467 h	24.00					59,200	0	0	0	0	59,200	2,467	
	- Hydraulic Drilling Machine	19.40	15.00	90% 6	1,332 h			19.40	15.00	0	0	25,841	14,386	40,227			
	- Drilling materials				29,630 m		0.70			0	20,741	0	0	20,741			
	Blasting									0	0	0	0	0	0	0	
	Average depth of holes	10 m								0	0	0	0	0	0	0	
	Number of holes	2,963 un								0	0	0	0	0	0	0	
	- Dynamite	1 kg / m³	32,000 m³	Losses 5%	33,600 kg		5.60			0	188,160	0	0	188,160			
	- Caps			Losses 5%	3,111 un		4.50			0	14,000	0	0	14,000			
	- M-P			4	987 h	24.00				23,680	0	0	0	23,680	987		
	- Explosives Truck	5.00	15.00	90% 1	222 h			5.00	15.00	0	0	0	1,110	2,398	3,508		

Item : (3681 to 3684)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption	
										24.00 \$					0.72 \$				
-	Misc. Blasting materials			32,000 m³					0.10							3,200			
	Mucking																		
	Production of	1,297 m³ / sh																	
	1.5 loose »»»»	1,946 m³ / sh																	
				25 sh															
				247 h															
-	M-P			##	3,207 h				24.00							76,960		76,960	3,207
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	2	444 h										0			
-	Cat 345 Hydraulic Excavator	40.00	60.00	90%	2	444 h										0			
-	Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	4	888 h										0			
-	Generator 5 kW (Tower light)	3.50	2.20	90%	2	444 h										0			
-	Cat 329DL Hydraulic Excavator	19.00	29.00	90%	1	222 h										0			
	Hauling distance		2.00 km																
	Loading	4																	
	Trip up	5	25 km / h																
	Unloading	4																	
	Back trip	3	35 km / h																
		16 min.																	
	Efficiency :	85%																	
			19 min. / trip																
			0.31 h / trip																
			9 h / sh																
			29 trips / sh																
	Cat 740 Articulated Dumper 40 T		21.0 m³																
			609 m³/mach/sh																
	Number of trucks per shift		4																
	Rock Support																		
	Supply																		
-	Rock bolts 6 m	200 un	Losses 3%			206 un			110.00							0		22,660	
	Rock bolts drilling and Installation																		
	Production of	100 m / sh				12 sh													
	6 m bolt	1,200 m	10 h / sh			120 h													
-	M-P			7	840 h				24.00							20,160		20,160	840
-	Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	90%	1	108 h										0			
-	Fork lift 15 T	13.00	9.00	90%	1	108 h										0			
-	Boom truck 17 tons	13.65	18.00	90%	1	108 h										0			
-	Drilling rig (on fork lift)			90%	1	108 h										0			
	Dewatering		Duration																
			2 months																
	Purchase of equipment and materials																		
-	Pumps					1 ls			20,000							0		20,000	
-	Miscellaneous					1,000 m			15.00							0		15,000	
																0		0	

Item : (3681 to 3684)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
	Installation			30 h						0	0	0	0	0	0	0		
	- M-P		7	210 h		24.00				5,040	0	0	0	0	0	5,040		210
	- Equipment			30 h					200.00	0	0	0	6,000	0	0	6,000		
	Pumping			54 days						0	0	0	0	0	0	0		
		9 weeks		6 d/w						0	0	0	0	0	0	0		
				20 h/day						0	0	0	0	0	0	0		
	- M-P		1	1,080 h		24.00				25,920	0	0	0	0	25,920		1,080	
	- Miscellaneous			9 weeks			110.00			0	990	0	0	0	990			
										0	0	0	0	0	0			
										0	0	0	0	0	0			
										0	0	0	0	0	0			
3683	Canal 3			92,000						308,880	294,203	0	201,229	145,230	949,542		12,843	

3684	Canal 4			4,500 m³													
	Overburden and rock excavation			4,500 m²						0	0	0	0	0	0	0	
	All frozen and considered as rock									0	0	0	0	0	0	0	
	Rock Excavation - Dry			4,500 m³						0	0	0	0	0	0	0	
	Drilling									0	0	0	0	0	0	0	
	Drilling grid ,9 x 1,2	0.90	1.20	1.08 m²						0	0	0	0	0	0	0	
	Drilling length			4,167 m						0	0	0	0	0	0	0	
	Production of			200 m / machine / sh						0	0	0	0	0	0	0	
				2 machines						0	0	0	0	0	0	0	
				10 h / s						0	0	0	0	0	0	0	
	- M-P		4	420 h		24.00				10,080	0	0	0	0	10,080		420
	- Hydraulic Drilling Machine	19.40	15.00	90% 2	189 h			19.40	15.00	0	0	0	3,667	2,041	5,708		
	- Drilling materials				4,167 m		0.70			0	2,917	0	0	0	2,917		
	Blasting									0	0	0	0	0	0	0	
	Average depth of holes			10 m						0	0	0	0	0	0	0	
	Number of holes			417 un						0	0	0	0	0	0	0	
	- Dynamite	1 kg / m³		4,500 m³	Losses 5%		5.60			0	26,460	0	0	0	26,460		
	- Caps				Losses 5%		4.50			0	1,971	0	0	0	1,971		
	- M-P		4	420 h		24.00				10,080	0	0	0	0	10,080		420
	- Explosives Truck	5.00	15.00	90% 1	95 h			5.00	15.00	0	0	0	475	1,026	1,501		
	- Misc. Blasting materials				4,500 m³		0.10			0	450	0	0	0	450		
										0	0	0	0	0	0		

Item : (3681 to 3684)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
	Mucking									0	0	0	0	0	0	0		
	Production of	429	m³ / sh							0	0	0	0	0	0	0		
	1.5 loose »»»»	643	m³ / sh							0	0	0	0	0	0	0		
				10	h / s					0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
	- M-P	6		630	h	24.00				15,120	0	0	0	0	0	0	15,120	630
										0	0	0	0	0	0	0		
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	90%	1	95	h			38.25	28.00	0	0	3,634	1,915	5,549		
	- Cat 345 Hydraulic Excavator	40.00	60.00	90%	1	95	h			40.00	60.00	0	0	3,800	4,104	7,904		
	- Cat 740 Articulated Dumper 40 T	32.00	27.90	90%	1	95	h			32.00	27.90	0	0	3,040	1,908	4,948		
	- Generator 5 kW (Tower light)	3.50	2.20	90%	2	189	h			3.50	2.20	0	0	662	299	961		
	Hauling distance		0.50	km						0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
	Loading	4								0	0	0	0	0	0	0		
	Trip up	1		25	km / h					0	0	0	0	0	0	0		
	Unloading	4								0	0	0	0	0	0	0		
	Back trip	1		35	km / h					0	0	0	0	0	0	0		
		10	min.							0	0	0	0	0	0	0		
	Efficiency :	85%		12	min. / trip					0	0	0	0	0	0	0		
				0.20	h / trip					0	0	0	0	0	0	0		
				9	h / sh					0	0	0	0	0	0	0		
				46	trips / sh					0	0	0	0	0	0	0		
	Cat 740 Articulated Dumper 40 T	21.0	m³							0	0	0	0	0	0	0		
		966	m³/mach/sh							0	0	0	0	0	0	0		
	Number of trucks per shift	1								0	0	0	0	0	0	0		
										0	0	0	0	0	0	0		
3684	Canal 4			4,500						35,280	31,798	0	15,278	11,293	93,649		1,470	

Item : (3710 to 3790)

WBS	DESCRIPTION			Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n				Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
												24.00 \$					0.72 \$		
3720	Supply and Installation of High voltage distribution plant																		
	Purchase and Installation																		
	12 kV Distribution center																		
	- PSG 1			1	un			275 000				0	0	275 000	0	0	275 000		
	- PSG 2			1	un			275 000				0	0	275 000	0	0	275 000		
	- PSG 3			1	un			175 000				0	0	175 000	0	0	175 000		
	- M-P			3	2 100 h	24.00						50 400	0	0	0	0	50 400	2 100	
	- Miscellaneous			3	un			500.00		2 000.00		0	1 500	0	6 000	0	7 500		
	400 V Principal distribution center																		
	- SG 1			1	un			155 000				0	0	155 000	0	0	155 000		
	- SG 2			1	un			155 000				0	0	155 000	0	0	155 000		
	- M-P			2	1 200 h	24.00						28 800	0	0	0	0	28 800	1 200	
	- Miscellaneous			2	un			1 000.00		18 000.00		0	2 000	0	36 000	0	38 000		
	400 V Secondary group distribution																		
	- SG 11			1	un			75 000				0	0	75 000	0	0	75 000		
	- SG 12			1	un			75 000				0	0	75 000	0	0	75 000		
	- M-P			2	1 200 h	24.00						28 800	0	0	0	0	28 800	1 200	
	- Miscellaneous			2	un			1 000.00		10 000.00		0	2 000	0	20 000	0	22 000		
	400 V Secondary distribution center																		
	- SG 21			1	un			80 000				0	0	80 000	0	0	80 000		
	- SG 22			1	un			80 000				0	0	80 000	0	0	80 000		
	- SG 23			1	un			85 000				0	0	85 000	0	0	85 000		
	- SG 24			1	un			85 000				0	0	85 000	0	0	85 000		
	- M-P			4	2 400 h	24.00						57 600	0	0	0	0	57 600	2 400	
	- Miscellaneous			4	un			800.00		10 000.00		0	3 200	0	40 000	0	43 200		
	«Barres sous gaine»																		
	- BSG 1			1	un			100 000				0	0	100 000	0	0	100 000		
	- BSG 2			1	un			100 000				0	0	100 000	0	0	100 000		
	- M-P			2	1 500 h	24.00						36 000	0	0	0	0	36 000	1 500	
	- Miscellaneous			2	un			1 000.00		16 000.00		0	2 000	0	32 000	0	34 000		
	Bus Bars																		
	- BB 1			1	un							0	0	0	0	0	0		
	- BB 2			1	un							0	0	0	0	0	0		
	- M-P			2	16 000 h	24.00						384 000	0	0	0	0	384 000	16 000	
												0	0	0	0	0	0		

Item : (3710 to 3790)

WBS	DESCRIPTION	%	n	Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
							Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$			
-	Miscellaneous			2	un			10 000.00		45 000.00		0	20 000	0	90 000	0	110 000		
	Cable 22 to 200 m outside tunnel																		
-	C1A			1.2	km				350 000		0	0	420 000	0	420 000	0	420 000		
-	C1B			1.2	km				350 000		0	0	420 000	0	420 000	0	420 000		
-	C1C			1.2	km				350 000		0	0	420 000	0	420 000	0	420 000		
-	C2A			1.2	km				350 000		0	0	420 000	0	420 000	0	420 000		
-	C2B			1.2	km				350 000		0	0	420 000	0	420 000	0	420 000		
-	C2C			1.2	km				350 000		0	0	420 000	0	420 000	0	420 000		
-	M-P	7.2 km		3000 h / km		21 600 h	24.00				518 400	0	0	0	0	518 400		21 600	
-	Miscellaneous			7.2	km			2 000.00		17 000.00		0	14 400	0	122 400	0	136 800		
	Cable heads 220 kV - Accessories																		
-	TDC			6	un				25 000		0	0	150 000	0	150 000	0	150 000		
-	TDCGIS			6	un				50 000		0	0	300 000	0	300 000	0	300 000		
-	RSC - Splice cables (600 m)			6	un				35 000		0	0	210 000	0	210 000	0	210 000		
-	GIS Between transfo and 200 kV cable			6	un				50 000		0	0	300 000	0	300 000	0	300 000		
-	M-P	150 h / un	24	3 600 h		24.00					86 400	0	0	0	0	86 400		3 600	
-	Miscellaneous			24	un			500.00		3 500.00		0	12 000	0	84 000	0	96 000		
	Battery System																		
-	125 V battery	Protect. A		1	un				40 000		0	0	40 000	0	40 000	0	40 000		
-	125 V battery	Protect. B		1	un				40 000		0	0	40 000	0	40 000	0	40 000		
-	125 V battery	Primer		1	un				15 000		0	0	15 000	0	15 000	0	15 000		
-	Batery charger 300 A			2	un				65 000		0	0	130 000	0	130 000	0	130 000		
-	Batery charger 50 A			1	un				25 000		0	0	25 000	0	25 000	0	25 000		
-	120 V «Onduleur»	10 kVA		1	un				40 000		0	0	40 000	0	40 000	0	40 000		
-	M-P	120 h / un	7	840.0 h		24.00					20 160	0	0	0	0	20 160		840	
-	Miscellaneous			7	un			100.00		600.00		0	700	0	4 200	0	4 900		
3720	Supply and Installation of High voltage distribution plant										1 210 560	57 800	5 445 000	434 600	0	7 147 960		50 440	

3730 Permanent camp Utilities Substation

3731 Water treatment Area Substation	0	0																	
Included in Underground utilities											0	0	0	0	0	0	0	0	0
3731 Water treatment Area Substation											0	0	0	0	0	0	0	0	0

Item : (3710 to 3790)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24.00 \$					0.72 \$			
3732	Administration Building Area Substation			0	0													
	Included in Underground utilities																	
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
3732	Administration Building Area Substation									0	0	0	0	0	0	0	0	0
3733	Sewage Treatment Area Substation			0	0													
	Included in Underground utilities																	
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
3733	Sewage Treatment Area Substation									0	0	0	0	0	0	0	0	0
3734	Fire & Process Water Area Pumping Station Substation			0	0													
	Included in Underground utilities																	
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
3734	Fire & Process Water Area Pumping Station Substation									0	0	0	0	0	0	0	0	0
3735	Maintenance Shop and Warehouse Area Substation			0	0													
	Included in Underground utilities																	
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
3735	Maintenance Shop and Warehouse Area Substation									0	0	0	0	0	0	0	0	0

Item : (3710 to 3790)

WBS	DESCRIPTION	%	n	Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
						M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption			
											24.00 \$					0.72 \$		
3736	Port Facility Substation			0	0													
	Included in 3210 - Warf facilities																	
											0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
											0	0	0	0	0	0	0	
3736	Port Facility Substation										0	0	0	0	0	0	0	
3740	Emergency Generator			0	0													
	Purchase and Installation										0	0	0	0	0	0	0	
	- Generator and Fuel tank			1	un			170 000			0	0	170 000	0	0	170 000	0	
	- M-P	1		100.0	h	24.00				2 400	0	0	0	0	2 400	0	100	
	- Miscellaneous			1	un		1 000		1 500	0	1 000	0	1 500	0	2 500	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
3740	Emergency Generator									2 400	1 000	170 000	1 500	0	174 900		100	
3750	Plant Communications			0	0													
	Purchase and Installation										0	0	0	0	0	0	0	
	- Communications			1	ls			280 000			0	0	280 000	0	0	280 000	0	
	- Fire detection			1	ls			500 000			0	0	500 000	0	0	500 000	0	
	- M-P			6.500	h	24.00				156 000	0	0	0	0	156 000	0	6.500	
	- Miscellaneous			1	un		25 000.0		100 000	0	25 000	0	100 000	0	125 000	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
3750	Plant Communications									156 000	25 000	780 000	100 000	0	1 061 000		6 500	

Item : (3710 to 3790)

WBS	DESCRIPTION	%	n	Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS						
						M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption									
											24.00 \$					0.72 \$								
	Cat 725 Articulated Dumper 25 T			18 trips / sh							0	0	0	0	0	0	0	0						
				12.0 m³							0	0	0	0	0	0	0	0						
				216 m³/mach/sh							0	0	0	0	0	0	0	0						
	Number of trucks per shift			6							0	0	0	0	0	0	0	0						
	- Pavement material			1.8 mt / m³							0	0	0	0	0	0	0	0						
				0.11 h / mt							27 000 mt	2.61	8.08	0.00	2.60	11.98	70 470	218 160	0	70 200	232 891	591 721		2 970
											0	0	0	0	0	0	0	0	0	0	0	0	0	0
											0	0	0	0	0	0	0	0	0	0	0	0	0	0
3770	Switch yard Site										101 670	218 160	0	87 778	244 763	652 371		4 270						

3780 Supply Line to Power Tunnel Intake				19 km																
Supply				19 000 m																
- Conductor							123.15				0	0	2 339 850	0	0	2 339 850				
- Concrete cover from 1				0.09 m³ / m	2.6082 h / m³	855 m³	62.54	17.48	155.35	24.23	22.05	53 468	14 942	132 822	20 716	13 576	235 524		2 230	
- Concrete cover from 2				0.09 m³ / m	4.0420 h / m³	855 m³	96.85	5.10	186.47	35.08	13.03	82 805	4 361	159 429	29 992	8 022	284 609		3 456	
- Miscellaneous (transfo, switch, etc.)						1 ls			200 000			0	0	200 000	0	0	200 000			
Install				19 000 m																
Production of				175 m / sh		109 sh					0	0	0	0	0	0	0	0	0	
16 m³				10 h / sh		1 086 h					0	0	0	0	0	0	0	0	0	
- M-P				6		6 514		24.00			156 343	0	0	0	0	156 343		6 514		
- Cat 329DL Hydraulic Excavator				19.00	29.00	90%	1	977 h		19.00	29.00	0	0	0	18 563	20 400	38 963			
- Readymix 8 m³				13.60	14.00	90%	1	977 h		13.60	14.00	0	0	0	13 287	9 848	23 135			
- Boom truck 17 tons				13.65	18.00	25%	1	271 h		13.65	18.00	0	0	0	3 699	3 512	7 211			
- Miscellaneous						19 000 m		5.00			0	95 000	0	0	0	95 000				
Concrete transportation from the Batching Plans				1 710 m³																
Average production				16 m³ / sh		109 sh					0	0	0	0	0	0	0	0	0	
Average hauling distance :				8.00 km							0	0	0	0	0	0	0	0	0	
Loading				10							0	0	0	0	0	0	0	0	0	
Going				16	30 km / h						0	0	0	0	0	0	0	0	0	
Unloading				15							0	0	0	0	0	0	0	0	0	
Return				14	35 km / h						0	0	0	0	0	0	0	0	0	
				55 min.							0	0	0	0	0	0	0	0	0	
Efficacité :				85%	65 min. / trip						0	0	0	0	0	0	0	0	0	
					1.08 h / trip						0	0	0	0	0	0	0	0	0	
					9 h / sh						0	0	0	0	0	0	0	0	0	
					9 trips / sh						0	0	0	0	0	0	0	0	0	
Readymix 8 m³				8 m³							0	0	0	0	0	0	0	0	0	
					72 m³ / truck-sh						0	0	0	0	0	0	0	0	0	
Number of trucks :				1							0	0	0	0	0	0	0	0	0	

Item : (3710 to 3790)

WBS	DESCRIPTION	%	n	Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
							Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					0.72 \$			
											0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		
											0	0	0	0	0	0	0		
3780	Supply Line to Power Tunnel Intake										292 616	114 303	2 832 101	86 257	55 358	3 380 635		12 200	

3790 Supply Line to Tunnel 1 Intake				20 km																
Supply				20 000 m																
- Conductor																				
- Concrete cover from 2				0.09 m³ / m	4.0420 h / m³	900 m³	96.85	5.10	186.47	35.08	13.03	87 163	4 590	167 820	31 570	8 444	299 587		3 638	
- Concrete cover from 3				0.09 m³ / m	1.8726 h / m³	900 m³	44.80	10.69	308.59	13.56	10.78	40 318	9 620	277 733	12 201	6 984	346 856		1 685	
- Miscellaneous (transfo, switch, etc.)						1 ls			200 000			0	0	200 000	0	0	200 000			
Install				20 000 m																
Production of				175 m / sh		114 sh														
16 m³				10 h / sh		1 143 h														
- M-P				6		6 857	24.00				164 571	0	0	0	0	164 571		6 857		
- Cat 329DL Hydraulic Excavator				19.00	29.00	90% 1	1 029 h			19.00	29.00	0	0	0	19 551	21 486	41 037			
- Readymix 8 m³				13.60	14.00	90% 1	1 029 h			13.60	14.00	0	0	0	13 994	10 372	24 366			
- Boom truck 17 tons				13.65	18.00	25% 1	286 h			13.65	18.00	0	0	0	3 904	3 707	7 611			
- Miscellaneous						20 000 m		5.00				100 000	0	0	0	100 000				
Concrete transportation from the Batching Plans						1 800 m³														
Average production				16 m³ / sh		114 sh														
Average hauling distance :				8.00 km																
Loading				10																
Going				16	30 km / h															
Unloading				15																
Return				14	35 km / h															
				55 min.																
Efficacité :				85%	65 min. / trip															
					1.08 h / trip															
					9 h / sh															
Readymix 8 m³					9 trips / sh															
					8 m³															
					72 m³ / truck-sh															
Number of trucks :				1																
3790	Supply Line to Tunnel 1 Intake										292 052	114 210	3 108 553	81 220	50 993	3 647 028		12 180		

Item : (3810 to 3866)

WBS	DESCRIPTION	UNIT PRICES										TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS								
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption												
							USD →	0.90				35.00 \$			0.72 \$												
	Drainage des eaux claires	835,000 \$										0	0	0	0	0	0	0	0	0	0	0	0	0			
	Drainage des eaux usées	730,000 \$										0	0	0	0	0	0	0	0	0	0	0	0	0			
	Drainage des eaux huileuses	361,000 \$										0	0	0	0	0	0	0	0	0	0	0	0	0			
	Manutention des huiles	470,000 \$										0	0	0	0	0	0	0	0	0	0	0	0	0			
		2,396,000 \$	80%	1	ls			1,916,800				0	0	1,916,800	0	0	0	0	0	0	0	0	0	0	1,916,800		
	- M-P			6,900	h	35.00						241,500	0	0	0	0	0	0	0	0	0	0	0	0	241,500	6,900	
	- Miscellaneous Equipment and Materials		2%	1	ls		47,920		119,800			0	47,920	0	119,800	0	0	0	0	0	0	0	0	0	167,720		
			5%									0	0	0	0	0	0	0	0	0	0	0	0	0	0		
												0	0	0	0	0	0	0	0	0	0	0	0	0	0		
3863	Sewage and Sanitary System											241,500	47,920	1,916,800	119,800	0	0	0	0	0	0	0	0	0	2,326,020	6,900	
3864	Compressed Air System																										
	Supply and Installation											0	0	0	0	0	0	0	0	0	0	0	0	0	0		
	Air comprimé basse pression (air de service)	608,000 \$	80%	1	ls			486,400				0	0	486,400	0	0	0	0	0	0	0	0	0	0	0	486,400	
	- M-P			1,750	h	35.00						61,250	0	0	0	0	0	0	0	0	0	0	0	0	0	61,250	1,750
	- Miscellaneous Equipment and Materials		2%	1	ls		12,160		30,400			0	12,160	0	30,400	0	0	0	0	0	0	0	0	0	0	42,560	
			5%									0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
												0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
3864	Compressed Air System											61,250	12,160	486,400	30,400	0	0	0	0	0	0	0	0	0	0	590,210	1,750
3865	Process Water System																										
	Supply and Installation											0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	Eau de service	586,000 \$										0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	Limnimètres et piézomètres	94,000 \$										0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
		680,000 \$	80%	1	ls			544,000				0	0	544,000	0	0	0	0	0	0	0	0	0	0	0	544,000	
	- M-P			2,000	h	35.00						70,000	0	0	0	0	0	0	0	0	0	0	0	0	0	70,000	2,000
	- Miscellaneous Equipment and Materials		2%	1	ls		13,600		34,000			0	13,600	0	34,000	0	0	0	0	0	0	0	0	0	0	47,600	
			5%									0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
												0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
3865	Process Water System											70,000	13,600	544,000	34,000	0	0	0	0	0	0	0	0	0	0	661,600	2,000
3866	CVAC																										

Item : (3810 to 3866)

WBS	DESCRIPTION	UNIT PRICES							TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	Man power	Consumable materials	Permanent Materials				Equipment Operation
							USD **	0.90		35.00 \$				0.72 \$			
	Supply and Installation									0	0	0	0	0	0	0	0
	Ventilation des bureaux informatique	200,000 \$								0	0	0	0	0	0	0	0
	Ventilation des bureaux	225,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Plancher alternateurs	600,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Bâches et conduites forcées	300,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Salle des compresseurs	10,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Salle mécanique	90,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Galerie des transformateurs	250,000 \$								0	0	0	0	0	0	0	0
	Alimentation d'air extérieur - Centrale	1,000,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Salle des batteries	50,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Salle des huiles et salle des hydrocarbures	75,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Hotte de soudure mobile	70,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Hotte de soudure des bâches	125,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Galerie d'accès permanent	90,000 \$								0	0	0	0	0	0	0	0
	Ventilation et chauffage - Salle de traitement des eaux usées	125,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Atelier mécanique	70,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Atelier électrique	60,000 \$								0	0	0	0	0	0	0	0
	Pressurisation des escaliers	80,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Salle de traitement d'eau potable	50,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Salle de mécanique de l'ascenseur	20,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Salle des pompes	20,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Gaz délétères	150,000 \$								0	0	0	0	0	0	0	0
	Ventilation et chauffage - Prise d'eau	125,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Toilettes	10,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Entrepôt des pièces de rechange	25,000 \$								0	0	0	0	0	0	0	0
	Ventilation - Salle électrique	60,000 \$								0	0	0	0	0	0	0	0
	Système de supervision SCADA	20,000 \$								0	0	0	0	0	0	0	0
	Système de supervision SCADA	20,000 \$								0	0	0	0	0	0	0	0
	Système de supervision SCADA	20,000 \$								0	0	0	0	0	0	0	0
	Système de supervision SCADA	20,000 \$								0	0	0	0	0	0	0	0
		3,960,000 \$ 80%		1 ls				3,168,000		0	0	3,168,000	0	0	3,168,000		
	- M-P			11,500 h		35.00				402,500	0	0	0	0	402,500		11,500
	- Miscellaneous Equipment and Materials		2% 5%	1 ls			79,200		198,000	0	79,200	0	198,000	0	277,200		
										0	0	0	0	0	0		
										0	0	0	0	0	0		
3866	CVAC									402,500	79,200	3,168,000	198,000	0	3,847,700		11,500

Item : (3910)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
3910	Service Building																
	Supply and install									0	0	0	0	0	0	0	
		<u>L</u>	<u>W</u>	<u>H</u>	<u>Volume</u>					0	0	0	0	0	0	0	
	- Service building	68	30	7	14280					0	0	5,497,800	0	0	0	5,497,800	
					14,280 m³					0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	
3910	Service Building									0	0	5,497,800	0	0	0	5,497,800	0

Item : (6213-6223)

WBS	DESCRIPTION			UNIT PRICES							TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Construction materials	Permanent Materials	Equipment Operation	Fuel Consumption				
- Cat 950H Wheel Loader	18.35 9.05	20%	1	260	h				18.35	9.05	24.00 \$	0	0	0	4,771	1,694	6,465		
- Miscellaneous				1	ls		5,000				0	0	5,000	0	0	0	5,000		
											0	0	0	0	0	0	0		
6213	General Site Operation										2,196,638	2,998,288	0	715,153	2,690,348	8,600,427			91,527

Item : (6243-6273)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Construction materials	Permanent Materials	Equipment Operation				Fuel Consumption
6240 NDE & QA/QC Testing Services										24.00 \$					0.72 \$			
6243 NDE & QA/QC Testing Services - Hydro Site 6g																		
	- General Laboratory Shop and Field services			1	ls					0	0	0	0	0	0	0	2,000,000	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
										0	0	0	0	0	0	0	0	
6243	NDE & QA/QC Testing Services									0	0	0	0	0	0	0	2,000,000	0
6250 Surveying																		
6253 Surveying - Hydro Site 6g																		
	Man Power is included in Item 8120																	
	Surveying Equipment									0	0	0	0	0	0	0	0	
	- Instruments Rental			78	mth		1,500.00			0	0	0	0	0	0	0	117,000	
										0	0	0	0	0	0	0	0	
	- Miscellaneous			78	mth		500.00			0	39,000	0	0	0	0	0	39,000	
										0	0	0	0	0	0	0	0	
6253	Surveying									0	156,000	0	0	0	0	0	156,000	0

Item : (6243-6273)

WBS	DESCRIPTION	UNIT PRICES										TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Construction materials	Permanent Materials	Equipment Operation	Fuel Consumption					
6260	Site Security											24.00 \$					0.72 \$			
6263	Site Security - Hydro Site 6g																			
	Man Power is included in Item 8120																			
	Safety Equipment and Materials																			
	Directs	1,591,019										0	0	0	0	0	0	0		
	Indirects	456,380										0	0	0	0	0	0	0		
	Miscellaneous	250,000										0	0	0	0	0	0	0		
		<u>2,297,399</u>	m - hours	100	months							0	0	0	0	0	0	0		
	1) First Aid											0	0	0	0	0	0	0		
	- First aid material and medic care		##### h			0.048						0	110,275	0	0	0	0	110,275		
	- Security Equipment and Accessories		##### h			0.038						0	87,301	0	0	0	0	87,301		
	- Miscellaneous		##### h			0.091						0	209,063	0	0	0	0	209,063		
	2) Fire Protection		1	ls		160,000						0	160,000	0	0	0	0	160,000		
	3) Signalisation		1	ls		20,000.00						0	20,000	0	0	0	0	20,000		
												0	0	0	0	0	0	0		
												0	0	0	0	0	0	0		
6263	Site Security											0	586,639	0	0	0	0	586,639	0	

6270 Man Power Transportation

6273	Man Power Transportation - Hydro Site 6g																		
	Hourly labour																		
	Directs	1,591,019	m - hours									0	0	0	0	0	0	0	
	Indirects	456,380	m - hours									0	0	0	0	0	0	0	
	Catering and maintenance	275,688		12%	Total m_hours							0	0	0	0	0	0	0	
	Miscellaneous	250,000	m - hours									0	0	0	0	0	0	0	
		<u>2,573,086</u>	m - hours									0	0	0	0	0	0	0	
	Say a trip at	40 days	400 h									0	0	0	0	0	0	0	
			6,433 trips									0	0	0	0	0	0	0	
	Helicopter flight	2 hours	903.00 \$			12	Passengers					0	0	0	0	0	0	0	
	Air fare		1,200.00 \$									0	0	0	0	0	0	0	
	- Personal transportation		2,103.00 \$			6,433	trips	2,103				0	13,528,599	0	0	0	0	13,528,599	
	Staff	4,602	m-month									0	0	0	0	0	0	0	
	Say a trip at	1 month	4,602 trips									0	0	0	0	0	0	0	
	Helicopter flight	2 hours	903.00 \$			12	Passengers					0	0	0	0	0	0	0	
	Air fare		1,650.00 \$									0	0	0	0	0	0	0	
	- Staff transportation		2,553.00 \$			4,602	trips	2,553.00				0	11,748,906	0	0	0	0	11,748,906	
	- Home office personal	Average	4 trips / mth			48	mth	192	trips	2,553.00		0	490,176	0	0	0	0	490,176	
												0	0	0	0	0	0	0	
6273	Man Power Transportation											0	25,767,681	0	0	0	0	25,767,681	0

Item : (6330)

WBS	DESCRIPTION			Qty	Un.	Cons. Mat.	Freight in/out	Depr.	Purchase	Construction materials	Freight	Depreciation	Purchase	GLOBAL PRICES
		%	n											
							250							
							125							

6300 Construction Equipment, Tools & Supplies

6330 Construction Equipment, Tools & Supplies - Hydro Site 6g															
TBM - Purchase														0	0
-	HP TBM 5,1 m dia with backup												11,900,000	0	
-	Rock support system												1,200,000	0	
-	12 km Conveyer												4,200,000	0	
-	Spare parts												1,200,000	0	
													18,500,000	0	
	Depreciation												11,100,000	60% 1	
	<u>Transportation to site</u>													0	
-	TBM					462	m³	250.00			115,500			0	
-	Tailing gear					520	m³	250.00			130,000			0	
		22	cont. 20'			38					211,675			0	
		5	cont. 40'			77				0	96,295	0	0	0	
-	Conveyer					5,393	m³	250.00			1,348,129			0	
		1	cont. 20'			38	m³	250.00		0	9,622	0	0	0	
-	Crane - Rough terrain 50 t (L-Belt)					194	m³	250.00	600,000	0	291,000	1,440,000	3,600,000	1,440,000	
-	Crane - Rough terrain 120 t (L-Belt)					169	m³	250.00	890,000	0	42,250	356,000	890,000	356,000	
-	Crane 150T - Crawler	10	77			770	m³	250.00	1,300,000	0	577,500	1,560,000	3,900,000	1,560,000	
-	Fuel Truck					110	m³	250.00	125,000	0	110,000	200,000	500,000	200,000	
-	Boom truck 17 tons					110	m³	250.00	226,600	0	330,000	1,087,680	2,719,200	1,087,680	
-	Readymix 8 m³					90	m³	250.00	200,000	0	135,000	480,000	1,200,000	480,000	
-	Explosives Truck					60	m³	250.00	120,000	0	75,000	240,000	600,000	240,000	
-	Welding or Mechanic Truck					60	m³	250.00		0					
-	Asphalt tanker (12 000 USgallon)					110	m³	250.00	125,000		55,000	100,000	250,000	100,000	
-	Concrete pump 45 m on truck					220	m³	250.00	800,000	0	110,000	640,000	1,600,000	640,000	
-	Cat 950H Wheel Loader					80	m³ / un	250.00	284,730		160,000	911,136	2,277,840	911,136	
-	Cat 980H Wheel Loader					118	m³ / un	250.00	501,080		118,000	801,728	2,004,320	801,728	
-	Cat 988H Wheel Loader					181	m³ / un	250.00	806,225		271,500	1,934,940	4,837,350	1,934,940	
											0	0	0	0	
-	Cat 329DL Hydraulic Excavator					112	m³ / un	250.00	314,325		280,000	1,257,300	3,143,250	1,257,300	
-	Cat 345 Hydraulic Excavator					290	m³ / un	250.00	800,000		217,500	960,000	2,400,000	960,000	
-	Cat 385CL Hydraulic Excavator					323	m³ / un	250.00	986,860		161,500	789,488	1,973,720	789,488	
-	Cat 311C U					48	m³ / un	250.00	150,000		48,000	240,000	600,000	240,000	
-	Cat 14M Motorgrader					93	m³ / un	250.00	537,823		46,500	430,258	1,075,646	430,258	
-	Cat D8T LGP Track-Type Tractor					68	m³ / un	250.00	765,404		103,500	1,836,970	4,592,424	1,836,970	
-	Cat D7R II LGP Track-Type Tractor					68	m³ / un	250.00	370,865		102,000	890,076	2,225,190	890,076	
-	Cat D6T LGP Track-Type Tractor					61	m³ / un	250.00	538,844		61,000	862,150	2,155,376	862,150	
-	Cat 442E 2WS Backhoe Loader					55	m³ / un	250.00	105,115		27,500	84,092	210,230	84,092	
-	Cat 740 Articulated Dumper 40 T					156	m³ / un	250.00	561,300		546,000	3,143,280	7,858,200	3,143,280	
-	Cat 725 Articulated Dumper 25 T					98	m³ / un	250.00	427,450		171,500	1,196,860	2,992,150	1,196,860	
-	10 Wheeler Truck					90	m³ / un	250.00	220,125		405,000	1,584,900	3,962,250	1,584,900	
-	Cat CS76 XT Vibratory Soil Compactor					45	m³ / un	250.00	155,125		135,000	744,600	1,861,500	744,600	
	Construction	Site 1	Site 2	Site 3	Site 4										
	Cat GEP 150 - 100KW	2	1	1	2	6									

Item : (6330)

WBS	DESCRIPTION			Qty	Un.	Cons. Mat.	Freight in/out	Depr.	Purchase	Construction materials	Freight	Depreciation	Purchase	GLOBAL PRICES
		%	n											
							250							
	Cat GEP 550 - 400KW	9	2	2	13									
	Cat GEP 910 - 910kW				0									
	Cat GEP 1250 - 1250kW	4			4									
	Camps													
	Cat GEP 550 - 400KW	3	2	2	7									
	Cat GEP 910 - 910kW	7	3	3	16									
	Cat GEP 1250 - 1250kW													
	Generators													
-	Cat GEP 88 - 50KW			6 m³ / un	40%	4	24 m³	250.00	21,635		6,000	34,616	86,540	34,616
-	Cat GEP 150 - 100KW			9 m³ / un	40%	6	54 m³	250.00	26,300		13,500	63,120	157,800	63,120
-	Cat GEP 550 - 400KW			20 m³ / un	40%	20	400 m³	250.00	84,100		100,000	672,800	1,682,000	672,800
-	Cat GEP 910 - 910kW			24 m³ / un	40%	16	384 m³	250.00	339,250		96,000	2,171,200	5,428,000	2,171,200
-	Cat GEP 1250 - 1250kW			24 m³ / un	40%	4	96 m³	250.00	500,000		24,000	800,000	2,000,000	800,000
-	Generator 5 kW (Tower light)			5 m³ / un	40%	26	130 m³	250.00	5,100		32,500	53,040	132,600	53,040
-	R1300 G - Scooptram			42 m³ / un	40%	2	84 m³	250.00	307,850		21,000	246,280	615,700	246,280
-	Pick-up Ford F-150 (4x4)			23 m³ / un	100%	86	1,978 m³	250.00	17,345		0	0	0	0
-	Crew-cab Ford F-150 (4x4)			26 m³ / un	100%	94	2,444 m³	250.00	26,884		611,000	2,527,096	2,527,096	2,527,096
-	Escape (4x4)			16 m³ / un	100%	7	112 m³	250.00	16,850		28,000	117,950	117,950	117,950
-	Tractor truck			90 m³ / un	60%	4	360 m³	250.00	138,900		90,000	333,360	555,600	333,360
-	Load Carrier - 65 T			60 m³ / un	60%	2	120 m³	250.00	51,400			61,680	102,800	61,680
-	Trailer			60 m³ / un	60%	4	240 m³	250.00	40,000		60,000	96,000	160,000	96,000
-	Fuel Trailer			170 m³ / un	60%	2	340 m³	250.00	125,000		85,000	150,000	250,000	150,000
-	Welding Machine - 400 A			3 m³ / un	60%	123	369 m³	250.00	15,000		92,250	1,107,000	1,845,000	1,107,000
-	Moyno pump			m³ / un	60%								0	0
-	Jack leg			m³ / un	60%	2	77 m³	250.00			38,500		0	0
-	Injection pump			m³ / un	60%								0	0
-	Shotcrete pump			m³ / un	60%								0	0
-	Fork lift 10 T			10 m³ / un	60%	2	20 m³	250.00	15,000		5,000	18,000	30,000	18,000
-	Fork lift 15 T			15 m³ / un	60%	3	45 m³	250.00	30,000		11,250	54,000	90,000	54,000
-	Furukawa HCR9-ES			25 m³ / un	60%	12	300 m³	250.00	75,000		75,000	540,000	900,000	540,000
-	Hydraulic Drilling Machine			35 m³ / un	60%	14	490 m³	250.00	100,000		122,500	840,000	1,400,000	840,000
-	Bus - 32 Passengers			60 m³ / un	80%	8	480 m³	250.00	55,000		0	0	0	0
-	Compressor - 1050 cfm (XRHS1100CD6)			21 m³ / un	60%	4	84 m³	250.00	160,175		21,000	384,420	640,700	384,420
-	Compressor - 750 cfm			21 m³ / un	60%	12	252 m³	250.00	111,110		63,000	799,992	1,333,320	799,992
-	Compressor XAHS 237 (500 cfm)			23 m³ / un	60%	17	391 m³	250.00	90,000		97,750	918,000	1,530,000	918,000
-	Furnace - 2 500 000 BTU			3 m³ / un	100%	3	9 m³	250.00	16,000		0	0	0	0
-	Furnace - 1 000 000 BTU			3 m³ / un	100%	10	30 m³	250.00	12,000		7,500	120,000	120,000	120,000
-	Asphalt Paver			39 m³ / un	60%	4	156 m³	250.00	225,000	0	0	0	0	0
-	Bomag Twin Roller			3 m³ / un	60%	8	24 m³	250.00	20,000	0	6,000	96,000	160,000	96,000
-	Plate damper			1 m³ / un	60%	20	20 m³	250.00	6,500	0	5,000	78,000	130,000	78,000
	TUNNELS													
-	Rocket Boomer E3 C	Base	1,300,000 \$	0.9	1,170,000 USD						0	0	0	0
	Tailrace access and Caverns			118 m³ / un	40%	1	118 m³	250.00	1,170,000	0	29,500	468,000	1,170,000	468,000
	Powerhouse and Power tunnel Access			118 m³ / un	40%	1	118 m³	250.00	1,170,000	0	29,500	468,000	1,170,000	468,000

Item : (6330)

WBS	DESCRIPTION						Qty	Un.	Cons. Mat.	Freight in/out	Depr.	Purchase	Construction materials	Freight	Depreciation	Purchase	GLOBAL PRICES		
																		%	n
									250										
-	Exciter cubicles	750	2	2	3	150	8	1,200 m³	125.00			0	150,000	0	0	0	0		
-	Exciter cubicles	1,000	1	2	3	19	2	38 m³	125.00			0	4,750	0	0	0	0		
-	Exciter transformer	4,000	3	2	3	47	2	94 m³	125.00			0	11,750	0	0	0	0		
-	Control and protection	1,400	2	2	3	113	6	675 m³	125.00			0	84,375	0	0	0	0		
-	Station transformer	9,000	3	3	3	23	1	23 m³	125.00			0	2,875	0	0	0	0		
-	Battery Charger/Inverter	750	2	2	2	6	1	6 m³	125.00			0	750	0	0	0	0		
-	Battery	6,000	2	2	2	12	2	12 mt	250.00			0	3,000	0	0	0	0		
	Overhead Crane	100,150	kg					1,002 mt	250.00			0	250,500	0	0	0	0		
	Piping							18 mt	250.00			0	4,500	0	0	0	0		
	CVAC			77	m³ Cnt		5	385 m³	125.00			0	48,125	0	0	0	0		
	Mechanical Intake			(kg)								0	0	0	0	0	0		
	Trash Rack	22,000						22 mt	250.00			0	5,500	0	0	0	0		
	Stop logs	9,000						9 mt	250.00			0	2,250	0	0	0	0		
	Embedded parts	24,000						24 mt	250.00			0	6,000	0	0	0	0		
	Gates	8,000						8 mt	250.00			0	2,000	0	0	0	0		
	Spreader	3,500						4 mt	250.00			0	875	0	0	0	0		
	Winches	7,000						7 mt	250.00			0	1,750	0	0	0	0		
	Lining	8,000										0	0	0	0	0	0		
	Tunnel 1			(kg)								0	0	0	0	0	0		
	Upstream Stop logs	5,000						5 mt	250.00			0	1,250	0	0	0	0		
	Stoplogs Embedded parts	19,000						19 mt	250.00			0	4,750	0	0	0	0		
	Gates	17,000						17 mt	250.00			0	4,250	0	0	0	0		
	Gates Embedded parts	22,000						22 mt	250.00			0	5,500	0	0	0	0		
	Stoplogs Lifting Beam	1,000						1 mt	250.00			0	250	0	0	0	0		
	Actuator	3,000						3 mt	250.00			0	750	0	0	0	0		
	Downstream Stop logs	5,000						5 mt	250.00			0	1,250	0	0	0	0		
	Downstream Stop logs Lifting Beam	1,000						1 mt	250.00			0	250	0	0	0	0		
	Monorail	4,000						4 mt	250.00			0	1,000	0	0	0	0		
		77,000										0	0	0	0	0	0		
	MARINE EQUIPMENT											0	0	0	0	0	0		
	Water route			L (ft)	W (ft)	H (ft)	V (cu ft)					0	0	0	0	0	0		
-	Landing barge (Unifloat)	18	8	6.0	864		20	489 m³	250.00			0	122,333	0	0	0	0		
-	Noze end				432		12	147 m³	250.00			0	36,700	0	0	0	0		
-	Service barge	50	12	6.5	3,900		2	221 m³	250.00			0	55,220	0	0	0	0		
-	Tug	12	6	8.0	576		1	16 m³	250.00			0	4,078	0	0	0	0		
-	Work boat	8	4	6.0	192		2	11 m³	250.00			0	2,719	0	0	0	0		
-	Miscellaneous (winches, anchors, generators, etc..)											0	0	0	0	0	0		
	Tunnel 1											0	0	0	0	0	0		
-	Noze end				432		6	73 m³	250.00			0	18,350	0	0	0	0		
-	Working barge	50	12	7	3,900		6	663 m³	250.00			0	165,660	0	0	0	0		
-	Tug	12	6	8	576		3	49 m³	250.00			0	12,233	0	0	0	0		
-	Miscellaneous (winches, anchors, generators, etc..)				1,669 m³		10%	167 m³	250.00			0	41,750	0	0	0	0		
6330	Construction Equipment, Tools & Supplies - Hydro Site 6g															0	19,038,014	119,698,716	58,817,800

Item : (6513-6553)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Construction materials	Permanent Materials	Equipment Operation			
										24.00 \$					9.72 \$		
6500	Construction Camp																
6510	Site Preparation - Hydro Site 6g																
6513	Site Preparation - Hydro Site 6g			212,600	m²												
	Site 1	250	x	250	62,500	m²					0	0	0	0	0	0	0
	Site 2	250	x	250	62,500	m²					0	0	0	0	0	0	0
	Site 3	240	x	240	57,600	m²					0	0	0	0	0	0	0
	Site 4	100	x	300	30,000	m²					0	0	0	0	0	0	0
					212,600	m²					0	0	0	0	0	0	0
					3,000	m² / sh					0	0	0	0	0	0	0
					71	sh					0	0	0	0	0	0	0
					710	h					0	0	0	0	0	0	0
					10	h / sh					0	0	0	0	0	0	0
	- M-P			7	4,970	h	24.00				119,280	0	0	0	0	119,280	4,970
	- Cat D7R II LGP Track-Type Tractor	38.25		28.00	90%	1	639	h		38.25	28.00	0	0	0	24,442	0	24,442
	- Cat 725 Articulated Dumper 25 T	24.00		20.00	90%	3	1,917	h		24.00	20.00	0	0	0	46,008	0	46,008
	- Cat 329DL Hydraulic Excavator	19.00		29.00	90%	1	639	h		19.00	29.00	0	0	0	12,141	0	12,141
	- Cat CS76 XT Vibratory Soil Compactor	14.85		20.00	45%	1	320	h		14.85	20.00	0	0	0	4,745	0	4,745
	- Cat 14M Motor grader	16.65		25.75	45%	1	320	h		16.65	25.75	0	0	0	5,320	0	5,320
	- Misc. (Dust control, accessories, etc.)			1	ls		10,000					0	0	0	0	0	0
												10,000	0	0	0	0	10,000
												0	0	0	0	0	0
	Pavement (roads, parking lot, etc.)				150 mm of crushed stone							0	0	0	0	0	0
					on 50% of the total ara							0	0	0	0	0	0
		<u>m²</u>		<u>m²</u>	<u>mt</u>												
	Site 1	62,500	50%	4,688	8,438												
	Site 2	62,500	50%	4,688	8,438												
	Site 3	57,600	50%	4,320	7,776												
	Site 4	30,000	50%	2,250	4,050												
	Supply																
	- Crushed stone - Site 1				0.11	h / mt											
					8,438	mt	2.61	8.08	0.00	2.60	11.98						928
	- Crushed stone - Site 2				0.08	h / mt											
					8,438	mt	1.84	1.30	0.00	2.08	3.08						675
	- Crushed stone - Site 3				0.08	h / mt											
					7,776	mt	1.84	1.97	0.00	2.04	3.90						622
	- Crushed stone - Site 4				0.07	h / mt											
					4,050	mt	1.80	1.38	0.00	2.03	3.15						284
												0	0	0	0	0	0
6513	Site Preparation - Hydro Site 6g			212,600	m²							119,280	10,000	0	92,656	0	221,936
																1.04	7,479

Item : (6513-6553)

WBS	DESCRIPTION	%	n	Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS					
						M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Construction materials	Permanent Materials	Equipment Operation	Fuel Consumption								
											24.00 \$					9.72 \$							
6520 Infrastructure																							
6523 Infrastructure - Hydro Site 6g																							
Installation and removal (Generators, sewers, water supply, communications, etc...)																							
				80 sh																			
				20 sh / site																			
				10 h / sh																			
- M-O			26	20,800 h	24.00						499,200	0	0	0	0	0	499,200	20,800					
- Crane - Rough terrain 50 t (L-Belt)	37.00	20.00	25%	1	200 h				37.00	14.40	0	0	0	7,400	2,074	9,474							
- Cat 329DL Hydraulic Excavator	19.00	29.00	25%	1	200 h				19.00	20.88	0	0	0	3,800	3,007	6,807							
- Boom truck 17 tons	13.65	18.00	50%	1	400 h				13.65	12.96	0	0	5,460	3,732	9,192								
Power Station (diesel generators)																							
Site 1																							
Needed	8 kW / p-d	(Reserve)	Total																				
Workers (Including T-Line)	306	0	306	0%																			
Staff	102	0	102	0%																			
				408																			
Needed capacity	4.1 mW																						
	48 months																						
	1,440 days	24 h / d			34,560 h													0	0	0	0	0	0
- Transformers	1.75 un / mW			7 un	3,330	103,330	0	23,310	723,310	0	0	0	0	0	0	746,620							
- Building	Isolated steel building	700			700 m²	290	830	0	203,000	581,000	0	0	0	0	784,000								
- Electrical mains				1,700 m	110	480	0	187,000	816,000	0	0	0	0	1,003,000									
- Outdoor lighting	On poles			6 un	420	1,830	0	2,520	10,980	0	0	0	0	13,500									
- Reefer hotlines				22 un	300	2,000	0	6,600	44,000	0	0	0	0	50,600									
Site 2																							
Needed	8 kW / p-d	(Reserve)	Total																				
Workers (Including T-Line)	162	0	162	0%																			
Staff	54	0	54	0%																			
				216																			
Needed capacity	2.2 mW																						
	36 months																						
	1,080 days	24 h / d			25,920 h													0	0	0	0	0	
- Transformers	1.75 un / mW			4 un	3,330	103,330	0	13,320	413,320	0	0	0	0	426,640									
- Building	Isolated steel building	600			600 m²	290	830	0	174,000	498,000	0	0	0	672,000									
- Electrical mains				1,700 m	110	480	0	187,000	816,000	0	0	0	0	1,003,000									
- Outdoor lighting	On poles			6 un	420	1,830	0	2,520	10,980	0	0	0	0	13,500									
- Reefer hotlines				22 un	300	2,000	0	6,600	44,000	0	0	0	0	50,600									
Site 3																							
Needed	8 kW / p-d	(Reserve)	Total																				
Workers (Including T-Line)	111	0	111	0%																			
Staff	37	0	37	0%																			
				148																			
Needed capacity	1.5 mW																						
	24 months																						
	720 days	24 h / d			17,280 h													0	0	0	0	0	
- Transformers	1.75 un / mW			3 un	3,330	103,330	0	9,990	309,990	0	0	0	0	319,980									
- Building	Isolated steel building	400			400 m²	290	830	0	116,000	332,000	0	0	0	448,000									
- Electrical mains				1,700 m	110	480	0	187,000	816,000	0	0	0	0	1,003,000									
- Outdoor lighting	On poles			6 un	420	1,830	0	2,520	10,980	0	0	0	0	13,500									

Item : (6513-6553)

WBS	DESCRIPTION			UNIT PRICES						TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Construction materials	Permanent Materials	Equipment Operation				Fuel Consumption
										24,00 \$					9,72 \$			
	- Cable for TV and Internet			1,500	m		18	14		0	0	0	0	0	0	0	0	0
										0	27,000	21,000	0	0	0	0	0	48,000
										0	0	0	0	0	0	0	0	0
	Site 2									0	0	0	0	0	0	0	0	0
	- Communication mast			1	un		13,000	63,000		0	13,000	63,000	0	0	0	0	0	76,000
										0	0	0	0	0	0	0	0	0
	- broadcast of Radio and W-Fi Terminal equipment			1	ls		20,000	108,000		0	20,000	108,000	0	0	0	0	0	128,000
										0	0	0	0	0	0	0	0	0
	- Cable for TV and Internet			1,500	m		18	14		0	27,000	21,000	0	0	0	0	0	48,000
	- Repeater Tower mast			1	un		13,000	63,000		0	13,000	63,000	0	0	0	0	0	76,000
										0	0	0	0	0	0	0	0	0
	Site 3									0	0	0	0	0	0	0	0	0
	- Communication mast			1	un		13,000	63,000		0	13,000	63,000	0	0	0	0	0	76,000
										0	0	0	0	0	0	0	0	0
	- broadcast of Radio and W-Fi Terminal equipment			1	ls		20,000	108,000		0	20,000	108,000	0	0	0	0	0	128,000
										0	0	0	0	0	0	0	0	0
	- Cable for TV and Internet			1,500	m		18	14		0	27,000	21,000	0	0	0	0	0	48,000
	- Repeater Tower mast			1	un		13,000	63,000		0	13,000	63,000	0	0	0	0	0	76,000
										0	0	0	0	0	0	0	0	0
	Site 4									0	0	0	0	0	0	0	0	0
	- Communication mast			1	un		13,000	63,000		0	13,000	63,000	0	0	0	0	0	76,000
										0	0	0	0	0	0	0	0	0
	- broadcast of Radio and W-Fi Terminal equipment			1	ls		20,000	108,000		0	20,000	108,000	0	0	0	0	0	128,000
										0	0	0	0	0	0	0	0	0
	- Cable for TV and Internet			1,500	m		18	14		0	27,000	21,000	0	0	0	0	0	48,000
	- Repeater Tower mast			1	un		13,000	63,000		0	13,000	63,000	0	0	0	0	0	76,000
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
										0	0	0	0	0	0	0	0	0
6523	Infrastructure - Hydro Site 6g									1,710,720	7,774,930	11,142,230	869,498	490,379	21,987,757			71,280

Item : (6513-6553)

WBS	DESCRIPTION	%	n	Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
							Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Construction materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$					9.72 \$			
6530 Camps																			
6533 Camps - Hydro Site 6g																			
Buildings																			
Site 1																			
Needed		8 kW / p-d		(Reserve)	Total														
Workers (Including T-Line)		306		0	306	0%													
Staff		102		0	102	0%													
					408														
Accommodations (including common rooms)																			
		men / mod.	Living Mod.	Common Mod.	Total Mod														
					24.36	m² / mod													
Workers	4	77	31	108	2,619														
Staff	2	51	20	71	1,730														
					179	4,349	m²												
Supply and install																			
- Workers					2,619 m ²	240	1,320												
- Staff					1,730 m ²	270	1,450												
					Mod	m²													
Service building																			
- Offices	0.0284 mod / men				12	292	292 m ²	270	1,320										
- Dining	20 seats / mod					341	341 m ²	270	1,470										
	1.2 m ² / p																		
	285.6 seats				14	341													
- Kitchen (incl. Day storage)	50% of dining				7	171	171 m ²	590	1,770										
- Laundry	1 mod / 200 p				3	73	73 m ²	590	1,770										
- Recreation Hall	1.5 m ² / p (70 %)	428			18	428	428 m ²	250	1,240										
- Infirmary	100 p / bed					49	49 m ²	680	2,020										
	4 beds																		
	2 beds / mod				2	49													
					56														
					234														
Workshops																			
- Mechanical	Isolated steel building				200	200 m ²	280	1,250											
- Electrical	Isolated steel building				200	200 m ²	280	1,250											
- Carpenter	Isolated steel building				200	200 m ²	280	1,250											
- Garage and Fire Fighting	Isolated steel building				400	400 m ²	290	830											
					1,000														
- Fire fighting Equipment	Isolated steel building				150	1 ls	330	366,670											
Site 2																			
Needed		8 kW / p-d		(Reserve)	Total														
Workers (Including T-Line)		162		0	162	0%													
Staff		54		0	54	0%													
					216														
Accommodations (including common rooms)																			
		men / mod.	Living Mod.	Common Mod.	Total Mod														
					24.36	m² / mod													
Workers	4	41	16	57	1,376														
Staff	2	27	11	38	926														
					95	2,302	m²												
Supply and install																			
- Workers					1,376 m ²	240	1,320												
- Staff					926 m ²	270	1,450												
					Mod	m²													

Item : (6513-6553)

WBS	DESCRIPTION	%	n	Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
						M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Construction materials	Permanent Materials	Equipment Operation	Fuel Consumption					
				49	m²		270	1,470			24.00 \$	0	13,230	72,030	0	0	0	85,260		
- Dining	20 seats / mod										0	0	0	0	0	0	0	0		
	1.2 m² / p										0	0	0	0	0	0	0	0		
	49.0 seats		2	49							0	0	0	0	0	0	0	0		
- Kitchen (incl. Day storage)	50% of dining		1	24	24	m²		590	1,770		0	14,160	42,480	0	0	0	0	56,640		
- Laundry	1 mod / 200 p		1	24	24	m²		590	1,770		0	14,160	42,480	0	0	0	0	56,640		
- Recreation Hall	1.5 m² / p (70 %)		63	22	22	m²		250	1,240		0	5,500	27,280	0	0	0	0	32,780		
											0	0	0	0	0	0	0	0		
- Infirmary	100 p / bed				12	m²		680	2,020		0	8,160	24,240	0	0	0	0	32,400		
	1 beds										0	0	0	0	0	0	0	0		
	2 beds / mod		1	12							0	0	0	0	0	0	0	0		
			33								0	0	0	0	0	0	0	0		
											0	0	0	0	0	0	0	0		
											0	0	0	0	0	0	0	0		
											0	0	0	0	0	0	0	0		
											0	0	0	0	0	0	0	0		
											0	0	0	0	0	0	0	0		
											0	0	0	0	0	0	0	0		
											0	0	0	0	0	0	0	0		
6533	Camps - Hydro Site 6g										0	13,547,055	81,614,737	0	0	0	0	95,161,792		0

6540 Catering

6543 Catering - Hydro Site 6g																			
Total Site 1 to 4																			
		m-hours	p-d	p-month															
Staff			138,060	4,602															
Workers		m-h / d>>>	10																
Directs		1,591,019	159,102	5,303															
Indirects		456,380	45,638	1,521															
Catering (including Camp Operation)			22,974	766	12%														
Miscellaneous		250,000	25,000	833															
			252,714	8,424															
			390,774	13,026															
Based on different sources, Total cost is estimated at 80.00 USD / m-d																			
This is including Catering and Operation of the camps																			
Catering	38%	38%																	
Operation		62%																	
Security and infirmary	8%																		
Maintenance	5%																		
Power station	49%																		
	100%	100%																	
			80.00 USD / p-d																
- Catering		2,433.33	USD / p-month	38%	13,026	p-mth	924.67				0	12,044,518	0	0	0	0	12,044,518		
											0	0	0	0	0	0	0		
6543	Catering - Hydro Site 6g										0	12,044,518	0	0	0	0	12,044,518		0

Item : (6513-6553)

WBS	DESCRIPTION			Qty	Un.	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n			M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Construction materials	Permanent Materials	Equipment Operation	Fuel Consumption					
	- Miscellaneous (Storage pond, membrane, piping, etc...)	20%		1	ls		9,940					24,00 \$	9,940	0	0	0	0	9,940		
	Site 2																			
	Needed 8 kW / p-d (Reserve) Total																			
	Workers (Including T-Line)	162	0	162	0%															
	Staff	54	0	54	0%															
	2.2 mW			216																
	Needed capacity																			
	36 months (First 2 months on pioneer camp)																			
	1,095 days 24 h / d			26,280	h															
	Spare																			
	Installed																			
3	Cat GEP 910 - 910KW	1	8.50	130.80	1,820	50%	2	26,280	h			8.50	130.80	0	0	0	223,380	2,474,945	2,698,325	
2	Cat GEP 550 - 400KW	1	6.50	102.40	400	50%	1	13,140	h			6.50	102.40	0	0	0	85,410	968,786	1,054,196	
					2,220.0															
	Fuel Storage																			
	Total fuel consumption		4,783	kL																
	133 kL / month																			
	Transportation from harbor																			
	75 kL / trip																			
	1.8 trips / mth																			
	64 trips 10 h / trip							640	h											
	- M-P					2	1,280	h		24.00				30,720	0	0	0	0	30,720	
	- Fuel Tanker 75 kL		11.50	15.00	90%	1	576	h				12	15.00	0	0	6,624	6,221	12,845		
	Fuel Tank Reserve for 1 month 266 kL																			
	- Fuel tank 795 kL 154,500 \$						1	un		154,500			154,500	0	0	0	0	154,500		
	- Miscellaneous (Storage pond, membrane, piping, etc...)	20%				1	ls			30,900			30,900	0	0	0	0	30,900		
	Site 3																			
	Needed 8 kW / p-d (Reserve) Total																			
	Workers (Including T-Line)	111	0	111	0%															
	Staff	37	0	37	0%															
	1.5 mW			148																
	Needed capacity																			
	24 months (First 2 months on pioneer camp)																			
	730 days 24 h / d							17,520	h											
	Spare																			
	Installed																			
3	Cat GEP 910 - 910kW	1	8.50	130.80	1,820	50%	2	17,520	h			8.50	130.80	0	0	0	148,920	1,649,964	1,798,884	
					1,820.0															
	Fuel Storage																			
	Total fuel consumption		2,292	kL																
	95 kL / month																			
	Transportation from harbor																			
	75 kL / trip																			
	1.3 trips / mth																			
	31 trips 15 h / trip							465	h											
	- M-P					2	930	h		24.00				22,320	0	0	0	0	22,320	
	- Fuel Tanker 75 kL		11.50	15.00	90%	1	419	h				12	15.00	0	0	4,819	4,525	9,344		
	Fuel Tank Reserve for 1 month 191 kL																			
	- Fuel tank 189 kL 49,700 \$						1	un		49,700			49,700	0	0	0	0	49,700		
	- Miscellaneous (Storage pond, membrane, piping, etc...)	20%				1	ls			9,940			9,940	0	0	0	0	9,940		

Item : (7130)

WBS	DESCRIPTION		Qty	Un.	TOTAL COSTS				GLOBAL PRICES	UNIT PRICES	MEN-HOURS
					Man power	65	Permanent Materials	Equipment Operation			
		%	n								
					25.60 \$				0.72 \$		

7000 EPCM Home Office

7100 EPCM Home Office - FEL 1 & 2

7130 EPCM Home Office - FEL 1 & 2 - Hydro Site 6g											
Contractor											
Percentage of direct costs											
191,167,128 \$ 2%											
General Managing											
3,823,343											
2,000,000											
7130 EPCM Home Office - FEL 1 & 2 - Hydro Site 6g											
5,823,343											

Item : (8130)

Art.	DESCRIPTION	%	n	Qty	Un.	COÛT UNITAIRE		TOTAL COSTS		GLOBAL PRICES	Men-hours
						M-P		Men Power			
						Monthly	Hourly	Monthly	Hourly		
8000	EPCM Field Office										
8100	EPCM Field Office - FEL 1 & 2										
8130	EPCM Field Office - FEL 1 & 2 - Hydro Site 6g										
	Contractor Staff										
	Site 1		1,876								
	Site 2		545								
	Site 3		248								
	Site 4		296								
			2,965 P-Months								
	General Managing Staff										
	Site 1		1,092								
	Site 2		257								
	Site 3		121								
	Site 4		149								
			1,619 P-Months								
			4,584								
-	Average		10,000 \$ / month		4,584	10,000.00		45,840,000		45,840,000	
8130	EPCM Field Office - FEL 1 & 2 - Hydro Site 6g									45,840,000	

Item : Crusher (1)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
	Crusher 1			56 751 mt													
	Crusher Plan System (Portable)																
	Needs																
	Site 1																
	Concrete	7 600 m³		(mt)													
		40-20	0.530 mt / m³	4 028													
		20-05	0.530 mt / m³	4 028													
		Sand	0.855 mt / m³	6 498													
				14 554 mt													
	Road Pavement (Using TBM excavated material)																
			1.8 mt / m³														
	Road Pavement	23 443 m³	1.8 mt / m³	42 197													
				42 197 mt													
	Operation during summer periods only																
	Stockpiling a small amount for next springtime start																
	Powerhouse area	100 mt / h (eff.)															
		(mt)	Operation	(hours)													
	2 011	20 000	200	200													
	2 012	10 000	100	100													
	2 013	15 000	150	150													
	2 014	15 000	150	150													
	2 015	3 300	33	33													
		63 300	633	633													
	Crushing and stockpiling	Total Hours		633													
				Say													
				630 h													
	- M-P			7	4 410 h	24.00										105 840	4 410
	- Cat 950H Wheel Loader	18.35	9.05	100%	2	1 260 h			18.35	9.05			23 121	8 210		31 331	
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	100%	1	630 h			38.25	28.00			24 098	12 701		36 799	
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	100%	3	1 890 h			24.00	20.00			45 360	27 216		72 576	
	- Crusher Assembly (300 t / h)			100%	1	630 h			50.00	84.00			31 500	38 102		69 602	
	Production of	1 800 mt / day															
		1.8 mt / m³	1 000 m³ / d														
	Average hauling distance :		2.00 km														
	Loading	3															
	Going	4	30 km / h														
	Unloading	3															
	Return	4	30 km / h														
		14 min.															
	Efficiency :	85%	16 min. / trip														

Item : Crusher (1)

WBS	DESCRIPTION	%	n	Qty	Un.	M-P	UNIT PRICES					TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
							Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
											24.00 \$						0.72 \$		
	0.27 h / trip										0	0	0	0	0	0	0		
	9 h / sh										0	0	0	0	0	0	0		
	33 trips / day										0	0	0	0	0	0	0		
	Cat 725 Articulated Dumper 25 T										0	0	0	0	0	0	0		
	12 m³										0	0	0	0	0	0	0		
	396 m³ / truck-sh										0	0	0	0	0	0	0		
	Number of trucks : 3										0	0	0	0	0	0	0		
	0										0	0	0	0	0	0	0		
	Stock pile Winter protection Shelter										0	0	0	0	0	0	0		
	100 x 15 m	300	15	4 500 m²							0	0	0	0	0	0	0		
	Supply			4 500 m²			80.00				0	360 000	0	0	0	0	0	360 000	
	Installation and removing			8 sh							0	0	0	0	0	0	0	0	
	10 h / s			80 h							0	0	0	0	0	0	0	0	
	- M-P		6	480 h		24.00					11 520	0	0	0	0	0	0	11 520	480
	- Boom truck 17 tons	13.65	18.00	90% 1	72 h			13.65	12.96		0	0	0	983	672	0	1 655		
	- Crane - Rough terrain 30 t (L-Belt)	33.00	18.00	90% 1	72 h			33.00	12.96		0	0	0	2 376	672	0	3 048		
	- Miscellaneous (footing, railing, etc...)			600 m			110.00				0	66 000	0	0	0	0	66 000		
	Heating	2 013	6 month	180 days							0	0	0	0	0	0	0		
	24 h / d			4 320 h							0	0	0	0	0	0	0		
	- M-P		2 h / d	2	720 h	24.00					17 280	0	0	0	0	0	17 280	720	
	- Boiler - 1500 kW	4.00	190.00		4 320 h			4.00	190.00		0	0	0	17 280	590 976	0	608 256		
	- Miscellaneous (piping,pumps, etc...)			1 ls			30 000				0	30 000	0	0	0	0	30 000		
	Crusher Installation and Removing			8 sh							0	0	0	0	0	0	0		
	10 h / sh			80 h							0	0	0	0	0	0	0		
	- M-P		7	560 h		24.00					13 440	0	0	0	0	0	13 440	560	
	- Cat 950H Wheel Loader	18.35	9.05	75% 1	60 h			18.35	9.05		0	0	0	1 101	391	0	1 492		
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	40% 1	32 h			38.25	28.00		0	0	0	1 224	645	0	1 869		
	- Cat 329DL Hydraulic Excavator	19.00	29.00	30% 1	24 h			19.00	29.00		0	0	0	456	501	0	957		
	- Miscellaneous			80 h			30.00				0	2 400	0	0	0	0	2 400		
	0										0	0	0	0	0	0	0		
	0 Crusher 1			56 751 mt							148 080	458 400	0	147 499	680 086	1 434 065	25.27	6 170	
	Unit costs			mt							2.61	8.08	0.00	2.60	11.98		0.11		

Item : Crusher (2)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
	Crusher 2			352,279 mt													
	Crusher Plan System (Portable)																
	Needs																
	Site 2																
	Concrete	4,000 m³		(mt)													
	40-20	0.530 mt / m³		2,120													
	20-05	0.530 mt / m³		2,120													
	Sand	0.855 mt / m³		3,420													
	Road Pavement	(Using TBM excavated material)															
	Road Pavement	1.8 mt / m³															
	19,805 m³			35,649													
	Dam impervious core																
	2,500 m³	1.8 mt / m³		4,500													
	Dam Filter																
	169,150 m³	1.8 mt / m³		304,470													
				352,279 mt													
	Operation during summer periods only																
	Stockpiling a small amount for next springtime start																
	Camp 2	100 mt / h (eff.)															
	(m) Operation (hours)																
	2,013	150,000	1,500	1,500													
	2,014	150,000	1,500	1,500													
	2,015	53,000	530	530													
		353,000	3,530	3,530													
	Crushing and stockpiling	Total Hours		3,530													
		Say		3,600 h													
	- M-P			7	25,200 h	24.00				604,800					604,800		25,200
	- Cat 950H Wheel Loader	18.35	9.05	100% 2	7,200 h			18.35	9.05				132,120	46,915	179,035		
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	100% 1	3,600 h			38.25	28.00				137,700	72,576	210,276		
	- Cat 725 Articulated Dumper 25 T	24.00	20.00	100% 3	10,800 h			24.00	20.00				259,200	155,520	414,720		
	- Crusher Assembly (300 t / h)			100% 1	3,600 h			50.00	84.00				180,000	217,728	397,728		
	Production of	1,800 mt / day															
	1.8 mt / m³	1,000 m³ / d															
	Average hauling distance :	2.00 km															
	Loading	3															
	Going	4	30 km / h														
	Unloading	3															
	Return	4	30 km / h														

Item : Concrete (2)

WBS	DESCRIPTION	UNIT PRICES									TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS		
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l / h	m - h / un	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption	
											24.00 \$					0.72 \$				
	Building and Equipment																			
	- Portable Batch plan - 25 m³ / h			1	un			75,000				0	0	75,000	0	0	75,000			
	- Building (Plan & Cement storage) 1,140 m²			1	un			53,540				0	0	53,540	0	0	53,540			
	- Control module 320 sf			1	un			65,500				0	0	65,500	0	0	65,500			
	- Boiler			1	un			18,000				0	0	18,000	0	0	18,000			
	- Miscellaneous (cement storage, water tank, etc.)			1	un			50,750				0	0	50,750	0	0	50,750			
												0	0	0	0	0	0			
												0	0	0	0	0	0			
	Cement																			
	- Purchase 350 kg / m³ 1,400 mt Losses 5%			1,470	mt			73.00				0	0	107,310	0	0	107,310			
	4,000 m³											0	0	0	0	0	0			
												0	0	0	0	0	0			
	- Freight 275.00 \$ CDN / mt 250.00 USD / mt			1,470	mt			250.00				0	0	367,500	0	0	367,500			
												0	0	0	0	0	0			
	- Insurance 0.25 \$ / 100\$			1,470	mt			0.1825				0	0	268	0	0	268			
												0	0	0	0	0	0			
												0	0	0	0	0	0			
	Aggregates																			
	- 40-20 and 20-05 crushed stone & Sand 2.00 mt / m³			8,000	mt			1.84	1.30	0.00	2.08	3.08	0.08	14,720	10,400	0	16,640	17,741	59,501	640
	- Additives 4,000 m³							2.00				0	0	8,000	0	0	8,000			
												0	0	0	0	0	0			
												0	0	0	0	0	0			
	Mixing 416 days																			
	10 h / d			4,160	h							0	0	0	0	0	0			
												0	0	0	0	0	0			
	- M-P 3			12,480	h	24.00						299,520	0	0	0	0	299,520		12,480	
	- Cat 950H Wheel Loader 18.35 9.05 90% 1			3,744	h				18.35	9.05		0	0	0	68,702	24,396	93,098			
	- Concrete plan 90% 1			3,744	h				10.00			0	0	0	37,440	0	37,440			
	Cement transportation Distance 20 km											0	0	0	0	0	0			
	1,470 mt											0	0	0	0	0	0			
	40 mt / trip 37 trips 6 h / trip			222	h							0	0	0	0	0	0			
												0	0	0	0	0	0			
	- M-P 4			888	h	24.00						21,312	0	0	0	0	21,312		888	
												0	0	0	0	0	0			
	- Tractor & Trailer 11.50 15.00 90% 1			200	h				11.50	15.00		0	0	0	2,300	2,160	4,460			
	- Crane - Rough terrain 30 t (L-Belt) 33.00 18.00 90% 1			200	h				33.00	18.00		0	0	0	6,600	2,592	9,192			
												0	0	0	0	0	0			
	Batch Plan Installation and Removing			24	sh							0	0	0	0	0	0			
				240	h							0	0	0	0	0	0			
												0	0	0	0	0	0			
	- M-P 9			2,160	h	24.00						51,840	0	0	0	0	51,840		2,160	
												0	0	0	0	0	0			
	- Cat 950H Wheel Loader 18.35 9.05 75% 1			180	h				18.35	9.05		0	0	0	3,303	1,173	4,476			
	- Cat D7R II LGP Track-Type Tractor 38.25 28.00 40% 1			96	h				38.25	28.00		0	0	0	3,672	1,935	5,607			
	- Cat 329DL Hydraulic Excavator 19.00 29.00 30% 1			72	h				19.00	29.00		0	0	0	1,368	1,503	2,871			
	- Welding Machine - 400 A 2.00 6.00 60% 1			144	h				2.00	6.00		0	0	0	288	622	910			
												0	0	0	0	0	0			
	- Miscellaneous 1 ls				ls			10,000				0	10,000	0	0	0	10,000			
												0	0	0	0	0	0			
0	Concrete 2			4,000	m³							387,392	20,400	745,868	140,313	52,122	1,346,095		16,168	

Unit costs m³

96.85 5.10 186.47 35.08 13.03 336.52 4.04

Item : Crusher (3)

WBS	DESCRIPTION	UNIT PRICES								TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Consumable materials	Permanent Materials	Equipment Operation			
										24.00 \$					0.72 \$		
Crusher 3				232,962 mt													
Crusher Plan System (Portable)																	
Needs																	
<u>Powerhouse area</u>																	
Concrete	1,400 m³		(mt)														
	40-20	0.530 mt / m³	742														
	20-05	0.530 mt / m³	742														
	Sand	0.855 mt / m³	1,197														
<u>Road Pavement</u>																	
	17,434 m³	1.8 mt / m³	31,381														
<u>Dam Filter</u>																	
	110,500 m³	1.8 mt / m³	198,900														
			<u>232,962 mt</u>														
Operation during summer periods only																	
Stockpiling a small amount for next springtime start																	
Camp 3																	
		100 mt / h (eff.)															
	(mt)	<u>Operation</u>	(hours)														
	2,014	232,962	2,330														
Crushing and stockpiling																	
		Total Hours	<u>2,330</u>														
		Say	2,330 h														
-	M-P		7	16,100 h	24.00					386,400						386,400	16,100
-	Cat 950H Wheel Loader	18.35	9.05	100% 2	4,600 h			18.35	9.05				84,410	29,974		114,384	
-	Cat D7R II LGP Track-Type Tractor	38.25	28.00	100% 1	2,300 h			38.25	28.00				87,975	46,368		134,343	
-	Cat 725 Articulated Dumper 25 T	24.00	20.00	100% 3	6,900 h			24.00	20.00				165,600	99,360		264,960	
-	Crusher Assembly (300 t / h)			100% 1	2,300 h			50.00	84.00				115,000	139,104		254,104	
	Production of	1,800 mt / day															
	1.8 mt / m³	1,000 m³ / d															
	Average hauling distance :	2.00 km															
	Loading	3															
	Going	4	30 km / h														
	Unloading	3															
	Return	4	30 km / h														
		14 min.															
	Efficiency :	85%	16 min. / trip														
			0.27 h / trip														
			9 h / sh														
			33 trips / day														
	Cat 725 Articulated Dumper 25 T	12 m³															
		396 m³ / truck-sh															

Item : Crusher (3)

WBS	DESCRIPTION	UNIT PRICES										TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
	Number of trucks : 3										24.00 \$					0.72 \$			
	Stock pile Winter protection Shelter 100 x 15 m 300 15 4,500 m²										0	0	0	0	0	0	0	0	
	Supply			4,500 m²			80.00				0	360,000	0	0	0	360,000	0		
	Installation and removing			8 sh							0	0	0	0	0	0	0		
	10 h / s			80 h							0	0	0	0	0	0	0		
	- M-P			6 480 h		24.00					11,520	0	0	0	0	11,520	480		
	- Boom truck 17 tons	13.65	18.00	90%	1			13.65	12.96		0	0	0	983	672	1,655			
	- Crane - Rough terrain 30 t (L-Belt)	33.00	18.00	90%	1			33.00	12.96		0	0	0	2,376	672	3,048			
	- Miscellaneous (footing, railing, etc...)			600 m			110.00				0	66,000	0	0	0	66,000			
	Heating 2,013 6 month 180 days			24 h / d							0	0	0	0	0	0	0		
				4,320 h							0	0	0	0	0	0	0		
	- M-P			2 h / d		24.00					17,280	0	0	0	0	17,280	720		
	- Boiler - 1500 kW	4.00	190.00					4.00	190.00		0	0	0	17,280	590,976	608,256			
	- Miscellaneous (piping,pumps, etc...)			1 ls			30,000				0	30,000	0	0	0	30,000			
	Crusher Installation and Removing			8 sh							0	0	0	0	0	0	0		
	10 h / sh			80 h							0	0	0	0	0	0	0		
	- M-P			7 560 h		24.00					13,440	0	0	0	0	13,440	560		
	- Cat 950H Wheel Loader	18.35	9.05	75%	1			18.35	9.05		0	0	0	1,101	391	1,492			
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	40%	1			38.25	28.00		0	0	0	1,224	645	1,869			
	- Cat 329DL Hydraulic Excavator	19.00	29.00	30%	1			19.00	29.00		0	0	0	456	501	957			
	- Miscellaneous			80 h			30.00				0	2,400	0	0	0	2,400			
											0	0	0	0	0	0	0		
0	Crusher 3			232,962 mt							428,640	458,400	0	476,405	908,663	2,272,108	9.75	17,860	

Unit costs	mt	1.84	1.97	0.00	2.04	3.90	0.08
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Item : Concrete (4)

WBS	DESCRIPTION	UNIT PRICES									TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	m - h / un	Man power	Consumable materials	Permanent Materials	Equipment Operation				Fuel Consumption
											24.00 \$					0.72 \$			
	15,300																		
	Purchase																		
	Building and Equipment																		
	- Portable Batch plan - 25 m³ / h			1 un				75,000				0	0	75,000	0	0	0	0	75,000
	- Building (Plan & Cement storage) 1,140 m²			1 un				53,540				0	0	53,540	0	0	0	0	53,540
	- Control module 320 sf			1 un				65,500				0	0	65,500	0	0	0	0	65,500
	- Boiler			1 un				18,000				0	0	18,000	0	0	0	0	18,000
	- Miscellaneous (cement storage, water tank, etc.)			1 un				50,750				0	0	50,750	0	0	0	0	50,750
												0	0	0	0	0	0	0	0
	Cement																		
	- Purchase 350 kg / m³ 805 mt Losses 5%			845 mt				73.00				0	0	61,685	0	0	0	0	61,685
	2,300 m³											0	0	0	0	0	0	0	0
	- Freight 250.00 USD / mt			845 mt				250.00				0	0	211,250	0	0	0	0	211,250
	275.00 \$ CDN / mt											0	0	0	0	0	0	0	0
	- Insurance 0.25 \$ / 100\$			845 mt				0.1825				0	0	154	0	0	0	0	154
												0	0	0	0	0	0	0	0
	Aggregates																		
	- 40-20 and 20-05 crushed stone & Sand 2.00 mt / m³			4,600 mt		1.80	1.38	0.00	2.03	3.15	0.07	8,280	6,348	0	9,338	10,433	34,399	34,399	322
	- Additives 2,300 m³							2.00				0	0	4,600	0	0	0	0	4,600
												0	0	0	0	0	0	0	0
	Mixing 377 days											0	0	0	0	0	0	0	0
	10 h / d			3,770 h								0	0	0	0	0	0	0	0
												0	0	0	0	0	0	0	0
	- M-P 3 11,310 h 24.00											271,440	0	0	0	0	0	0	271,440
												0	0	0	0	0	0	0	0
	- Cat 950H Wheel Loader 18.35 9.05 90% 1 3,393 h								18.35	9.05		0	0	0	62,262	22,109	84,371	84,371	
	- Concrete plan 90% 1 3,393 h								10.00			0	0	0	33,930	0	33,930	33,930	
	Cement transportation Distance 40 km											0	0	0	0	0	0	0	0
	845 mt											0	0	0	0	0	0	0	0
	40 mt / trip 21 trips 10 h / trip			210 h								0	0	0	0	0	0	0	0
												0	0	0	0	0	0	0	0
	- M-P 3 630 h 24.00											15,120	0	0	0	0	0	0	15,120
												0	0	0	0	0	0	0	0
	- Tractor & Trailer 11.50 15.00 90% 1 189 h								11.50	15.00		0	0	0	2,174	2,041	4,215	4,215	
	- Crane - Rough terrain 30 t (L-Belt) 33.00 18.00 90% 1 189 h								33.00	18.00		0	0	0	6,237	2,449	8,686	8,686	
												0	0	0	0	0	0	0	0
	Water route 20 h / trip			420 h								0	0	0	0	0	0	0	0
												0	0	0	0	0	0	0	0
	- M-P 3 1,260 h 24.00											30,240	0	0	0	0	0	0	30,240
												0	0	0	0	0	0	0	0
	- Marine Equipment 420 h								60.00			0	0	0	25,200	0	25,200	25,200	
												0	0	0	0	0	0	0	0
	Batch Plan Installation and Removing			24 sh								0	0	0	0	0	0	0	0
	10 h / sh			240 h								0	0	0	0	0	0	0	0
												0	0	0	0	0	0	0	0
	- M-P 9 2,160 h 24.00											51,840	0	0	0	0	0	0	51,840
												0	0	0	0	0	0	0	0

Item : Concrete (4)

WBS	DESCRIPTION	UNIT PRICES										TOTAL COSTS					GLOBAL PRICES	UNIT PRICES	MEN-HOURS	
		%	n	Qty	Un.	M-P	Cons. Mat.	Perm. Mat.	Equip. Op.	Fuel l/h	m - h / un	Man power	Consumable materials	Permanent Materials	Equipment Operation	Fuel Consumption				
												24.00 \$					0.72 \$			
	- Cat 950H Wheel Loader	18.35	9.05	75% 1	180 h				18.35	9.05		0	0	0	3,303	1,173		4,476		
	- Cat D7R II LGP Track-Type Tractor	38.25	28.00	40% 1	96 h				38.25	28.00		0	0	0	3,672	1,935		5,607		
	- Cat 329DL Hydraulic Excavator	19.00	29.00	30% 1	72 h				19.00	29.00		0	0	0	1,368	1,503		2,871		
	- Welding Machine - 400 A	2.00	6.00	60% 1	144 h				2.00	6.00		0	0	0	288	622		910		
	- Miscellaneous			1 ls			10,000				0	0	0	0	0	0		0		
											0	10,000	0	0	0	0		10,000		
											0	0	0	0	0	0		0		
											0	0	0	0	0	0		0		
0	Concrete 4			3,000 m³							376,920	16,348	540,479	147,772	42,265		1,123,784		15,682	
	Unit costs			m³							125.64	5.45	180.16	49.26	14.09		374.59		5.23	

